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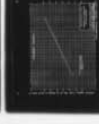
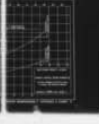
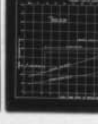
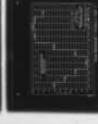
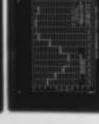
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PROJECT

ASKA

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DESIGN MEMORANDUM-NO-7-
GENERAL DESIGN MEMORANDUM

DESIGN

POWER

TRANSMISSION

3 - GEOLOGY AND MATERIALS

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APPENDIX A

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SNETTISHAM PROJECT, ALASKA

DESIGN MEMORANDUM NO. 7

Appendix A - Power

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SNETTISHAM POWER STUDIES
PERTINENT DATA - POWERPLANT

Location:

Near mouth of Speel River, 28 air miles southeast of Juneau, Alaska.

Power Installation:

Long Lake

Crater Lake

First stage development:

Number of units	2
Installed capacity	40,600
Capability-115% rated capacity, KW	46,700

Second stage development:

Number of units	1
Installed capacity, KW	20,300
Capability-115% rated capacity, KW	23,350

General:

Pool elevations, ft msl:

Normal power pool	843	961
Maximum power pool	895	1022
Minimum power pool	720 <u>1/</u>	828 <u>1/</u>
Maximum uncontrolled, spillway design flood, cfs	21,300 el. 903	<u>2/</u>

Tailwater elevations, ft msl:

Normal tailwater	0.5
Maximum tailwater	16.0
Minimum tailwater	- 1.0

Reservoir storage, acre-feet

Storage upper limit	484,000	121,000
Storage lower limit	232,000	40,000
Active storage	252,000	81,000

- 1/ Physical limitations of intake conditions may limit minimum pool elevation.
2/ Not yet determined.

SNETTISHAM POWER STUDIES
PERTINENT DATA - POWERPLANT (Cont'd)

Generation:	<u>Long Lake</u>	<u>Crater Lake</u>
Prime power, KW	25,700	12,100
Primary energy, KWH/yr	225,000,000	106,000,000
Dependable capacity, KW	46,700	23,350
Load Factor	55	52

Turbines:

Type	Francis	Francis
Head, ft:		
Mean operating	848	967
Maximum	895	1022
With standard project flood	901	-
Minimum	721	828
Capability, h/p, at rated head	32,000	
Turbine discharge, cfs, rated head (full gate)	510	<u>1/</u>
Synchronous speed, rpm	514	

Generators:

Rating KVA	22,555
Power Factor	.9
Rating, nameplate, KW	20,300
Rating, overload, (115%), KW	23,350

1/ Design not final

SNETTISHAM DESIGN MEMORANDUM NO. 7
GENERAL DESIGN MEMORANDUM

Appendix A - Power

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| 1. | Letter to Federal Power Commission | 31 January 1964 |
| 2. | Letter from Federal Power Commission | 16 February 1965 |
| 3. | Letter from Federal Power Commission | 19 February 1965 |
| 4. | Letter from U. S. Bureau of Reclamation | 24 February 1965 |
| 5. | Letter from Federal Power Commission | 25 February 1965 |
| 6. | Letter from U. S. Bureau of Reclamation | 29 March 1965 |
| 7. | Letter from U. S. Bureau of Reclamation | 1 April 1965 |
| 8. | Letter from U. S. Bureau of Reclamation | 1 April 1965 |
| 9. | Letter from U. S. Bureau of Reclamation | 23 April 1965 |

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SECTION 1

INTRODUCTION

SNETTISHAM DESIGN MEMORANDUM NO. 7
GENERAL DESIGN MEMORANDUM

APPENDIX A - POWER

SECTION 1 - INTRODUCTION

1-01 PURPOSE:

Appendix A to Design Memorandum No. 7 contains the results of engineering studies, criteria, supplementary data and information supporting the proposed accomplishments of the recommendations given in the main report. Power developments of the Snettisham project, in relation to Juneau's power market, are determined and evaluated. Detailed studies and investigations made to establish the recommended power installation are presented and design criteria, power characteristics, power output and utilization, comparative evaluation of alternative power costs and related economic factors are set forth. Statements regarding project authorization and a resume of prior reports tracing the legislative and research history prior to initiation of general design memorandum studies for the Snettisham project are contained in Section 1 of the main report. Studies to determine the scope of the project were presented in Design Memorandum No. 3.

1-02 GENERAL:

a. Snettisham powerhouse will be located about 28 air miles southeast of Juneau, Alaska, in the Tongass National Forest near the mouth of the Speel River. The project will not affect fish or wildlife resources, there are no requirements for flood control or for maintaining live streams at the lake outlets and recreation potentials are minor; therefore, the design is based solely upon power production and no other function has been assigned. The initial stage will develop the resources of Long Lake. As power demand

in the Juneau area increases, Crater Lake, the second phase of the overall project will be developed to supply the additional power. The complete project will consist of a single powerhouse containing 3 units with an installed capacity of 60,900 KW. Ultimate production will be an annual firm output of 331 million kilowatt-hours per year with an estimated non-firm potential of 23,200,000 kilowatt-hours per year. Juneau area and project location are shown on Plate 1.

b. Basic power market information, load growth data and alternative power values were furnished by either the San Francisco Regional Office of the Federal Power Commission or the Juneau office of the U. S. Bureau of Reclamation. The data were supplemented and updated with actual records of the local distributor for the Juneau area. See Exhibits 2, 4, 5 and 7.

1-03 COORDINATION WITH OTHER AGENCIES:

a. Federal Power Commission - The San Francisco Regional Office of the Federal Power Commission was furnished information on the general status of the project when the authorizing document was written. General agreement was obtained as to the selected site location, normal pool elevation and installed capacity. The Federal Power Commission furnished alternative power values for determination of benefits as stated in Exhibit 2. The FPC also provided the power values used for scoping the project (Exhibit 4) as presented in Design Memorandum No. 3. The question of adequate reserves was also coordinated with the Federal Power Commission (Exhibits 1, 2 and 3). In these letters, requests and explanations are given for reserve capacity, available firm power and alternative value of power.

b. Bureau of Reclamation - In accordance with the March 1962 agreement between the Department of the Interior and the Department of the Army, the Corps of Engineers will design and construct the project, which, upon completion, will be operated and maintained by the Bureau of Reclamation. The Bureau's prime responsibility will be to operate and maintain the reservoirs, penstocks, powerplant and transmission lines. The Bureau also has the responsibility of marketing the power. Close liaison has been maintained with the U. S. Bureau of Reclamation to facilitate joint agreement in project development. The Bureau has provided transmission line criteria, load growth data and general information as well as suggestions for project improvement based on experience gained through operation of Eklutna and other projects.

1-04 PREVIOUS STUDIES AND PROPOSED POWER INSTALLATION:

The power potential of Crater and Long Lakes has been investigated by both private and governmental interests. The first investigations were made in 1913. In 1922, a second investigation was made and in 1928 extensive investigations were carried on by a Mr. George T. Cameron. The U. S. Bureau of Reclamation began field investigations of the project in 1958 and the feasibility report was completed in July 1959. The report titled, "Crater-Long Lake Division, Snettisham Project, Alaska," was published early in 1961 as House Document No. 40, 87th Congress, 1st Session. Power investigations and studies performed to complete the feasibility report recommended a three stage project of three units with a total installed capacity of 48,000 KW. Shortly thereafter the potential industrial load upon which the project feasibility has been based was eliminated by the decision of the Georgia Pacific Alaska Corporation to forego construction of a newsprint mill in the

area. This situation created a substantial change in assumed marketing conditions for the Snettisham project and precipitated a review of the plan of development and economic feasibility. The project was reanalyzed and a reappraisal recommended an increase in installed capacity to 60,000 KW with only minor revisions in other phases of the project; however, the authorizing document and substantiating material called attention to the possibility of project improvement with a dam at the Long Lake outlet. Power and regulation studies conducted by the Corps of Engineers have shown that an eighty-foot dam to elevation 895 will yield maximum net benefits. The presently recommended project, as outlined in Design Memorandum No. 3, is a two stage plan with three equally sized units and a total installed capacity of 60,900 KW. The following tabulation gives pertinent comparative project and powerplant data:

<u>Item</u>	<u>USBR</u>		<u>CofE</u>
	<u>1958</u>	<u>1961</u>	<u>1965</u>
Generators rated Capacity KW	48,000	60,000	60,900
Power Factor	0.9	0.9	0.9
Type of Turbine	Francis	Francis	Francis
Turbine rated Capacity HP	65,760	82,200	83,430
Synchronous speed, RPM	<u>1/</u>	<u>1/</u>	514
<u>Initial Installation</u>			
Units Installed	1	1	2
Skeleton Units	2	2	1
Plant rated Capacity KW	16,000	20,000	40,600
<u>Ultimate Installation</u>			
Units Installed	3	3	3
Plant rated Capacity KW	48,000	60,000	60,900
Overload Capacity KW	<u>1/</u>	<u>1/</u>	70,000
<u>Maximum Pool</u>			
Long Lake	815	815	895
Crater Lake	1022	1022	1022
<u>Minimum Pool</u>			
Long Lake	650	647	723
Crater Lake	820	820	828

1/ Unknown

SECTION 2

POWER MARKET

&

LOAD GROWTH

SECTION 2 - POWER MARKET AND LOAD GROWTH

2-01 POWER MARKET

The market for project power would be domestic, commercial, industrial, and incidental electrical loads in the Juneau power market area. Present Juneau industries are built around governmental operations and the utilization of the natural resources of fish and forests. Gold mines and mills were once major users of power but are now dormant because of adverse economic factors. Establishment of a pulp mill in the Juneau area would increase utilization of timber resources and increase industrial power demand. Justification for the project is based on the future power needs of residential, commercial, small industrial and public agency users. The power market area for the project is limited by two factors: first, adverse topography and heavy forest cover shorten the distance of economical power transmission; and second, electrical demands in Juneau and vicinity are expected to require the entire plant output. The market area to which power could be delivered extends northwest of Juneau to the southern reaches of Lynn Canal, west to include all of Douglas Island, northeast to the Taku River mining district, and southeast to the Tracy Arm mineral zones. Transmission distances in any direction would not exceed 60 miles.

a. Geography - The Juneau trading area extends from Cape Fanshaw to Glacier Bay along the mainland and includes Douglas Island and the north and eastern sides of Admiralty Island. Gastineau Channel, a narrow, salt water channel, separates Douglas Island from the mainland. The City of Douglas, located on Douglas Island, is connected to Juneau by 2 miles of paved highway and a steel bridge across Gastineau Channel.

Mt. Roberts and Mt. Juneau rise abruptly behind Juneau to elevations of 3,819 feet and 3,576 feet, respectively. Beyond these peaks to the east, higher peaks and the Juneau ice fields are encountered. To the west, Douglas Island, Admiralty Island and Chichagof Island intervene between Juneau and the open Pacific Ocean some 80 miles away. To the north, Lynn Canal and Favorite Channel, long fingers of the sea, extends 65 miles from Tee Harbor, near Juneau, to Haines. Haines is the terminus of the only Alaska Highway link to southeast Alaska. To the south, 900 miles of islands, salt water passages and open sea, separates Juneau from Seattle, Washington.

b. History - The northern part of southeastern Alaska was explored in 1794 by Vancouver, the English navigator. The area was nominally under Russian control until 1867 when the United States purchased Alaska. There was little activity in the Gastineau Channel area until the discovery of gold bearing quartz and rich gravels in the Gold Creek Basin in the summer of 1880. Numerous locations were made during the following year, including quartz claims on Douglas Island. Development of the large, low-grade ore bodies was rapid and the Juneau area soon became the most important producer in Alaska. In 1900 the seat of government was officially transferred to Juneau from Sitka, although the actual move was made gradually over several years. The area has experienced a steady growth, with minor fluctuations, since its first settlement. In spite of complete cessation of mining in 1944 growth has continued. Increasing governmental activities are chiefly responsible for this situation. Fishing, lumbering and tourism presently provide an important diversification of the economic base. The surrounding untapped timber resources are expected to play an important part in the area's future economy.

c. Climate - The outstanding features of Juneau's climate are heavy precipitation and mild temperatures. Normal annual precipitation is 90.1 inches, although this varies markedly with little change in altitude or distance. Annual snowfall averages about 100 inches. Winter rains and mild temperatures normally preclude a large accumulation of snow on the ground. Juneau has a July mean temperature of 56°F and a January mean of 29.5°F, the latter about the same as that of Salt Lake City, Utah. The lowest temperature recorded at Juneau was -15°F and the highest was 89°F. At the Juneau airport, 8 miles northwest of the city, the range in temperatures is slightly greater and the normal annual precipitation is about two-thirds that of Juneau. There is an average of 11 days each year of minimum temperature below zero at the airport compared with 2 days at Juneau.

d. Juneau - Juneau was built, for the most part, on the lower slopes of Mt. Juneau and Mt. Roberts, the city making a picturesque appearance against a backdrop of steep mountains. Virtually all of the flat land along the shoreline has been created by a fill of mine tailings. Many of its buildings are supported by wood piling. Although evidence of its early history can still be seen, Juneau is a thriving, modern community. Large office buildings attest to its major source of income. Many new homes, apartments and other buildings are evidence of its continuing growth. Its commercial establishments and professional services can meet practically all requirements commensurate with the needs and it is the primary trading center for the entire area. Utility services are provided by both the city and private companies. The water system is municipally owned. Electric and telephone services are supplied by private corporations. An efficient

voluntary fire department with modern equipment provides fire protection. The Juneau airport, 8 miles northwest of the city, is municipally owned and operated. A paved, 6,500 foot long runway accommodates jets and the four-engine Constellations used by major airlines. Within the city limits a seaplane terminus is the base of operations for local flights to most southeastern Alaska cities and villages.

e. Other communities and facilities - About 50 miles of improved highway, over half of which is paved, provides easy access to Juneau from small communities and rural homesites on the mainland and on Douglas Island. Most of the suburban residents are employed in Juneau. Douglas, the only other incorporated city in that area, is located on Douglas Island. Although it was larger than Juneau early in the century, it is now principally a residential suburb of the capitol. Some commercial services are available in Douglas; however, most shopping is done in Juneau. The City of Douglas maintains its own water and sewer system, streets, etc. Auke Bay is the only other populated area that provides limited commercial and community services. Although small at the present time, it promises to expand rapidly as the Juneau area grows. The Alaska ferry now docks near Auke Bay and is providing increased economic benefits to the entire area due to increased tourist traffic. The entire Juneau area is within the Juneau-Douglas Independent School District. The school facilities are modern and educational standards are rated quite high. A new high school was completed early in 1958 and addition of an adjoining gymnasium and auditorium was recently completed. In addition to the public school system, a parochial grade school is also located in Juneau.

In 1908, the City of Juneau gave the Alaska Electric Light and Power Company a 50-year franchise. Gold Creek hydroelectric plant with an installed capacity of 800 kilowatts was completed in 1914; however, low winter flows precluded firm power. The company continued to supply the winter load with steam engine driven generators until 1917 when the Alaska-Juneau Gold Mining Company (now known as A-J Industries) constructed an 8,000 kilowatt oil-fired steam plant. By 1915 A-J Industries owned 5 hydroelectric plants with an installed capacity of 13,505 kilowatts. Two of these developments, Nugget Creek and Sheep Creek were run-of-river plants and consequently were useless in winter. A 170 foot high concrete arch dam, built in 1915 on Salmon Creek, impounded 19,000 acre-feet of water for use by the two Salmon Creek plants. Total installed capacity of the two plants was 5,600 kilowatts, but usable capacity does not exceed 4,400 kilowatts at present. At the Annex Creek plant, two 1,400 kilowatt units were installed. They are supplied by water from Upper Annex Lake which provides 23,000 acre-feet of storage. A sixth hydroelectric plant owned by the mining company, Treadwell Ditch No. 1 on Douglas Island, had an installed capacity of 1,000 kilowatts but like Nugget Creek and Sheep Creek, had no water supply in winter. The original development of water power at this site was begun in 1882, but generators were not installed until 1922. All these plants were operated at almost full capability until 1942, when mining operations were curtailed. Complete shutdown of the gold mine in 1944 idled a good portion of the generating capacity of the mining company. Until 1957 the industry attempted to maintain mine, mill and powerplants in a reasonable state of repair in anticipation of reopening the mine at some future date. Through disuse, however, waterways of the Nugget Creek and Sheep Creek plants deteriorated

until they are not now in operating condition and the 8,000 KW steam plant is beyond repair. In March of 1962 A-J Industries applied for a 50-year renewal of its Salmon and Annex Creek hydro sites but did not include Sheep Creek, Nugget Creek or Treadwell Ditch. A 25-year license was subsequently issued by FPC in October of 1963 with stipulations requiring rehabilitation of all portions of both sites over a five year period. In 1949 and 1950 the Alaska Electric Light and Power Company replaced the penstock at its Gold Creek plant and installed a new 800 kilowatt hydroelectric unit to supplement the existing installation. No new storage was provided; however, and the units are idle in winter. Only the Gold Creek plant of the Alaska Electric Light and Power Company was built for the exclusive purpose of supplying power to the community. All other capacity may, if needed, be diverted for private industrial uses. Taking cognizance of this fact and its own lack of firm capacity in winter, Alaska Electric Light and Power Company added a 1,250 kilowatt diesel unit at its Gold Creek plant in 1952 and a second identical unit in 1954. In 1963 a 4,120 KW diesel was added, increasing Alaska Light and Power Company firm capacity to 7,390 KW.

The Juneau area with its nearby communities of Douglas and Thane and the rural loads south of the Mendenhall River are served by the Alaska Electric Light and Power Company. Fifty to seventy percent of its total power requirements are purchased from A-J Industries. A-J Industries owns presently installed hydroelectric capacity of 8,400 kilowatts and inoperative hydroelectric and steam capacity of 13,905 kilowatts. The maximum firm generation which these plants could produce without extensive repairs is about 7,200 kilowatts. This dependable capacity is represented by the Salmon Creek and Annex Creek plants. A 23,000 volt transmission system

In 1908, the City of Juneau gave the Alaska Electric Light and Power Company a 50-year franchise. Gold Creek hydroelectric plant with an installed capacity of 800 kilowatts was completed in 1914; however, low winter flows precluded firm power. The company continued to supply the winter load with steam engine driven generators until 1917 when the Alaska-Juneau Gold Mining Company (now known as A-J Industries) constructed an 8,000 kilowatt oil-fired steam plant. By 1915 A-J Industries owned 5 hydroelectric plants with an installed capacity of 13,505 kilowatts. Two of these developments, Nugget Creek and Sheep Creek were run-of-river plants and consequently were useless in winter. A 170 foot high concrete arch dam, built in 1915 on Salmon Creek, impounded 19,000 acre-feet of water for use by the two Salmon Creek plants. Total installed capacity of the two plants was 5,600 kilowatts, but usable capacity does not exceed 4,400 kilowatts at present. At the Annex Creek plant, two 1,400 kilowatt units were installed. They are supplied by water from Upper Annex Lake which provides 23,000 acre-feet of storage. A sixth hydroelectric plant owned by the mining company, Treadwell Ditch No. 1 on Douglas Island, had an installed capacity of 1,000 kilowatts but like Nugget Creek and Sheep Creek, had no water supply in winter. The original development of water power at this site was begun in 1882, but generators were not installed until 1922. All these plants were operated at almost full capability until 1942, when mining operations were curtailed. Complete shutdown of the gold mine in 1944 idled a good portion of the generating capacity of the mining company. Until 1957 the industry attempted to maintain mine, mill and powerplants in a reasonable state of repair in anticipation of reopening the mine at some future date. Through disuse, however, waterways of the Nugget Creek and Sheep Creek plants deteriorated

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interconnects all the mining company's hydroelectric plants with a sub-station near the old steam-generating plant in Juneau. Through the distribution system of the Alaska Electric Light and Power Company, the utility's Gold Creek plant interconnects with the mining company's power system. During January 1965 the peaking demand for all the interconnected plants was reported as 10,000 kilowatts as listed in Table 1. However, the peak demand listed does not include any power retailed by A-J Industries. Table 2 summarizes the above electric developments of the Juneau area.

The Juneau area has sufficient power capacity to meet its present power requirements; however, the usual benefit of low-cost power from hydro sources is not apparent. Wholesale rates are high, necessitating relatively high retail rates. The Alaska Electric Light and Power Company has paid from 15 to 27.5 mills per kilowatt-hour (depending on point of delivery) for energy purchased from A-J Industries. Average for all energy purchased in 1958 was 17.3 mills. Recent hearings before the State Utilities Commission indicate this figure may average 22.5 mills in the future. The Glacier Highway Electric Association has been paying a flat rate of 22.5 mills per kilowatt-hour for energy purchased from Alaska Electric Light and Power Company. The Glacier Highway Electric Association, an R.E.A.-financed cooperative serves an area north of Juneau which includes Auke Bay. They do not have production capability except for a 125 KW standby unit and purchase all of their energy from Alaska Light and Power Company.

2-02 LOAD GROWTH

In arriving at the power forecast (Table 3 and Exhibit 4) for the Juneau market area, the Bureau of Reclamation considered the factors of future high levels of employment, income and probable development of new

local use industries in conjunction with expansion of existing ones. Greatly increased utilization of the vast timber resources of the Juneau area is anticipated within the next few years. As the State Capitol, Juneau's governmental facilities should undergo continued expansion. With new local use industry and more governmental activities, it is estimated that the 1965 population of approximately 11,000 people will double by 1975. Electricity is presently a well-used commodity in Juneau area homes. Furthermore, average customer use is rapidly increasing each year. A drop in retail power rates with availability of project power could be expected to accelerate the rate of increase in use. Assuming project completion, the average customer of 1975 would probably use 8,000 to 10,000 kilowatt-hours a year. The average use of electricity by commercial establishments is relatively low at the present time; however, there is every likelihood that this use will also increase steadily. Competition has become more keen within the past few years and modern lighting, advertising and display are being used more extensively. New commercial buildings are being built in downtown Juneau, as well as new shopping centers in outlying areas, such as Auke Bay and Douglas. Such new construction incorporates modern design and increased use of electricity. As the capitol of the new state governmental activities continues to grow, a steady increase of Federal Government activities can also be expected. With the lower cost in power, street lighting will become a substantial part of the power load. Small local use industry is continuing to grow to supply the increasing needs of the Juneau area. In the past, the growth of the power market for the Juneau area has consistently been underestimated. However, the Bureau of Reclamation has developed a realistic growth rate in their letter of 24 February, Exhibit 4.

2-03 POWER REQUIREMENTS

Power requirements for the Juneau area are shown in Table 3 and Exhibit 4. To conform with the economic studies shown in Design Memo No. 3, the forecast is expressed in terms of fiscal years. Typical daily, weekly, and annual load shapes are presented in Table 4. From the forecast power requirements and the load shapes as developed it is anticipated that the first unit of Long Lake which is scheduled to go on line during Fiscal Year 1970 will meet demand until 1972 at which time the second unit of Long Lake will go on line. The Long Lake phase will meet the demand until 1979 at which time Crater Lake will come on line. The combined projects will then reach maximum capability in 1983 at which time a new source of power must be ready. Power requirements and scheduling of units is shown on Chart 3 and Table 14.

2-04 EXISTING AND FUTURE RESERVES

As pointed out in paragraph 2-01, the present combined capacity of the Alaska Electric Light and Power Company and A-J Industries is about 14,590 KW (Table 2) including about 4,120 KW of reserve. The present peak demand in the Juneau area is about 10,000 KW. Based on the FPC reserve requirements, the Juneau area will no longer be able to meet demand and have an adequate reserve after demand passes 10,500 KW. The Federal Power Commission recommends that the minimum reserve in any system be equal to the capacity of its largest unit. After demand reaches 10,500 KW (1965 or 1966) and providing no other power source has been added to the system, the area demand will encroach on the 4,120 KW reserve. This will mean that Alaska Electric Light and Power Company must either purchase more production capability or take the risk of inadequate reserves until the

Snettisham project is on line. When the Snettisham project comes on line, it is believed that existing reserves in Juneau will be about 10,000 KW of hydro and 10,000 KW of diesel. The power market study shows only that portion of the total power requirements of the area which might reasonably be supplied by project power. Locally available power wholesales at a high rate at the present time although its actual cost of production is unknown. Therefore, present productive capability has not been considered as competitive with the cost of project power. Project power would thus be used to supply the entire load of the area until fully utilized.

SECTION 3

BASES FOR PROJECT
POWER ANALYSIS

SECTION 3 - BASES FOR POWER ANALYSIS

3-01 PROJECT AND SYSTEM OPERATING CONDITIONS:

a. General - The fifty-year economic life of the Snettisham project will span two general power system operational conditions. For purposes of this study, Snettisham development has been separated into two different length periods. The first period contains the full two-stage construction sequence of three unit installation and covers the years when demand is less than plant capability. The load growth forecast indicates that the first period will last approximately thirteen years. The second period includes the balance of the fifty-year economic life with full project capability required. Conditions chosen for each period were assumed to be approximately representative of the entire period.

b. Growth Period - The first construction stage includes all Long Lake features and both of the Long Lake units to meet the Juneau requirements until Fiscal Year 1979. The initial point, with reference to time and system development for power installation studies and evaluation of the recommended power development, has been assumed to be the presently scheduled and earliest completion date, December 1969. The second construction stage will complete the project by adding all features of Crater Lake including a third unit. Full capability of second-stage construction and the project will be reached in approximately 1983. Numerous studies were made of the growth period with variable growth rates and using synthetic inflow patterns as described in paragraph 3-03. A typical summary of operation during the growth period is included as Table 11.

c. Second Period - Although the second period only covers thirty seven years of the economic life, full fifty-year studies of regulation

and power production were made for both Long and Crater Lakes. Period of record flows were used for the detail studies. The first analysis year for Long Lake was assumed to be 1983 and compared to calendar year 1916. In order to maintain compatability of calendar years, the first analysis year for Crater Lake was assumed to be 1980 since the first year of record is 1913. Details of studies during this period are listed on Tables 5 through 10 and shown on Plates 2 through 14.

3-02 STREAMFLOW AND PERIOD OF RECORD

The U. S. Geological Survey collects streamflow records at Long River and Dorothy Creek, near Juneau. Streamflow records used in this study are as follows:

<u>Station</u>	<u>Period of Record</u>	<u>Drainage Area (sq mi)</u>
Crater Creek at Crater Lake Outlet	Feb '13 to Dec '20	11.4
	Jun '23 to Sep '23	
	Jun '24 to Sep '24	
	Jun '27 to Dec '32	
Dorothy Creek near Juneau	Oct '29 to Oct '41	15.2
	Sep '42 to Dec '43	
	Jun '44 to present	
Long River near Juneau	Oct '15 to Sep '24	32.5
	Oct '26 to Dec '26	
	Jun '27 to May '33	
	Oct '51 to present	

These three stations were correlated with each other to develop continuous streamflow data from 1916 to 1964 for Long River and from 1913 to 1964 for Crater Creek. Tables 15 and 16 contain this data. Correlation procedures

described in Leo R. Beards' "Statistical Methods in Hydrology" were used. Details of the methods used in correlating the flows of Dorothy Creek, Long River and Crater Creek are presented in Design Memorandum No. 1, Hydrology. Use of these flows to develop simulated monthly flows is described in detail in Design Memorandum No. 7, main report.

3-03 SIMULATED MONTHLY FLOWS

The present practice in design of water resource projects includes the routing of monthly streamflows based on past records through project facilities in order to determine project accomplishments. A system of synthesizing monthly runoff is used to overcome some of the deficiencies in the present practice of assuming that historical runoff will be repeated. Synthesizing permits a reasonable mathematical determination of expected project benefits insofar as hydrologic factors are concerned. In order to examine the project under more realistic assumptions, forty 50-year periods of monthly records were generated synthetically using the procedures outlined in "Technical Bulletin No. 1, Simulation of Monthly Runoff" published by the Hydrologic Engineering Center of the U. S. Army Corps of Engineers, Sacramento, California, in November 1964. The project was then examined using higher than and lower than normal flows for the next 50-year period. The results of this examination show that should the next 50 years be considerably wetter than normal, the effect on prime power would be negligible. However, should the next 50 years be considerably dryer than the previous 50 years, the effect would be a reduction of up to 3 percent in the amount of prime power that could be generated and that the project could actually go dry if the prime capability was maintained. A frequency curve was derived using the values of the developed forty 50-year periods

and is shown on Chart 4. As may be seen from this chart, there is a 90 percent chance that the average annual volume in acre-feet for the next 50 years will be in the range of 316,500 to 330,400 acre-feet.

3-04 SEDIMENTATION AND EVAPORATION:

a. There are no data presently available concerning the actual sediment loads carried by streams tributary to the Long Lake reservoir. Evaporation studies are detailed in Design Memorandum No. 1 but show no significant losses. These streams are predominately glacial and as such carry significant suspended sediment and bed loads. Measurements of the amount of sediments suspended in the lake were made in March and July of 1965. These measurements indicate an average sediment concentration near the outflow of about 5 parts per million.

b. Because of the lack of actual sediment inflow rates, it is not possible to compute the annual storage depletion by direct methods; however, a reasonable estimate may be made by indirect methods. The Tonsina River, located in the Copper River Basin, has a drainage area with characteristics similar to Long Lake. Forty-seven suspended sediment measurements made during the last few years indicate an average suspended sediment concentration of 113 parts per million. Subtracting the 5 parts per million measured at the outflow and applying this ratio to Long Lakes annual inflow of 324,300 acre-feet yields an annual suspended sediment inflow of 35 acre-feet per year. No data are available concerning movement as bed load; however, for the purpose of this study an average annual contribution of 15 acre-feet has been assumed. Observation of the uniformity of the suspended sediment concentration through the lake indicate that almost all of the inflowing sediments are deposited shortly after reaching the lake water; therefore, it may be

described in Leo R. Beards' "Statistical Methods in Hydrology" were used. Details of the methods used in correlating the flows of Dorothy Creek, Long River and Crater Creek are presented in Design Memorandum No. 1, Hydrology. Use of these flows to develop simulated monthly flows is described in detail in Design Memorandum No. 7, main report.

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assumed that all of the 50 acre-feet per year of storage depletion will occur in live storage.

3-05 CRITICAL PERIODS:

a. General - For the period of record flows the critical periods corresponded very closely with those presented in the authorizing document. However, higher demand required a slightly longer refilling time after minimum pool. The critical period for Long Lake extends from November 2014 to September 2031 which corresponds to period of record years 1949 to 1963 and a recycle period of 1916 to 1918. A proportionately smaller volume of storage in Crater Lake causes extensive pool fluctuations throughout the period of study with the pool approaching minimum several times. The critical period for Crater Lake extended from November 1986 to August 1991 or the water year equivalent of 1919 to 1924.

b. Synthetic Flows - Critical periods for the high and mean synthetic flow patterns did not show significant difference from the period of record flows; however, synthetic low flows showed a critical period much more severe than the period of record. Since the critical period of the period of record is normally used for determination of dependable capacity and primary energy the same procedure was used in this study. The significance of the longer critical period is discussed in paragraph 6-02.

3-06 COST OF ALTERNATIVE POWER:

a. General - The Federal Power Commission provided alternative power values for computation of both benefits and project scoping. Their letters of 19 Feb 65 and 25 Feb 64 are included as Exhibits 3 and 5.

b. Energy - The value of hydroelectric energy at market in Juneau, Alaska, based on cost of alternative power produced at a Juneau steam-

electric plant is estimated to be 6.91 mills per kilowatt-hour. This figure was used in both the benefit and scoping computations developed in Design Memorandum No. 3.

c. Capacity - The computation of benefits as presented herein used a value of \$65.11 per kilowatt-year. However, the scoping analysis presented in Design Memorandum No. 3 used a capacity value of \$29.11 per kilowatt year.

SECTION 4

POWER INSTALLATION STUDIES

SECTION 4 - POWER STUDIES

4-01 INTRODUCTION

a. Since the original 1958 feasibility study by the U. S. Bureau of Reclamation, several major changes have called for reanalysis. Prior to Congressional authorization, the Georgia-Pacific Alaska Company relinquished their rights to development of a newsprint mill in the area, causing a reanalysis of the project in 1961. During the above reanalysis drilling was initiated to determine the feasibility of a dam at Long Lake outlets. Time limitations precluded complete examination and the project was authorized without a dam at Long Lake. Subsequent Corps of Engineer studies prior to final construction considered reduction in unit size to provide firmer reserves combining the reservoirs by raising Long Lake, updating the area's power demand and feasibility of a dam at the Long Lake outlet. Results presented in Design Memorandum No. 3 recommend an eighty foot dam at Long Lake, three equal size units and two stage construction.

b. The information and data presented in this appendix are the results of detailed analyses of the recommended plan. An electronic computer was used to examine maximum project potential, revised tailwater elevations, rule curve operation, synthetic flow patterns, updated load requirements, updated area-storage curves and revised efficiencies.

4-02 SCOPE

a. Preliminary studies were made to examine various alternatives including several pool elevations for both Long and Crater Lakes and possible combined operations by lowering Crater Lake while raising Long Lake. Other variables considered in the preliminary studies were energy requirements, load growth, flow patterns and construction sequences. In

all, eleven alternative plans or a minimum of 33 different conditions were examined to determine benefit-cost comparisons and to provide the basis for the incremental scoping analysis presented in Design Memorandum No. 3.

b. Detailed studies of Long Lake at elevation 895 and further studies of Crater Lake at elevation 1022 are presented herein. The more detailed studies were conducted to determine optimum primary and secondary potential, establish unit design criteria and determine operational limitations. The studies and analyses presented herein considered the latest growth rate forecast (Exhibit 4 and Table 3), synthetic flows and the variable conditions previously mentioned.

4-03 PRELIMINARY STUDIES:

a. Studies were made for eleven alternative plans and were complete enough to provide unit design criteria for estimating unit sizes and costs. The alternatives studied covered possible operation of Long Lake between elevations 815 and 1022, Crater Lake between elevations 1022 and 1220 and the two lakes as a single reservoir between elevations 860 and 1022. Results of preliminary studies are shown on Plates 20 to 22 and listed on Tables 12 and 13.

b. Long Lake was examined at four maximum pool elevations (815, 900, 950 and 1020) with energy production loading that varied from 192.6 million kilowatt-hours per year to 263.0 million kilowatt-hours per year. The rise in pool elevation allowed an increase in active storage from 193,800 acre-feet to a maximum of 350,000 acre-feet at the upper pool. The availability of this increase in storage allowed an average annual reduction in wasted water from 7,000 acre-feet per year to 1,850 acre-feet per year. For maximum pool elevation of 950, the minimum pool was 797 or 18 feet below the

natural lake elevation. In this case only a slight lowering of the lake would have sufficed for building the intake structure. Design head for the units varied from approximately 768 up to 973 at the upper pool. Tables 12 and 13 list other data.

c. Crater Lake was examined at three maximum pool elevations (1022, 1122 and 1222) with energy production that varied from 104.8 million kilowatt-hours per year to 133.0 million kilowatt-hours per year. Wasted water or water not usable for power production was reduced from an annual average of 4910 acre-feet at elevation 1022 to an average of 1500 acre-feet at elevation 1222. Due to more efficient water use, the maximum drawdown was reduced from 175 feet at the lower elevation to 115 feet at elevation 1222. Tables 12 and 13 list other data.

d. The two lakes operating as a common reservoir were examined at pool elevations of 860, 900, 950 and 1020. The primary energy production varied from 294.2 million kilowatt-hours per year to a maximum of 381.3 million kilowatt-hours per year. Active storage increased from 264,000 acre-feet at elevation 860 to 348,000 acre-feet at elevation 1022. An overall summary of results (Table 13) lists the selected studies and also compares the totals of the project document plan with the combined operation at elevation 860. The decrease in waste flow of an average annual volume of 7,400 acre-feet is realized by combined operation, and is reflected in increased production of 14.2 million kilowatt-hours per year of prime energy. It should be noted that under no condition of combined operation would it be possible to lower the Crater Lake intake structure much below elevation 750 because of reduction in storage and possible future sedimentation difficulties. A small increment of storage would therefore be dead

storage and would not be available for combined pool operation.

4-04 FINAL STUDIES:

a. Final studies were made to determine the optimum primary power potential of the project. With the aid of an electronic computer, synthetic flows were developed to study the effect of higher and lower than period of record flows during the critical periods. Final studies verified the primary energy capability, size and number of units and powerhouse criteria. Samples of computations are illustrated on Plates 6 through 14. Annual summaries are shown on Tables 6 through 9.

b. Long Lake was studied with a dam to elevation 895. A firm energy output of 225 million kilowatt-hours per year and a nonfirm output of 10.8 million kilowatt-hours per year was the energy potential resulting from the studies. With the dam, Long Lake will provide an active storage of 252,000 acre-feet with the pool fluctuating from elevation 895 to a minimum elevation of 720. On the basis of two 20,300 kilowatt units for the Long Lake phase and the power forecast for the Juneau area, studies indicated that the Long Lake phase of the project should go on line by 1970 to meet the demand. Growth studies showed that the Long Lake phase can be scheduled to meet demand until 1979 thereby allowing deferment of the construction costs of the Crater Lake phase. In the final studies, a rule curve analysis (Charts 1 & 2) was devised to provide optimum production of nonfirm power by allowing the pools to be drawn to predetermined elevations during the low flow winter months.

c. Using the natural lake elevation as the normal pool, the Crater Lake phase of the project can provide a firm energy output of 106 million kilowatt-hours per year and a nonfirm energy output of 12.4 million

kilowatt-hours per year. The pool will fluctuate between elevations 1022 and 828 providing an active storage of 81,000 acre-feet. If at a future date, a dam at the outlet of Crater Lake is found to be feasible, a reservoir would provide desirable additional storage and a greater firm energy output would be realized. Rule curve analysis allows optimum use of the nonfirm power by allowing the pools to be drawn down to predetermined elevations during the low flow winter months. The results of rule curve operation indicate that Crater Lake can be drawn down as low as elevation 800; however, since physical conditions may dictate the elevation of the intake invert, the minimum used in determining energy potential was established without application of the rule curve.

4-05 OPERATING CONDITIONS AND BASIC CRITERIA:

a. Reservoir Requirements - The Snettisham project is unique in its remote location. It will be used for the production of power only. The normal allowance for fish, flood control, navigation, irrigation or water supply for downstream requirements have not been considered in its justification due to this remoteness. Operation of the reservoirs will be determined by the load shape, demand and water supply only.

b. All available historical runoff records were used in the regulation and power demand studies. Extensive correlation was also made with Dorothy Creek, a similar stream and located near the proposed project. A maximum of 49 years of record were available for study at this time. For Long Lake, the period from 1916 through 1963 was used, while for Crater Lake, the period from 1913 through 1963 was used. For examining the lakes operating as a combined unit, the 1916 through 1963 flow records were combined resulting in a 46-year period for the combined lakes. In

conducting the final regulation and power studies, it was assumed that the first year of water record would be considered as 1980 and the studies ran from 1980. Tidal fluctuations in the project are extreme, with a maximum range of 28 feet. The project document assumed a uniform tailwater elevation of 2 feet. The present studies assumed an outlet sill elevation of -1.0 msl and average tide conditions.

c. With two exceptions, terms used in this power appendix are based upon definitions as found in the "Glossary of Terms" 1949 edition, compiled under the supervision of the Federal Power Commission. The two exceptions are the terms "Spill" and "Waste" which are defined as follows: "Spill" is the discharge passed through the turbines for the production of secondary energy. "Waste" is the discharge over the spillway which cannot be used for production of power due to lack of machine capacity.

4-06 POWER VALUES AND TRANSMISSION LOSSES:

a. Power values were furnished by the Federal Power Commission in their letter of 25 February 1964 (Exhibit 5) and were based on a privately financed 49,500 kilowatt oil-fired steam plant located at tidewater in Juneau. The value of power was determined to be \$65.11 per kilowatt-year for net dependable capacity and 6.91 mills per kilowatt-hour for net primary energy.

b. Transmission losses from the project to the Juneau area were assumed to be 6 percent overall.

4-07 ENERGY DETERMINATIONS:

a. General - The basis for determination of power demand or output was the estimated total requirement of the Juneau area, including all line losses and other losses attributal to regular power use. Studies during

the 50-year period were based upon a constant demand throughout the period of study and it was assumed that machine capacity would be provided to meet the demand both base and peaking.

b. Regulation Studies - The use of a special computer program facilitated examination of many different alternatives. Different power demands for regulation as well as different pool elevations for each alternative were considered. The program takes into consideration variation in head, operational efficiencies of the units, varying hydraulic losses, rule curve operation and discriminates between usable spill and waste dependent upon the assumed machine capacity of the alternative being studied. In order to initiate a study of any one condition, several preliminary computations and basic assumptions were made as follows:

(1) It was assumed that the pool was full in late summer but by January 1 was drawn down by a volume determined from the preliminary computations.

(2) A uniform annual energy load was determined using mass curves, assumed water losses, assumed average head, and the established average annual inflow. A minimum of 3 annual energy loads was used to develop curves for each condition being studied.

(3) The annual energy load was then distributed by months as listed in Table 4.

(4) Installed capacity was determined as outlined in 4-07g.

(5) Machine capacity at maximum pool was determined assuming 15 percent overload at the appropriate reduced efficiency.

(6) A minimum allowable pool elevation was chosen at approximately 70 percent of design head. For the Snettisham project, this represented a

drawdown greater than 200 feet in all cases. None of the alternatives studied could be drawn down this far and recover full pool without reduction in demand.

c. Efficiencies used at various pool elevations were based on EM 1110-2-1701 and USBR Engineering Monograph No. 20. Generator efficiency was considered to be 95 percent while turbine efficiency varied from below 80 percent to a maximum of 91.4 percent, depending upon the operational range or percent of design head. Hydraulic efficiency was based on percent of gross head and varied from 96 percent to 98 percent of design head. Plate No. 17 shows the various efficiencies. Overall efficiency varies from 76.0 to 84.8 percent depending upon percent of design head.

$$\text{The basic power formula is: } KW = \frac{Q H E_o}{11.8}$$

Where KW = kilowatts

Q = flow in cfs

H = gross head (difference between pool and tailwater elevations)

E_o = overall efficiency

Dividing efficiencies by the constant (11.8) the formula reduces

$$\text{to: } KW = C Q H$$

Where C varies from 0.065 to 0.0718 and is dependent upon percent of design head as shown on Plate 17.

d. Power Demand - Power demand was based on preliminary estimates of the potential possible at each pool elevation for each alternative considered. For preliminary studies a minimum of three different loadings or power demand sequences were placed on each pool elevation and each regulation run. Plates 20 through 22 show typical results and the curves resulting

from plotting various items such as minimum pool elevation, maximum drawdown, total potential revenue, average pool elevation and others for the various conditions and alternatives.

e. Load Factors - Load factors used in these studies were the same as presented in the project document. Information received from the Juneau area during the last 3 years indicates that the load factor continues to be near the 55 percent previously established.

f. Dependable Capacity - The dependable capacity for each alternative studied was based upon the minimum pool reached in each regulation study and considered the reduction in overall plant efficiency but allowed operation up to full gate. Tidal effect was considered during minimum pool conditions; however, no significant reduction in dependable capacity is expected due to conservative estimates of hydraulic losses used in the studies, excess capacity designed into the unit and the nominal period of time that such conditions would prevail.

g. Installed Capacity - The machine capability or installed capacity for each alternative studied was based on the annual load shape and the load factor. The capabilities used for preliminary studies are not necessarily the final choice of installed capacity for the project. They were used only for comparative purposes.

Installed capacity was computed as follows:

$$KW = \frac{\text{Annual load}}{8760 \times 0.55 \times 115}$$

Total machine capability was determined by using the following

formula:
$$KW = \frac{\text{Annual load}}{8760 \times 0.55}$$

Where Annual load = number of KWH required per year

115 = allowable machine loading in percent

8760 = number of hours per year

0.55 = assumed load factor

h. Electronic Computer Use - A program was developed for use with an electronic computer and was utilized in making all basic power output determinations for the range of development conditions covered. Use of this computer program permits simulation of actual powerplant operation on an average basis. Various loads were placed in the computer as were inflows for the period of record. The machine computes outflow, average head, pool elevations, end of the month storage, primary energy, secondary energy and waste. The program is designed to simulate monthly operation of up to 10 projects. To obtain secondary power during the winter operation, a rule curve routine was developed. Pilot studies were used in the preliminary investigations and to arrive at the Long Lake dam height elevation of 895. Subsequent modifications have produced the final studies as listed in Tables 5 through 9.

4-08 INSTALLATION SCHEDULE:

The total project has been scheduled to meet the power forecast as determined in the Bureau of Reclamation forecast (Table 3) and yet defer the Crater Lake phase cost for several years. This schedule will place the first unit of the Long Lake phase on line by 1970 in time to meet the increasing power demand of the Juneau area. The second unit of Long Lake must be on line no later than 1972. The Long Lake phase will then meet demand until 1979 when the Crater Lake phase must be on line.

4-09 SNETTISHAM UNIT CRITERIA:

The basic decision to make all unit sizes equal (Exhibit 6) determined

the unit size to be installed at 20,300 KW. This size will give the unit a dependable capacity with overload of 23,350 KW. In the case of Long Lake, the maximum pool elevation will be 895, the minimum pool elevation 720 and with an average pool elevation 843. Maximum discharge through the Long Lake phase will be 980 cfs. For Crater Lake, the maximum pool elevation is 1022, the minimum pool elevation is 828 and with an average pool elevation 967. Maximum discharge through the Crater Lake phase under normal operating conditions is 410 cfs. Minimum pools may be dependent upon the physical limitations of the tunnel intake structures. Provisions of interconnections to permit inspection of either penstock while two units remain in operation requires a maximum discharge of 530 cfs from Crater Lake. This flow would be required only for peaking (early summer peak - 34,000 KW) during closure of Long Lake penstock. Although the tidal fluctuation in Speel Inlet is about 28 feet, a minimum tailwater has been established by placing a tailrace sill at elevation -1.0 msl. Maximum tailwater elevation corresponds to maximum tide or about 16.0 feet msl. The above unit criteria, as summarized in Table 11, are based upon maximum capability derived from FPC criteria and actual load shapes shown in Table 4.

4-10 PROJECT COSTS:

Total cost of the recommended project plan is estimated at \$59,760,000. The first phase, development of Long Lake, is estimated to cost \$45,338,000. The second phase, development of Crater Lake, is estimated to cost \$14,422,000. Based on data furnished by the Bureau of Reclamation, annual operation, maintenance and interim replacement costs for the Long Lake phase are estimated to total \$475,000 and it is estimated that ultimate addition of the Crater Lake phase will increase this requirement by \$8,000 for a project total

O M & R cost of \$483,000 per year. Computations are based on an 8-year construction period for the Long Lake phase and a 7-year construction period for the Crater Lake phase. Present projections of power load growth in the area show that power from the Crater Lake phase of the project will be needed seven years after construction of the Long Lake phase is completed. Costs of the Crater Lake phase have been present-worthed accordingly. An interest rate of 3-1/8 percent has been used to establish the capital investment and to compute the annual costs of interest and amortizations. On these bases the average annual cost of the project for a 50-year life has been computed as follows:

Long Lake Phase (Elev. 895)

Construction Cost	\$40,300,000
Interest During Construction	
(3-1/8 percent - 8 years)	<u>5,037,000</u>
Total Investment	\$45,337,000
Annual Costs	
Interest and Amortization	
(45,337,000 x 0.03979)	\$ 1,804,000
Operation, Maintenance and Replacements	<u>475,000</u>
Average Annual Costs	\$ 2,279,000

Crater Lake Phase (Elev. 1022)

Construction Cost	\$13,000,000
Interest During Construction	
(3-1/8 percent - 7 years)	<u>1,422,000</u>
Total Investment	\$14,422,000
Annual Costs	

Interest and Amortization

(14,422,000 x 0.03979) \$ 574,000

Operation, Maintenance and Replacements 8,000

Average Annual Costs \$ 582,000

Average Annual Costs for Complete Project

Long Lake \$2,279,000

Crater Lake Present-Worthed to Completion

Date of Long Lake (582,000 x 0.80622) 469,000

Total Annual Costs (50-year life) \$2,748,000

Application of the same procedures to computations based on a 100-year project life results in computed average annual costs, as follows:

Long Lake Phase	\$1,966,000
Crater Lake Phase	<u>387,000</u>
Complete Project	\$2,347,000

4-11 POWER BENEFITS:

The Snettisham project has the sole purpose of producing power for the Juneau area. All benefit determinations and comparisons reflect credit for prime power only. Energy and capacity creditable to this specific power plant during its economic life were based on criteria shown in paragraph 4.06 and as furnished by the Federal Power Commission in their letter of 25 February 1964. Based on a privately-financed, 49,500 KW, oil-fired steam plant, the at-market value of power was determined to be \$65.11 per kilowatt-year for net dependable capacity and 6.91 mills per kilowatt-hour for net primary energy. At-site dependable capacity for the Long Lake phase of the recommended plan was determined to be 46,700 kilowatts. The decision to make all generating units the same size results in provision

of 23,350 kilowatts of at-site dependable capacity for the Crater Lake phase. At-site primary energy was computed to be 225,000,000 kilowatt-hours annually for Long Lake and 106,000,000 kilowatt-hours annually for Crater Lake. To derive net or at-market values, station use and transmission line losses were assumed to total 6 percent for both capacity and energy. Benefits were not assigned for secondary energy. Benefit computations for the recommended project plan are summarized as follows:

ANNUAL BENEFITS

Long Lake @ 895

Dependable Capacity Value ($46,700 \times 65.11 \times 0.94$)	=	\$2,858,000
Energy Value ($225,000,000 \times 0.00691 \times 0.94$)	=	<u>1,461,000</u>
Annual Benefits		\$4,319,000

Crater Lake @ 1022

Dependable Capacity Value ($23,350 \times 65.11 \times 0.94$)	=	\$1,429,000
Energy Value ($106,000,000 \times 0.00691 \times 0.94$)	=	<u>689,000</u>
Annual Benefits		\$2,118,000

Complete Project

Long Lake	=	\$4,319,000
Crater Lake, present worth to completion date of Long Lake ($\$2,109,000 \times 0.80622$)	=	<u>1,707,000</u>
Total Average Annual Benefits		\$6,026,000

4-12 ECONOMIC ANALYSIS:

The economic relationship resulting from cost and benefit evaluation are summarized as follows:

<u>Project Phase</u>	<u>50-Year Life</u>			<u>100-Year Life</u>		
	<u>Ann. Cost</u>	<u>Ann. Benefit</u>	<u>B/C</u>	<u>Ann. Cost</u>	<u>Ann. Benefit</u>	<u>B/C</u>
Long Lake	\$2,279,000	\$4,319,000	1.90	\$1,966,000	\$4,319,000	2.20
Complete Project	2,748,000	\$6,029,000	2.19	2,347,000	\$6,029,000	2.57

The above analysis did not recognize the value of secondary energy; therefore, benefits are lower than maximum. Assuming the minimum secondary energy production, as listed in Tables 6 through 9, average annual benefits and B/C ratios would then be:

<u>Project Phase</u>	<u>50-Year Life</u>		<u>100-Year Life</u>	
	<u>Annual Benefit</u>	<u>B/C</u>	<u>Annual Benefit</u>	<u>B/C</u>
Long Lake	\$4,365,000	1.92	\$4,365,000	2.22
Complete Project	\$6,119,000	2.23	\$6,119,000	2.61

The value of secondary energy is assumed to be 5 mills/KWH. Cost vs benefit curves are relatively flat in the range of dam height proposed. An incremental scoping analysis was developed in Design Memorandum No. 3 to determine the economical dam height. Federally financed power values of \$29.29/KW for capacity and \$0.00691/KWH for primary energy were used in the scoping analysis. In making the economic analysis both 50-year and 100-year life was computed.

4-13 DISCUSSION:

During the study period new load growth rates, the decision to make equal sized units and greater refinement of the computer studies have resulted in a 3-unit powerplant with a dependable capacity of 70,000 KW capable of delivering 331 million kilowatt-hours of firm power and about 20.8 million kilowatt-hours of secondary energy at site annually. An

additional feature of rule curve operation was added to the computer program to provide the most efficient use of the secondary energy by allowing the pools to be drawn to predetermined elevations throughout the year. The two-stage development is scheduled to place an initial unit on line at the end of 1969 and a second unit on line by 1972. The second phase will place the third unit on line in 1980 and demand on the project is expected to reach full capacity in 1983.

SECTION 5

POWER FACILITIES

SECTION 5 - POWER FACILITIES

5-01 GENERAL

Preparation of powerhouse plans and estimates in this appendix to the General Design Memorandum and the following text material concerning the power facilities has been accomplished by the HydroElectric Design Branch of the North Pacific Division. Two proposed plans are presented herein, the conventional indoor-type powerhouse and an underground plant. These plans cannot be considered final but do afford a sound basis for cost determination between the two types of plants. For cost comparison purposes the plants were compared for their ultimate capacity of three units with a vertical Francis-type turbine. The number of units and kilowatt capacity of initial and ultimate installations are considered firm; however, further study will be required to select final type, size and setting of turbines and unit spacing. Location of the powerplant with respect to other features of the project is shown on Plate 1.

5-02 POWERHOUSE

a. Conventional The powerhouse will be reinforced concrete indoor type structure equipped with a 75-ton bridge crane to serve for erection and maintenance of the units. The powerhouse initially will consist of two generator bays, erection bay and skeleton bay, all complete with superstructure. Units one and two will be served by the penstock from Long Lake and unit three will be served by the penstock from Crater Lake. The erection bay is of sufficient width and length to provide adequate space for an unloading area and a rotor erection area at elevation 23.0. The generator bay is of sufficient width and length to adequately house the generators and turbines and provide necessary clearances. A 3-ton monorail attached to the

downstream exterior face of the powerhouse wall, complete with lifting beam and sling, will be used for handling the draft tube bulkhead. Space allocations and location of powerhouse equipment are shown on Plates 23 through 27.

b. Underground Space allocation and location of powerhouse equipment for the underground plant are shown on Plates 28 through 32. The generator room width was increased to include the draft tube slots and spherical valves so that the valves and draft tube bulkheads could be handled by the bridge crane. The auxiliary and control area shown in the conventional plant to be upstream from the generator room was moved to the erection bay in the underground plant. This was done to reduce the width of the cavity excavated for the underground plant. No provisions will be made in the underground plant for a maintenance area or electrical shop. The portions of the wall above the erection floor level and balcony level will be exposed rock, which in the event of loose rock, will be covered with chain link fabric. No structural reinforcement of the roof is contemplated, however a metal arch roof, acoustically treated, will be used for protection from falling of rock spalls and moisture. Access to the underground plant will be by tunnel approximately 17 feet wide and 15 feet high. Ventilation air will be taken from the powerhouse access tunnel. Exhaust air will be carried to atmosphere either by a duct within the powerhouse access tunnel or a separate exhaust tunnel.

5-03 TURBINES, GENERATORS AND ELECTRICAL EQUIPMENT

a. Criteria In accordance with the results of studies presented in this appendix each unit will be designed to provide a dependable capacity of 23,350 KW. The generators will be designed to produce this load at 115

percent generator nameplate rating at 0.9 power factor. Other criteria are listed in Table 11.

b. Turbines The turbines will be of the vertical shaft, Francis-type with steel spiral cases and concrete elbow draft tubes. Each unit will be designed to produce a guaranteed output of 32,000 HP (23,350 KW) at minimum pool elevation. Best efficiency will be achieved at the average pool elevation. Guard valves will be provided upstream from each unit for emergency shutdown and maintenance. A valved connection will be provided between the Crater Lake unit and the adjacent Long Lake unit to assure the operation of two units when the Long Lake waterway is shut down for inspection or maintenance. Preliminary studies indicate that pressure regulators are required for the conventional powerhouse to provide acceptable speed regulation without the need for abnormal amounts of generator WR^2 . The cost of the regulators has been included in the estimate for the conventional plant. The final selection of the type and size of guard valves and pressure regulators will be the subject of further study in the Powerhouse Preliminary Design Report.

c. Electrical Features-Conventional Plant Subject to completion of detailed studies, the anticipated layout of the electrical features of the conventional powerhouse will conform to normal accepted practice for such plants. A common auxiliary and control area is planned at the turbine floor elevation upstream from the units, where station switchgear, switchboards, control equipment and other electrical equipment will be located as indicated on Plate 25. The vertical generators will be rated 22,600 KVA, 0.9 PF, 13.8 KV at 60°C. temperature rise with capability of operating continuously at 115 percent rated KVA. Detailed electrical and mechanical

characteristics will be as determined by additional studies; however, higher than normal WR^2 will probably be required because of the long penstock. Individual three phase power transformers for each unit are presently planned to insure having one unit available initially for service at all times. These transformers will be rated 26,000 KVA, 13.8-138 KV Gnd Y, type FOA, and are tentatively planned to be located at a 138 KV switchyard near the powerhouse. Low voltage connections would be via aerial 15 KV cables from the powerhouse. Design of the control features will be as required for eventual complete remote control of the plant from Juneau, Alaska, via carrier current. Anticipated station service power scheme is a 13.8 KV-480 volt, double ended substation supplied from the terminals of the two generators with an independently driven emergency generator capable of supplying essential loads only.

d. Electrical Features-Underground Plant - The electrical features of the underground plant are essentially the same as the conventional plant. The location of this equipment will be as indicated on Plate 31. The increased distance from the powerhouse to the switchyard may result in the transformers being located closer to the generators with 138 KV cable connections to the switchyard depending on the results of future studies.

5-04 POWER PENSTOCKS

Although the data presented in Shettisham Project Design Memorandum No. 3 indicates diameters of 6 and 7 feet for the Crater Lake and Long Lake penstocks respectively, future studies will be presented in the Preliminary Design Report on the Powerhouse to justify the most economical size.

SECTION 6

POWER OUTPUT

&

UTILIZATION

SECTION 6 - POWER OUTPUT AND UTILIZATION

6-01 POWER POTENTIAL:

The Long Lake and Crater Lake phases of the Snettisham project are designed to operate at maximum gross heads of 895 and 1022 feet, respectively, with average annual inflows of 448 cfs and 203 cfs. The project will provide potential energy of about 331 million kilowatt-hours annually depending upon operation. Utilization of potential generation will be solely in the Juneau area. Final studies indicate that maximum revenue will be derived from the most efficient production of secondary energy after the project reaches maximum capabilities. Integrated operation of this system may find that the Crater Lake phase with its greater waste should be operated primarily as a base load plant and Long Lake with its greater storage capabilities be operated as a peaking plant. Area power requirements and resource studies indicate that initial generation will be usable as soon as it can be made available.

6-02 CONDITIONS OF OPERATION:

a. General - Since power production is the sole objective of the Snettisham project, it will be operated as efficiently as possible while producing a maximum number of saleable KWH. Operation or reservoir regulation will be characterized by two general conditions; one will be the winter low stream flow season of October through May, while the other will cover the summer and fall high runoff season of May to October. Maximum energy output and minimum waste will be dependent upon forecasting ability and operational procedures that combine the optimum use of available water and machines. Operational experience in conjunction with improved data accumulation will result in improved rule curves and regulation criteria.

b. Rule Curve Operation - Examination of regulation studies provided monthly minimum pool allowances which were used to develop rule curves (Charts 1 and 2). With rule curve operation, secondary energy can be produced throughout the year whenever surplus water is available. Use of these preliminary rule curves increased production of secondary energy while reducing waste flow. The refined rule curves that will be developed will result in optimum use of available water. It must be assumed that plant factors will not be the same as the Juneau load factor or even show similarity between Long and Crater Lake operations.

c. Synthetic Flows - Synthetic flows were developed and compared to period of record flows in order to determine the probability of major changes in critical periods. The studies of high and mean synthetic flows indicated only minor differences in inflow patterns and no change in energy production potential. On the other hand the study of low synthetic flows indicated that there is a fifty percent chance of experiencing a more severe critical period than occurred during the period of record. Although the critical period may be more severe, the degree of severity is important since a three percent reduction in primary energy would have an occurrence probability of only one in five hundred. Therefore, even though a more severe critical period may occur in the next fifty years, the possibility of significant reduction in primary energy is minimal and of less importance than other factors at this time.

d. Low Stream Flow Season - The period of low inflow to both Long and Crater Lakes occurs during the winter months, generally October to May. Although heavy precipitation, in the form of rain and snow, occurs during this period, it is retained in the snowpack and very little runoff is

available for inflow. Since this is also the period of greatest power demand, the pools will invariably be drawn down with minimum pool occurring in early May. It is during this period that accurate forecasting and judicious production of secondary energy will reduce or eliminate waste during the summer months.

e. High Stream Flow Season - During the summer months of May through October, the runoff not only includes snowmelt but relatively heavy direct precipitation in summer and fall rains. During this period the demand is at its annual minimum and the pools will refill. Optimum operation during this period would be to fill the pools without wasting water.

f. Growth Years - During the anticipated thirteen-year growth period, there will be excess water available for production of secondary energy; however, demand is not expected to be present. The resulting apparent waste was not used in computing the average annual waste listed for either Long or Crater Lakes. The fact that waste is inevitable makes both hydraulic or electrical losses meaningless during the growth period.

6-03 POWER PRODUCTION:

The detail studies have shown that optimum economical development of the available power potential of the Snettisham project required that Long and Crater Lakes be operated as separate reservoirs connected to a common powerhouse by pressure tunnels and penstocks. Generally water from the Long Lake phase will be used to operate two units; however, provision has been made to operate two units with water from Crater Lake during periodic inspection of the Long Lake tunnel and penstock. Both phases will be operated to provide maximum firm energy and to produce secondary energy with the remaining flow up to unit capacity. Operation of the project will provide

basic criteria to further refine the rule curves and hence provide optimum operation.

EXHIBITS

COPY

U. S. ARMY ENGINEER DISTRICT, ALASKA

CORPS OF ENGINEERS

P. O. Box 7002

Anchorage, Alaska 99501

ADDRESS REPLY TO
THE DISTRICT ENGINEER
(NOT TO INDIVIDUALS)

REFER TO FILE NO.

NPAEN-PR-R

31 January 1964

Mr. M. Boyd Austin
Regional Director
Federal Power Commission
555 Battery Street
Room 415
San Francisco, California 94111

Dear Mr. Austin:

This office is presently preparing the General Design Memorandums for the Snettisham Project near Juneau, Alaska, (House Document No. 40, 87th Congress). In connection with the Hydropower Capacity studies of this memorandum, it would be appreciated if this office could be apprised of your opinions, suggestions and/or comments regarding the following major questions.

- a. What sort of reserve would be required for an isolated system of this type?
- b. What portion of the available prime power could be considered as firm with no reserve and a single line (115,000 v, 38.7 miles) from site to market, especially in view of the terrain in the Juneau area?
- c. What would be the alternative value of power for a project of this size in this area?

Sincerely yours,

/s/t/K. T. SAWYER
Colonel, Corps of Engineers
District Engineer

EXHIBIT 1.1

FEDERAL POWER COMMISSION

REGIONAL OFFICE

555 BATTERY STREET, ROOM 415

SAN FRANCISCO, CALIF. 94111

February 16, 1965

AIRMAIL

Colonel Clare F. Farley
District Engineer
U.S. Army Engineer District, Alaska
Corps of Engineers
Post Office Box 7002
Anchorage, Alaska 99501

Dear Colonel Farley:

Mr. Mark Nelson of your division office in Portland has discussed with me the implications of our letter of February 25, 1964, concerning the Snettisham Project, and has asked that I provide some amplification of the comments in line with our telephone conversations.

The statements made in our letter of February 25, 1964, concerning the maintenance of firm capability are, we think, self-evident and need no further explanation as such. However, the buyer and seller may agree on some measure of reliability which is less than 100 percent firm. Thus, the buyer may be willing to drop some of his load in case of an outage that is not covered by reserves. In some instances, a load may be served which is not greatly inconvenienced by an interruption of power supply. At other times, there will be inconvenience, but the purchaser of service takes a calculated risk that it will not happen, in order to obtain service at a lower cost. Such conditions of risk and price are subject to some negotiation between buyer and seller, and as such are not of direct concern to the Federal Power Commission so long as the negotiations are carried on with the full knowledge of both parties as to the risks involved and savings resulting therefrom.

These statements would also apply to the transmission of the power to load center. Transmission lines and substations are subject to failures of various kinds. A single line is vulnerable to any and all of these potential failures. When a failure occurs the power supply over that line is entirely lost. A double circuit even on the same set of towers is less vulnerable and more reliable than a single circuit, since many of the failures could affect only one circuit. There would still remain the chances

EXHIBIT 2.1

Colonel Clare F. Farley

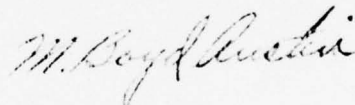
-2-

February 16, 1965

of a tower failure or other failures which would affect both circuits, so it would still not be entirely firm but could be considered as being within reasonable bounds of reliability. Two circuits on separate structures but within the same right of way would be considered more reliable than two separate circuits on the same structure, but less reliable than separate circuits on separated alignments.

The whole tenor of our discussion has to do with what might be termed the degree of reliability. Our letter of February 25, 1964 was couched in terms of 100 percent reliability. Such reliability may not be desirable from an economic point of view. Assured firm power (100 percent) might cost half again as much as power with a somewhat lesser degree of reliability. The purchaser of power might be better satisfied and more willing to purchase the less reliable power supply. Our concern is that the alternatives be fully considered and the degree of reliability recognized in whatever plan is developed, and that the prospective purchasers are aware of any lack of reliability that is involved.

Sincerely yours,



M. Boyd Austin
Regional Engineer

EXHIBIT 2.2

FEDERAL POWER COMMISSION

REGIONAL OFFICE

555 BATTERY STREET, ROOM 415

SAN FRANCISCO, CALIF.

94111

February 19, 1965

AIR MAIL

Colonel Clare F. Farley
District Engineer
U. S. Army Engineer District, Alaska
P. O. Box 7002
Anchorage, Alaska 99051

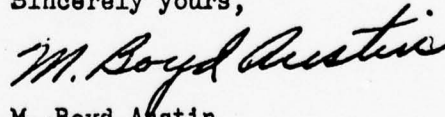
Dear Colonel Farley:

Reference is made to the February 17 telephone request from Messrs. Lambert and Wade for a value of power based on federal financing for use in the Corps' studies of the Snettisham Project.

The value of hydroelectric power at-market in Juneau, Alaska, based on cost of alternative power produced by a federally financed steam-electric plant, is estimated to be \$29.29 per kilowatt-year for dependable capacity plus 6.91 mills per kilowatt-hour. This estimate excludes taxes and is based on cost of money at 3.125 percent. As in the estimated value furnished in 1960 for a privately financed plant, the steam-electric plant assumed for this value has 3 units with a total capacity of 49,500 kilowatts, and is located at Juneau. The cost of fuel is based on bunker oil at 50.4 cents per million Btu.

Please advise if we can be of further assistance.

Sincerely yours,



M. Boyd Austin
Regional Engineer

EXHIBIT 3.1



UNITED STATES
DEPARTMENT OF THE INTERIOR

BUREAU OF RECLAMATION

ALASKA DISTRICT HEADQUARTERS

P. O. BOX 2567, JUNEAU, ALASKA 99801

IN REPLY
REFER TO: 620

February 24, 1965

AIRMAIL

District Engineer
U.S. Army Engineer District, Alaska
P.O. Box 7002
Anchorage, Alaska 99501

Dear Sir:

Enclosed is a tabulation of hourly generation values for the Juneau area during the period from November 18, through 25, 1964 as obtained from the Alaska Electric Light and Power Company and Alaska-Juneau Industries, Inc. These values were taken during what was considered a normal fall week and should be representative of the load pattern during the season.

Also enclosed is an updated forecast of the Juneau area load requirements for use with the Snettisham Project studies. This forecast was prepared in conjunction with a representative of the Alaska Electric Light and Power Company, the local distributor in the area. The forecast has been revised to include the trend of the latest load developments, and the accelerated rate of growth, which has developed in the area since Statehood.

As viewed at the present time, we believe this to be a very realistic forecast for the area.

Sincerely yours,

George N. Pierce
District Manager

Enclosures

EXHIBIT 4.1

POWER & ENERGY REQUIREMENTS - JOHNS VALLEY - 1965 FORECAST

<u>Calendar Year</u>	<u>Net Energy X 1000 KWH</u>		<u>Net Peak Kilowatts</u>		<u>Load Factor</u>
1950	14,050		3,800		50.1
51	15,522		3,850		48.8
52	17,364		4,090		48.5
53	19,771		4,350		51.9
54	21,108		5,025		48.0
1955	22,913		5,030		52.0
56	23,810		5,353		50.8
57	23,193		4,837		54.7
3/ 58	24,402		5,105		54.6
59	26,437		5,465		55.2
1960	29,156		5,837		57.0
61	32,282		7,750		47.6
62	34,712		7,056		56.0
63	37,150		9,044		46.9
1/ 64	41,327		9,424		50.0
1965	45,400		10,370		50.0
66	49,900		11,400		50.0
67	54,900		12,540		50.0
68	60,400		13,800		50.0
69	66,500		15,180		50.0
2/ 1970	73,100		16,690		50.0
71	83,500		18,690		51.0
72	95,400		20,940		52.0
73	108,900		23,450		53.0
74	124,200		26,260		54.0
1975	141,700		29,410		55.0
76	155,900		32,350		
77	171,500		35,590		
78	188,600		39,140		
79	207,500		43,060		
1980	228,200		47,370		
81	251,000		52,100		
82	276,100		57,310		
83	303,700		63,040		
84	334,100		69,350		
85	367,500		76,500		
86	404,300		83,900		
87	444,700		92,300		
88	489,100		101,500		

Load
Growth
Historical
Average

8.0%

Load
Growth

6.0%

10.9 %

10%

12%

10%

1/ All preceding values are actual record.

2/ Project assumed to go into operation end of P.Y. 1970

3/ Bill passed for Statehood.

EXHIBIT 4.1
Enclosure

FEDERAL POWER COMMISSION

REGIONAL OFFICE

555 BATTERY STREET, ROOM 415

SAN FRANCISCO 11, CALIF.

February 25, 1964

AIR MAIL

Colonel K. T. Sawyer
District Engineer
U. S. Army Engineer District, Alaska
Corps of Engineers
P. O. Box 7002
Anchorage, Alaska 99501

Dear Colonel Sawyer:

This is in answer to your letter of January 31, 1964 (your file reference NPAEN-PR-TP) requesting comments on specific questions which concern your study of the Snettisham Project.

a. The amount of reserve capacity required to maintain firm service would be equal to the largest reduction in total system dependable capacity which might result from an outage of a generating unit or transmission circuit at the time of the system peak.

At the present time the power supply in the Juneau area consists of small hydroelectric and internal-combustion plant generating units. The largest unit is 3,500 kilowatts. Another unit or a group of units, in reserve, equal in capacity to this unit would permit loads to be served on a firm basis. No reserve capacity would be needed if some loads could be dropped until the largest unit is returned to service, in the event of a forced outage.

With the addition of generating capacity such as may be installed at the Snettisham Project, firm service to the loads would be maintained by providing standby capacity in other power plants in the Juneau area equal to the dependable capacity of the largest project unit, or the capacity of its largest transmission circuit. This assumes that the greatest reduction in the supply to the Juneau area would be due to the loss of a Snettisham unit or line. On the other hand, it is conceivable that a thermal plant might be built or other hydro installations may be constructed with units larger than installed at Snettisham.

EXHIBIT 5 1

Colonel K. T. Sawyer

- 2 -

February 25, 1964

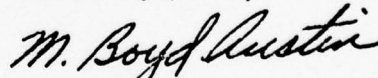
b. No amount of project power would be firm at the Juneau market if delivery is to be by one transmission circuit.

c. The alternative source of power to the project is considered by us at present to be a privately financed steam-electric plant located at Juneau tidewater. If power can be made dependably available from outlying system sources at a lower market cost, this cost would be a measure of the value of project power at market.

The value of hydroelectric power at Juneau based on cost of steam-electric power estimated in 1960 was \$65.11 per kilowatt-year for dependable capacity plus 6.91 mills per kilowatt-hour for energy. The total unit value of power for a 70 percent load factor delivery, amounts to 17.53 mills per kilowatt-hour. This value was based on financing of a 49,500-kilowatt steam-electric plant with cost of money at 7 percent and with taxes of 6.77 percent of the investment. The cost for fuel was based on bunker oil at Juneau delivered for \$3.20 per barrel.

By our letter of February 14, 1964, you were furnished at-steam plant values for the southwest area of Alaska for various plant sizes. These were based on cost of producing power at a 3 percent rate of interest and with no taxes. If the alternative plant capacity is to be initially comparable in size to the Snettisham installation, the value (because of the high cost of smaller units and other factors than were used for developing estimates of values furnished) would be somewhat greater.

Sincerely yours,



M. Boyd Austin
Regional Engineer

EXHIBIT 5.2



UNITED STATES
DEPARTMENT OF THE INTERIOR

BUREAU OF RECLAMATION

ALASKA DISTRICT HEADQUARTERS

P. O. BOX 2567, JUNEAU, ALASKA 99801

IN REPLY
REFER TO: 100

March 29, 1965

AIRMAIL

District Engineer
U.S. Army Engineer District, Alaska
P. O. Box 7002
Anchorage, Alaska 99501

Dear Sir:

This is to confirm our March 24, 1965 discussions with members of your staff, and with Messrs. Mark L. Nelson and W. L. Sasser of the North Pacific Division Office, relative to certain functional design criteria for the Snettisham Project.

With respect to total number of units, the discussions centered around a total of five units (three for Long Lake and two for Crater Lake) versus three units (two for Long and one for Crater). Due primarily to the difficulties which would be encountered in design, fabrication, installation, and operation and maintenance of the penstock interconnections requested in my letter to you dated February 11, 1965, together with the additional costs involved, a five-unit installation appears highly undesirable. We would much prefer a three-unit installation. Additionally, the larger units are more desirable from a system operating standpoint.

To provide greater operating flexibility, and to minimize stocks of spare parts required to be kept on hand, we would prefer that all three generators have the same kva rating. It is understood that this would result in a lower overall plant factor for the Crater Lake unit than for the Long Lake units, but this would not present any operating problem.

Also discussed was the matter of dependable plant capacity, and whether or not sufficient reserves will be available from other sources to at least equal the capacity of the largest unit at Snettisham. In this

EXHIBIT 6.1

connection, the local power supplying entities estimate there will be approximately 20,000 kw of capacity available for standby in the Juneau area by the end of 1969:

On the subject of arrangement of tunnel intake stoplogs and trashracks, my letter to you of February 11, 1965 visualized separate slots for these two features. At the March 24 meeting, your staff suggested the possibility of a single slot for stoplogs only, with trashracks fixed in place by means of stainless steel bolts. This latter arrangement would be satisfactory to us.

Sincerely yours,



George N. Pierce
District Manager

EXHIBIT 6.2



UNITED STATES
DEPARTMENT OF THE INTERIOR

BUREAU OF RECLAMATION

ALASKA DISTRICT HEADQUARTERS
P. O. BOX 2567, JUNEAU, ALASKA

IN REPLY
REFER TO: 620

April 1, 1965

AIRMAIL

District Engineer
U.S. Army Engineer District, Alaska
Post Office Box 7002
Anchorage, Alaska 99501

Dear Sir:

In accordance with the request of your staff during our March 24, 1965 meeting, we are furnishing our recommendations for power factor rating of Snettisham Project electrical equipment.

It is reported by the local power company that the area load power factor varies between 0.90 and 0.95; therefore, we recommend that the Project be designed for 0.90 power factor operation.

We would prefer that the generators be rated at a nameplate power factor of not greater than 0.90 in order to give a fairly wide range of voltage control without undue sacrifice of nameplate kilowatt capacity. It is realized that, as the peak load approaches the nameplate capacity of the Project, the need for supplemental voltage control equipment in the form of voltage regulator and/or banks of capacitors situated at the Juneau Substation will probably be required. This can readily be worked out during the later stages of construction if proper provisions are made in the initial stage.

The switchyard and substation capacities should be sized to the capacity of the generating units. If multiple transformers are used, an additional emergency rating (by use of supplemental cooling) should be provided against the possibility of a failure of one of the parallel transformers. The capacity of the Juneau Substation could be less than that of the Switchyard by amount of line losses.

Sincerely yours,

Geo. N. Pierce
George N. Pierce
District Manager

EXHIBIT 7.1



IN REPLY
REFER TO: 100

UNITED STATES
DEPARTMENT OF THE INTERIOR

BUREAU OF RECLAMATION

ALASKA DISTRICT HEADQUARTERS
P. O. BOX 2867, JUNEAU, ALASKA

April 1, 1965

AIRMAIL

District Engineer
U.S. Army Engineer District, Alaska
P. O. Box 7002
Anchorage, Alaska 99501

Dear Sir:

This is further to my letter to you of March 29, 1965 relative to our discussions of March 24, 1965, with representatives of your office and of the North Pacific Division Office, on functional design criteria for the Snettisham Project.

It was brought out that Corps of Engineers specifications now require that generators shall be capable of operating continuously at 15 percent overload. It is also the practice of the Corps to utilize this 15 percent overload capability in measuring project benefits for benefit-cost analyses. This is reflected in Table 5 of the draft of Snettisham Design Memorandum No. 3, which we presently have under informal review in this office.

At the March 24 meeting, we agreed to advise you of Bureau of Reclamation practices with respect to the amount of generator overload capacity required by our standard specifications, the use or non-use of such overload capacity in benefit-cost analyses, and whether or not such overload capacity is considered saleable under Department of the Interior power marketing policies.

The Bureau's treatment of generator overload capacity has been discussed both with the Office of our Commissioner and of our Chief Engineer. Current Bureau practices are as follows:

1. In a number of recent instances, our generator specifications have required a continuous overload capability of 15 percent at rated voltage, power factor and frequency.

EXHIBIT 8.1

2. Generator overload capability is not considered in measuring project benefits, either in benefit-cost analyses for plan formulation, or in determination of project feasibility.
3. Generator overload capability is not considered in our average rate and repayment analyses, which determine the rates at which power must be sold to recover, with interest, all project costs allocated to power.
4. Power from a project is marketed only up to total name plate capacity, regardless of generator overload capability, even in those instances where a continuous overload capability has been included in the specifications.

Thus, while it is desirable that your specifications for Snettisham generators include a requirement for a continuous overload capability of 15 percent, when the Project goes into operation we will limit the contract obligation of the Project to meet area demands to total name plate capacity.

In connection with scoping the Snettisham Project, and in particular the determination of the desirability of a dam at Long Lake, the effect of any resulting increase in required power rates must be seriously considered. We already know that Snettisham power rates will be higher than those indicated in the documents on which authorization was based, due to increases both in interest rate and in construction costs. This amount of rate increase alone will affect the marketability of Project power, particularly in the industrial field. It would certainly be advantageous to the Project and to the Juneau area if a dam at Long Lake would make additional power and energy available at no increase in unit power cost. However, any significant increase in power rates which would be required by the addition of a dam at Long Lake could only have an additional adverse effect on the marketability of Project power, and on the ability of the Project to return to the Treasury all Project costs, with interest.

Sincerely yours,



George N. Pierce
District Manager

EXHIBIT 8.2



UNITED STATES
DEPARTMENT OF THE INTERIOR

BUREAU OF RECLAMATION

ALASKA DISTRICT HEADQUARTERS
P. O. BOX 2567, JUNEAU, ALASKA

IN REPLY
REFER TO: 700

April 23, 1965

AIRMAIL

District Engineer
U.S. Army Engineer District, Alaska
Corps of Engineers
P. O. Box 7002
Anchorage, Alaska 99501

Dear Sir:

We have used the revised Snettisham cost estimates and construction schedules transmitted by your letter of 6 April 1965, reference NPAEN-PR-R, in computation of firm power rate and in benefit-cost comparisons for each of the seven alternates. A summary tabulation of the results of these computations for the overall project is attached.

Our computations used the annual O&M estimate of \$390,000 contained in our letter of April 6, 1965 and a separately determined replacement estimate. The O&M estimate was held constant for all plans and stages, since a full complement of men and maintenance equipment will be required from the start of operation, while the replacement estimate has varied to reflect stage development. Benefit determinations used the firm power values of \$29.29 per kilowatt-year, and 6.91 mills per kilowatt-hour which were obtained from your office. At our annual load factor of 55%, these rates are equivalent to a single charge of 13 mills per kilowatt-hour for firm energy. Non-firm energy benefits and revenues were assumed to be 7 and 5 mills per kilowatt-hour, respectively.

We note your comment that the current cost estimates do not reflect changes in costs for penstock interconnections, transmission line access roads, and revision of housing requirements. However, these changes should be equivalent for the various alternates and should have little effect on selection of dam size.

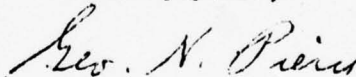
EXHIBIT 9.1

On the basis of the above evaluations, we concur in your finding that inclusion in the plan of development of a dam at Long Lake is merited. We also find that the alternate which would raise the Long Lake water surface elevation to 895 feet would produce the greatest excess of benefits over costs.

Our average rate and repayment analyses show that the most favorable firm power rate (9 mills/kwh) would be obtained with the plan which would raise Long Lake to elevation 840. The other alternates, including the no-dam plan and the 895 alternate, would require slightly higher power rates. It appears that the increase in the firm power rates for the more favorable alternate dam heights would not be so great as to affect the willingness of potential power customers to utilize all available project power. This condition likely would not prevail, however, should there be any further significant increases in estimated costs.

There was one additional point discussed during our March 24, 1965 discussions with representatives of your office and of the North Pacific Division, which could effect your decision whether or not to include a dam at Long Lake in your recommended plan of development. It was the consensus of that meeting that, in view of the urgent need for Snettisham power at the earliest possible date, if inclusion of a Long Lake dam would require reauthorization of the Project by the Congress, no dam should be recommended.

Sincerely yours,



George N. Pierce
District Manager

Enclosure

CHARTS

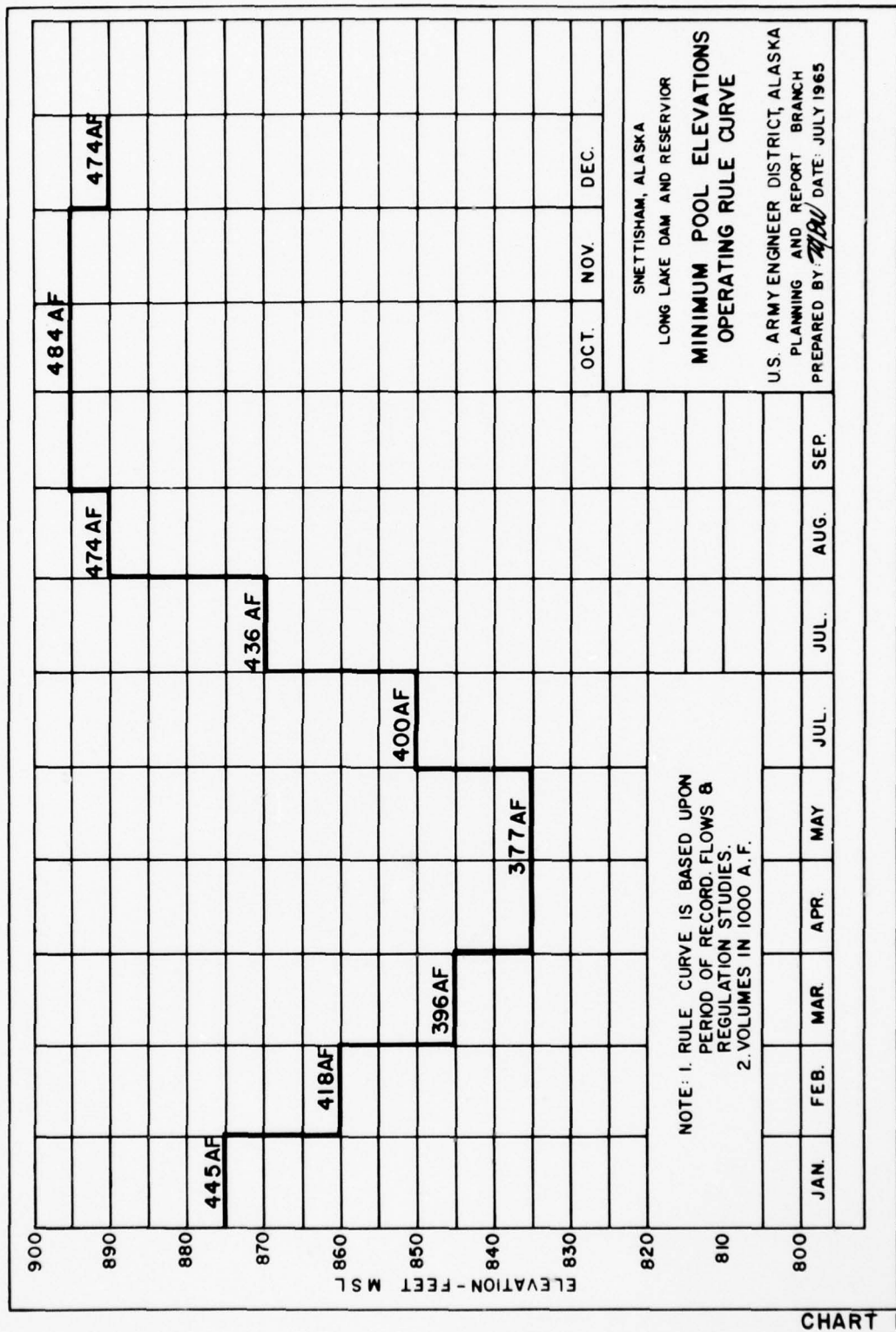
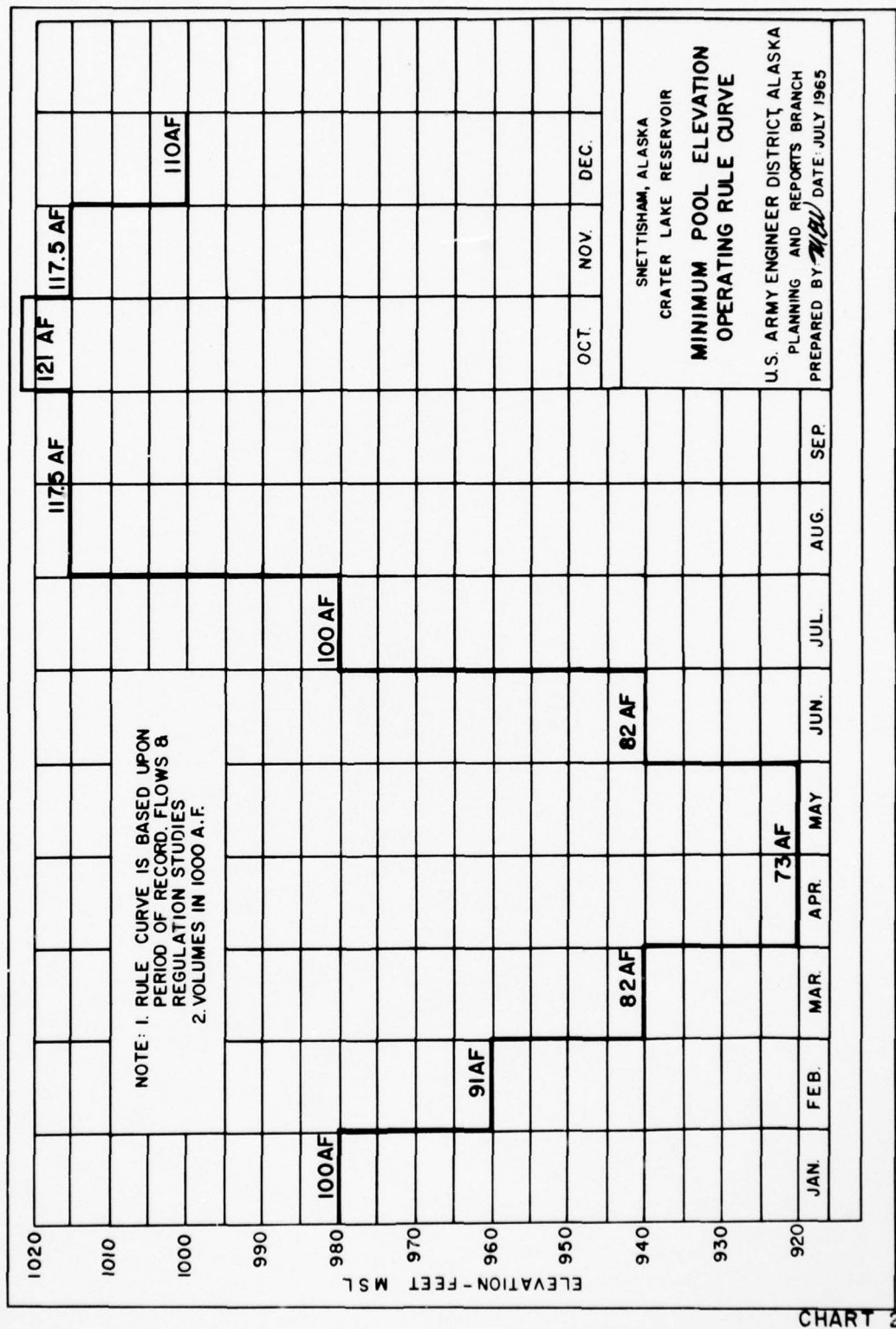
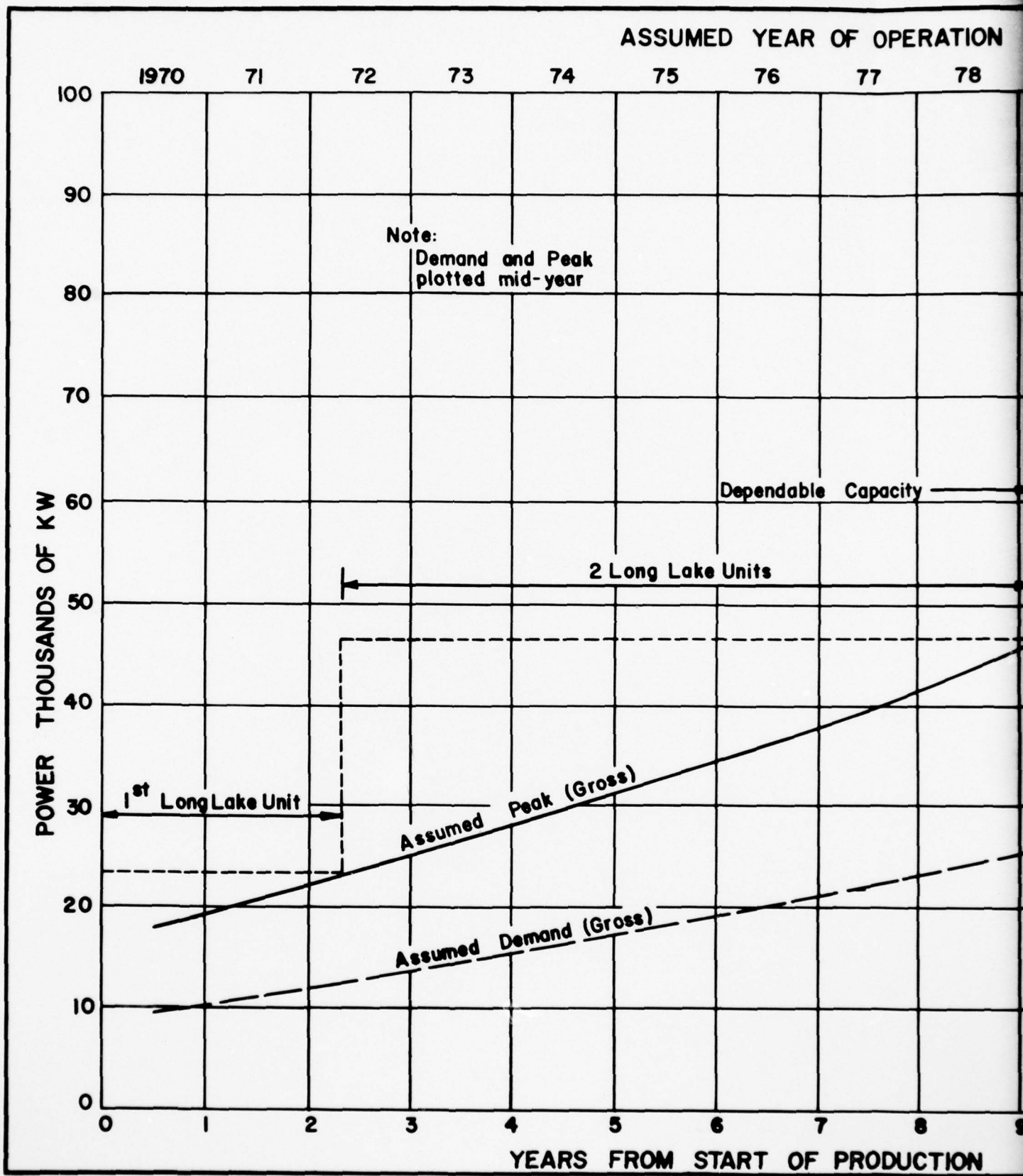
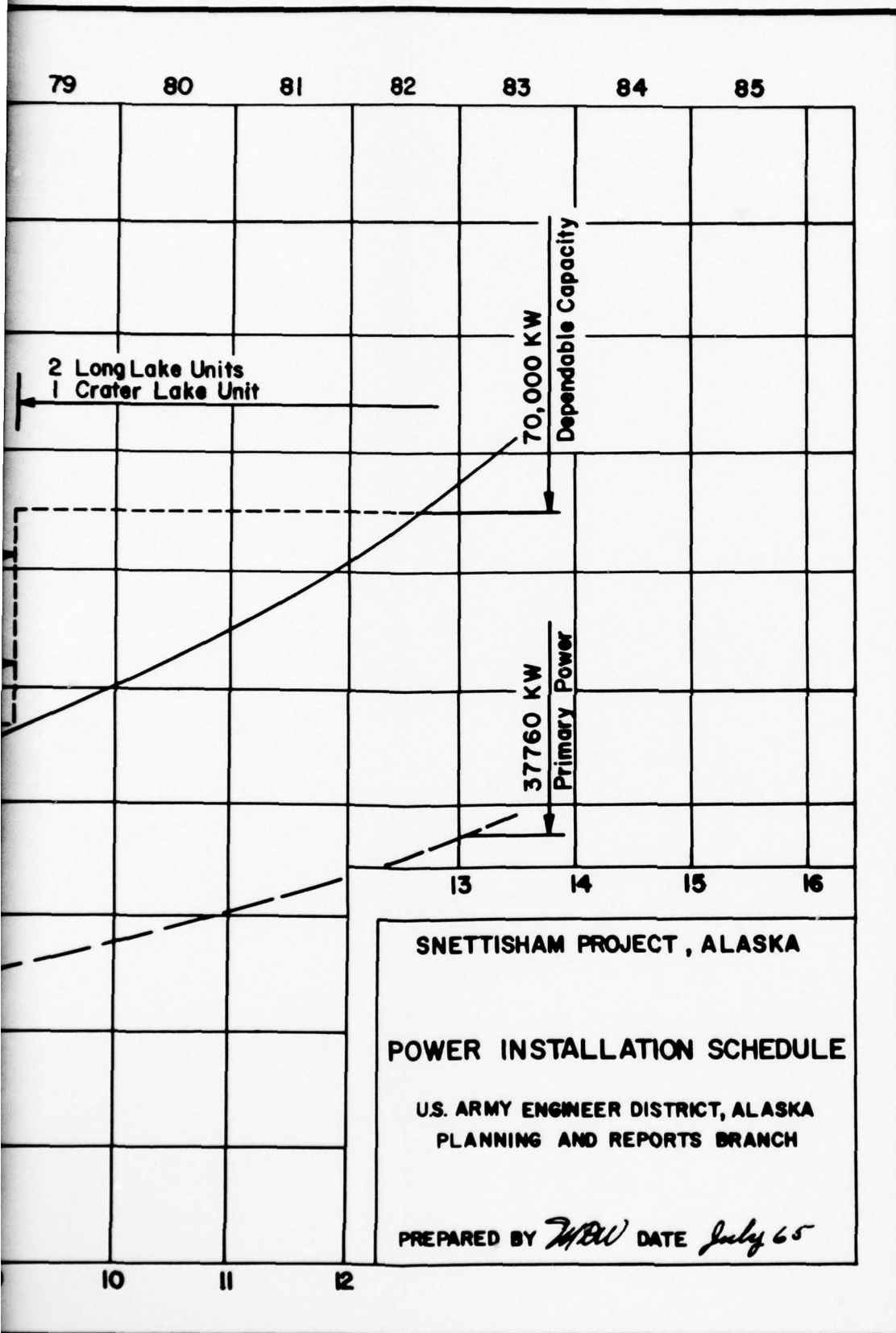


CHART I

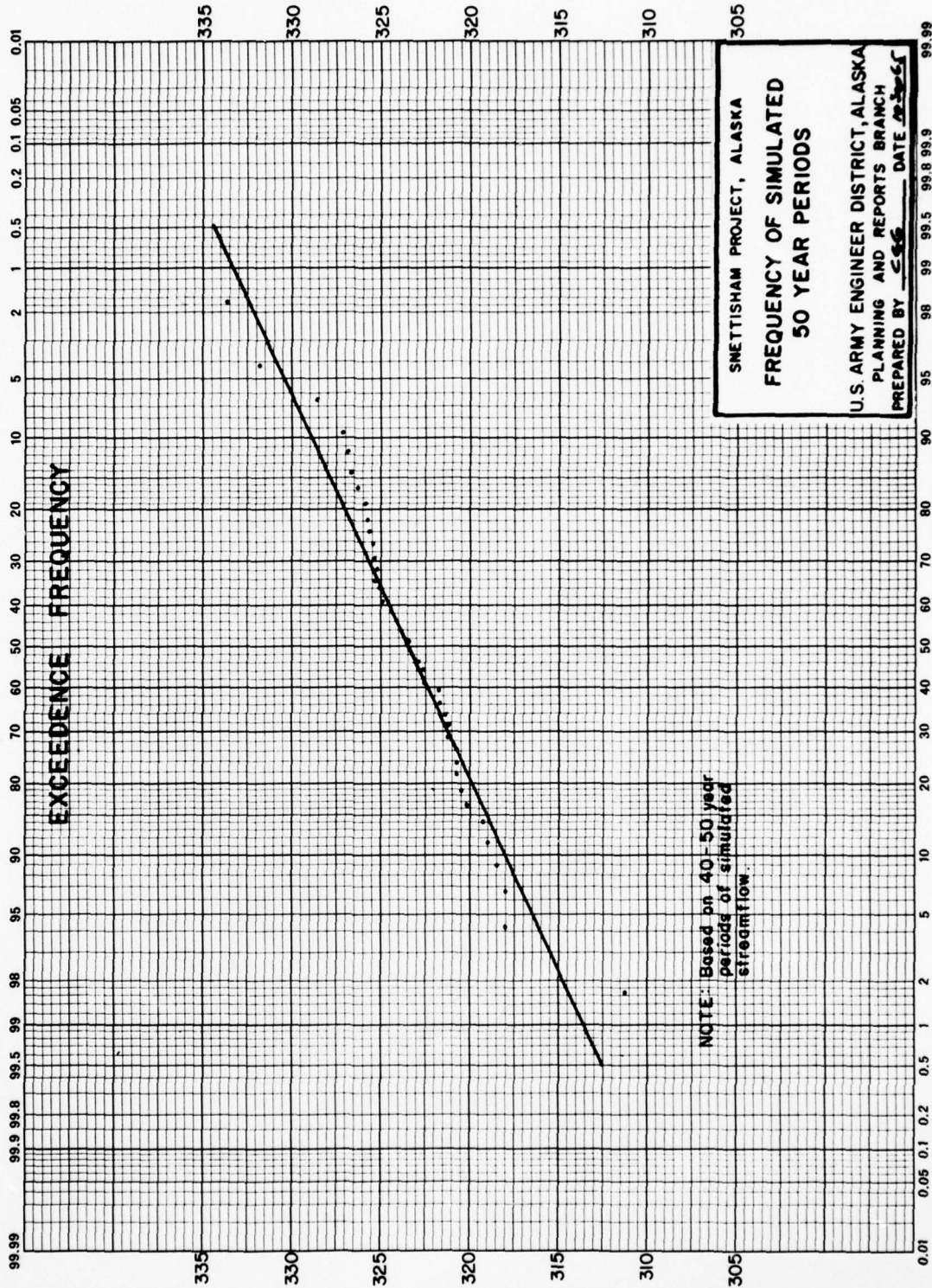


DESIGN MEMORANDUM 7 APPENDIX A CHART 2





AVERAGE ANNUAL FLOW PER 50 YR. PERIOD IN 1000'S ACRE FT.



SNETTISHAM PROJECT, ALASKA
 FREQUENCY OF SIMULATED
 50 YEAR PERIODS
 U.S. ARMY ENGINEER DISTRICT, ALASKA
 PLANNING AND REPORTS BRANCH
 PREPARED BY CSS DATE 20-3-65

TABLES

TABLE 1

HISTORICAL DATA - JUNEAU AREA POWER AND ENERGY DEMAND

1950 through 1955

PERIOD	19 50			19 51			19 52			19 53			19 54			19 55		
	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS
JAN	2950	1,290		3245	1,391		3660	1,634		4100	1,774		4325	1,848		4100	1,909	
FEB	2740	1,096		3070	1,227		3320	1,373		3600	1,498		3960	1,662		4350	1,796	
MAR	2390	1,136		2870	1,287		2870	1,397		3390	1,599		4540	1,763		4290	2,029	
APR	2420	1,084		2775	1,190		3040	1,350		3500	1,410		3350	1,683		4320	1,847	
MAY	3200	1,132		2645	1,246		2890	1,371		3020	1,454		3380	1,608		4200	1,798	
JUN	2360	1,064		2380	1,207		2640	1,332		3110	1,460		3260	1,627		4000	1,705	
JUL	2280	1,126		2590	1,224		2640	1,376		3630	1,614		3140	1,679		3620	1,728	
AUG	2580	1,131		2845	1,232		2820	1,415		3680	1,652		3500	1,633		3720	1,854	
SEP	2640	1,155		3160	1,243		3200	1,431		3960	1,776		3600	1,752		3920	1,797	
OCT	2780	1,239		3395	1,363		3620	1,520		4130	1,829		4060	1,839		4000	1,978	
NOV	3200	1,269		3630	1,429		4090	1,518		4350	1,813		4210	1,912		5030	2,107	
DEC	3100	1,328		3630	1,482		3730	1,647		4150	1,892		5025	2,047		4700	2,365	
ANNUAL	3200	14,050		3630	15,521		4090	17,364		4350	19,771		5025	21,108		5030	22,913	
L.FACTOR		50.1%			48.8%			48.5%			51.9%			48.0%			52.0%	
%GROWTH				13.4	10.5		12.7	11.9		6.4	13.9		15.5	6.8		0.1	8.6	

NOTES:

1. The above information from records of Alaska Electric Light and Power Company, Juneau, Alaska
2. This only includes power sold by Alaska Light and Power Company

TABLE 1 (cont'd)
HISTORICAL DATA - JUNEAU AREA POWER AND ENERGY DEMAND
1956 through 1961

PERIOD	19_56_			19_57_			19_58_			19_59_			19_60_			19_61_		
	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS
JAN	5310	2,367		4837	2,135		4741	2,203		5312	2,484		5341	2,530		6030	2,897	
FEB	4096	2,118		4256	1,888		4550	2,004		4373	2,160		5410	2,402		5450	2,655	
MAR	3705	2,071		3937	1,922		4005	2,001		4172	2,272		4688	2,499		4964	2,781	
APR	3668	1,771		3968	1,828		4160	1,894		4430	2,136		4543	2,333		5250	2,603	
MAY	3835	1,796		3780	1,899		4284	1,953		3399	2,098		4558	2,249		5486	2,504	
JUN	3392	1,792		3750	1,747		3906	1,760		4047	2,005		4558	2,190		5016	2,405	
JUL	3790	1,850		4191	1,773		3900	1,852		4419	2,096		4357	2,220		5230	2,386	
AUG	3709	1,290		3637	1,826		3938	1,950		4393	2,133		4639	2,322		5159	2,544	
SEP	4096	1,915		4000	1,901		4318	2,054		4773	2,182		4655	2,461		6062	2,647	
OCT	4388	1,990		4788	2,029		4672	2,190		4908	2,348		5055	2,630		5610	2,885	
NOV	5353	1,990		4580	2,050		4828	2,155		5465	2,381		5593	2,759		6903	3,028	
DEC	4704	2,160		4747	2,195		5105	2,386		5407	2,564		5837	2,884		7750	3,277	
ANNUAL	5353	23,310		4837	23,193		5105	24,402		5465	26,859		5837	29,479		7750	32,612	
L.FACTOR		50.8%			54.7%			54.6%			56.1%			57.7%			48.0%	
%GROWTH	6.4	3.9		-9.6	-2.6		5.5	5.2		7.1	10.1		6.8	9.8		32.8	10.6	

TABLE 1 (cont'd)

TABLE 1 (cont'd)
HISTORICAL DATA - JUNEAU AREA POWER AND ENERGY DEMAND
1962 to present

PERIOD	19_62			19_63			19_64			19_65			19_			19_		
	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS	PEAK KW	1,000 KWH	DAYS
JAN	7056	3,305		6295	3,057		9158	3,741		10023	4137							
FEB	5583	2,923		5899	2,824		7695	3,491		9092	3643							
MAR	6177	3,040		7334	2,700		7401	3,587		7952	3723							
APR	5345	2,786		6270	3,010		7182	3,374										
MAY	5617	2,744		5985	3,028		6992	3,205										
JUN	5464	2,585		5947	2,801		6555	3,109										
JUL	5480	2,596		6137	2,939		6621	3,129										
AUG	5630	2,686		6175	2,971		6630	3,110										
SEP	5781	2,794		6603	3,164		7021	3,336										
OCT	6110	3,045		7800	3,183		8027	3,630										
NOV	6747	3,080		9044	3,860		9367	3,681										
DEC	6510	3,123		8408	3,613		9424	4,144										
ANNUAL	7056	34,712		9044	37,150		9424	41,537										
L.FACTOR		56%			46.9%			50.3%										
%GROWTH	-9.0	6.4		25.7	7.0		4.2	11.8										

TABLE 1 (cont'd)

TABLE 2
Present Power Supply -- Juneau Power Market Area

Plant	Owner- Ship	Installed Capacity	Capability when Operating KW	Present firm Capacity KW	Approximate date Installed	Number Units	Unit Capacity KW	Unit Type
Salmon Creek #1	A-J	2,800	1,600	1,600	1915	2	1,400	Hydro
Salmon Creek #2	A-J	2,800	2,800	2,800	1915	2	1,400	Hydro
Annex Creek	A-J	2,800	3,600	2,800	1915	2	1,400	Hydro
Sheep Creek	A-J	2,225	2,150	Inoperative	1914	3	1,400	Hydro
Nugget Creek	A-J	2,880	2,350	Inoperative	1914	2	1,000	Hydro
Treadwell Ditch #1	A-J	800	800	(Inoperative) (No storage)	1922	1	810	Hydro
Juneau Steam Plant	A-J	8,000	8,000	Inoperative	1917	2	6,000	Steam
Gold Creek Hydro	AE&P	1,600	1,600	No storage	1914	3	2,000	Steam
Gold Creek Diesel	AE&P	7,140	7,760	7,390	1914 1914 1950 1952 1954 1961 1963	4	400 400 800 1,250 1,250 1,140 4,120	Hydro Hydro Hydro Diesel Diesel Diesel Diesel
TOTAL		31,045	30,660	14,590				
Juneau Lumber Co. 1/		1,000	unknown	unknown				

1/ Not available for public use.

TABLE 3
JUNEAU AREA
ENERGY & PEAK REQUIREMENTS
1965 Forecast 1/

Fiscal Year	Net Energy 1000 KWH	Gross Energy 1000 KWH	Net Peak KW	Gross Peak KW	Load Factor %
1950	14800	15750	3400	3650	50.1
1951	16450	17500	3850	4100	48.8
1952	18550	19750	4200	4500	48.5
1953	20450	21750	4700	5000	51.9
1954	22000	23400	5050	5350	48.0
1955	23350	24850	5200	5500	52.0
1956	23500	25000	5100	5400	50.8
1957	23800	25300	4950	5300	54.7
1958	<u>4/</u> 25400	27000	5300	5600	54.6
1959	27800	29550	5650	6000	55.2
1960	30700	32650	6800	7250	57.0
1961	33500	35650	7400	7850	47.6
1962	35950	38250	8050	8550	56.0
1963	39250	41750	9250	9800	46.9
1964	<u>2/</u> 43350	46100	9900	10550	50.0
1965	47650	50700	10900	11600	50.0
1966	52400	55750	11950	12800	50.0
1967	57650	61350	13150	14000	50.0
1968	63450	67500	14500	15400	50.0
1969	69800	74250	15950	16950	50.0
1970	<u>3/</u> 78300	83300	17700	18800	50.0
1971	89450	95150	19800	21100	51.0
1972	102150	108650	22200	23600	52.0
1973	116550	124000	24850	26450	53.0
1974	132950	141450	27850	29600	54.0
1975	148800	158300	30900	32850	55.0
1976	163700	174150	33950	36150	55.0
1977	180050	191550	37350	39750	55.0
1978	198050	210700	41100	43700	55.0
1979	217850	231750	45200	48100	55.0
1980	239600	254900	49800	52900	55.0
1981	263550	280350	54700	58200	55.0
1982	289900	308400	60200	64000	55.0
1983	318900	339250	66200	70400	55.0
1984	350800	373200	72800	77500	55.0
1985	385900	410500	80100	85200	55.0
1986	424500	451600	88100	93700	55.0
1987	466900	496700	96900	103100	55.0
1988	513600	546400	106600	113400	55.0
1989	565000	601000	117300	124800	55.0
1990	621400	661000	129000	137250	55.0

1/ Based on U.S.B.R. letter of 24 February 1965, exhibit 4.

2/ All preceding values are actual record of net requirements.

3/ Project assumed to go into operation end of C.Y. 1969.

4/ Bill passed for Statehood.

TABLE 3

TABLE 4

PRESENT JUNEAU ENERGY DEMAND

Load Distribution (Percents of Total KW-hr)

<u>Annual</u>			<u>Weekly (November)</u>			<u>Daily - Fall (Nov)</u>	
<u>Month</u>	<u>KWH</u>	<u>%</u>	<u>Day</u>	<u>KWH</u>	<u>%</u>	<u>2 Hr Aver</u>	<u>KWH</u>
January	3,001,000	8.91	Thursday	148,190	14.22	12 Mid	5650
February	2,736,000	8.12	Friday	148,350	14.24	2	3650
March	2,813,000	8.35	Saturday	138,800	13.32	4	3550
April	2,702,000	8.02	Sunday	130,450	12.53	6	3450
May	2,635,000	7.82	Monday	155,100	14.89	8	5050
June	2,513,000	7.46	Tuesday	158,900	15.25	10	7000
July	2,560,000	7.60	Wednesday	162,000	15.55	12 Noon	7450
August	2,628,000	7.80				2	7450
September	2,763,000	8.20				4	7500
October	2,951,000	8.76				6	8500
November	3,150,000	9.35				8	7900
December	3,238,000	9.61				10	6850
Load Factor		51.7			71.0		

NOTE: Loadshape for annual, weekly and daily fall and spring energy demand based upon the period
2 hour averages 19-24 Nov 64, 2-8 May 65, respectively.

Daily - Spring (May)

<u>%</u>	<u>2 Hr Aver</u>	<u>KWH</u>	<u>%</u>
7.64	12 Mid	8525	6.22
4.93	2	7075	5.15
4.80	4	6610	4.82
4.66	6	7890	5.75
6.82	8	12180	8.87
9.46	10	14315	10.43
10.06	12 Noon	14165	10.32
10.06	2	14305	10.42
10.14	4	14480	10.55
11.49	6	13315	9.70
10.68	8	12845	9.36
9.26	10	11550	8.41
73.0			80.0

ods of record for 1959 to 1964, 19-25 Nov 64 and

TABLE 4

2

AD-A067 895

CORPS OF ENGINEERS ANCHORAGE AK ALASKA DISTRICT
SNETTISHAM PROJECT ALASKA. FIRST STAGE DEVELOPMENT. DESIGN MEMO--ETC(U)
OCT 65

F/G 13/2

UNCLASSIFIED

NL

2 OF 6

AD
A067895



TABLE 5
TYPICAL SUMMARY OF GROWTH PERIOD
LONG LAKE TO ELEVATION 895

YEAR	INFLOW 1000's of Acre-Feet	END OF YEAR Reservoir Content	POOL ELEVATION			Firm Millions
			Maximum	Minimum	Average	
1970	310	482	895	847	873	77.8
1971	260	481	895	889	894	86.7
1972	233	482	895	880	891	96.9
1973	217	466	895	879	890	107.8
1974	239	476	895	864	884	120.1
1975	288	475	895	869	887	138.9
1976	302	479	895	863	884	149.7
1977	271	474	895	861	884	166.4
1978	281	481	895	858	883	185.4
1979	221	461	895	852	879	206.7

NOTES: 1. Both units are assumed to be operating.
2. Uniform growth rate was assumed.
3. Inflows are high synthetic as developed.

ENERGY		WASTE
Available Secondary of Kilowatt	Total Hours	Non-Usable Acre-Feet
105.0	182.8	79,940
151.4	238.1	26,466
122.5	219.4	14,847
104.6	212.4	21,450
96.7	216.8	11,359
110.1	249.0	47,356
96.7	246.4	63,382
73.8	240.2	47,324
78.0	263.4	7,839
27.6	234.3	0

TABLE 5

TABLE 6

TYPICAL SUMMARY OF RESERVOIR OPERATION
LONG LAKE TO ELEVATION 895

YEAR	INFLOW	END OF YEAR Reservoir Content 1000's of Acre-Feet	POOL ELEVATION			ENERGY	
			Maximum	Feet Above MSL Minimum	Average	Firm Millions of Kil	Second of Kil
1983	299	441.7	895	833	868	224.9	21.0
1984	344	461.5	895	819	860	225.6	18.0
1985	326	458.6	895	829	867	224.9	19.6
1986	291	446.8	895	835	866	224.9	3.6
1987	268	412.3	879	821	855	224.9	
1988	291	391.3	862	797	832	225.6	
1989	302	375.8	850	776	817	224.9	
1990	340	398.1	858	771	815	224.9	
1991	358	447.6	887	788	840	224.9	
1992	293	433.1	895	828	863	225.6	5.5
1993	346	467.7	895	830	865	224.9	9.3
1994	332	464.4	895	843	873	224.9	16.3
1995	316	475.3	895	832	867	224.9	5.9
1996	335	443.7	895	855	880	225.6	43.8
1997	284	424.8	888	820	854	224.9	
1998	282	399.8	864	814	842	224.9	
1999	312	397.5	866	776	825	224.9	
2000	292	371.9	852	773	818	225.6	
2001	423	478.7	895	761	820	224.9	
2002	352	453.4	895	845	875	224.9	30.2
2003	336	452.9	895	838	869	224.9	29.9
2004	357	466.9	895	829	866	225.6	30.2
2005	344	447.5	895	843	873	224.9	41.6
2006	298	445.1	885	830	860	224.9	
2007	357	444.8	895	830	866	224.9	36.2
2008	421	476.4	895	832	867	225.6	30.2
2009	357	468.8	895	851	879	224.9	40.9
2010	349	441.3	895	840	872	224.9	30.2
2011	315	455.6	895	819	861	224.9	0.5
2012	329	456.8	895	838	870	225.6	18.9
2013	349	456.9	895	832	870	224.9	26.7
2014	338	463.7	895	834	868	224.9	19.1
2015	252	415.4	888	830	861	224.9	
2016	266	365.5	861	798	833	225.6	
2017	358	400.6	864	752	805	224.9	
2018	332	422.7	884	786	835	224.9	
2019	296	410.8	861	818	841	224.9	
2020	278	376.5	864	787	830	225.6	

Page 1 of 2 pages

WASTE

Waste	Total kilowatt hours	Non-Usable Acre-Feet
.0	245.9	
.0	243.6	
.6	244.5	5,299
.6	228.5	
	224.9	
	225.6	
	224.9	
	224.9	
	224.9	
.5	231.1	
.3	234.2	
.3	241.2	18,011
.9	230.8	
.8	269.4	15,920
	224.9	
	224.9	
	224.9	
	225.6	
	224.9	
.2	255.1	42,418
.9	254.8	654
.2	255.8	5,344
.6	266.5	13,026
	224.9	
.2	261.1	11,889
.2	255.8	51,517
.9	265.8	17,888
.2	255.1	40,791
.5	225.4	
.9	244.5	5,374
.7	251.6	17,447
.1	244.0	8,624
	224.9	
	225.6	
	224.9	
	224.9	
	224.9	
	225.6	

TABLE 6

2

TABLE 6
(cont'd)
TYPICAL SUMMARY OF RESERVOIR OPERATION
LONG LAKE TO ELEVATION 895

YEAR	INFLOW 1000's of Acre-Feet	END OF YEAR Reservoir Content 1000's of Acre-Feet	POOL ELEVATION Feet Above MSL			Firm Millions
			Maximum	Minimum	Average	
2021	313	366.6	841	759	804	224.9
2022	308	349.2	835	756	799	224.9
2023	327	350.8	842	748	799	224.9
2024	309	327.3	825	738	788	225.6
2025	343	331.5	822	720	770	224.9
2026	394	400.3	874	735	801	224.9
2027	325	413.6	867	788	831	224.9
2028	311	417.9	883	809	845	225.6
2029	299	408.6	879	793	838	224.9
2030	344	444.5	887	791	839	224.9
2031	326	458.6	895	816	859	224.9
2032	291	446.8	895	835	866	225.6
Average					845	
Minimum					720	
Average Annual						225.1

NOTE: Starting year 1983 for Long Lake is used to be compatible with the water year 1980 for

ENERGY

WASTE

Secondary Total
of Kilowatt Hours

Non-Usable
Acre-Feet

224.9
224.9
224.9
225.6
224.9
224.9
224.9
225.6
224.9
224.9
233.3
228.6

8.4
3.0

9.8 234.9

5,084

for Crater Lake.

TABLE 6

2

TABLE 7

TYPICAL SUMMARY OF RESERVOIR OPERATION
 LONG LAKE TO ELEVATION 895
 RULE CURVE OPERATION

YEAR	INFLOW	END OF YEAR Reservoir Content 1000's of Acre-Feet	POOL ELEVATION			ENERGY	
			Feet Above MSL			Fire Millions of Kilow	Secondary Millions of Kilow
			Maximum	Minimum	Average		
1983	299	441.7	895	829	866	224.9	20.3
1984	344	461.5	895	819	860	225.6	18.0
1985	326	458.6	895	828	866	224.9	20.2
1986	291	444.5	894	831	863	224.9	4.6
1987	268	409.3	878	820	853	224.9	
1988	291	387.5	860	794	829	225.6	
1989	302	370.7	846	772	814	224.9	
1990	340	391.1	854	766	811	224.9	
1991	358	438.7	883	782	835	224.9	
1992	293	429.7	894	822	858	225.6	
1993	346	467.7	895	828	863	224.9	6.2
1994	332	464.4	895	832	867	224.9	27.2
1995	316	474.0	895	831	867	224.9	6.7
1996	335	443.7	895	835	868	225.6	51.1
1997	284	424.8	888	820	854	224.9	
1998	282	399.8	864	814	842	224.9	
1999	312	397.5	866	776	825	224.9	
2000	292	371.9	852	773	818	225.6	
2001	423	474.0	895	761	820	224.9	3.7
2002	352	453.4	895	832	867	224.9	32.8
2003	336	452.9	895	835	866	224.9	29.1
2004	357	466.9	895	829	866	225.6	30.3
2005	344	447.5	895	835	868	224.9	47.4
2006	298	445.1	885	830	860	224.9	
2007	357	444.8	895	830	865	224.9	42.0
2008	421	474.0	895	832	866	225.6	35.3
2009	357	468.8	895	835	868	224.9	41.9
2010	349	441.8	895	835	868	224.9	33.5
2011	315	455.6	895	819	861	224.9	0.5
2012	329	456.8	895	835	866	225.6	21.4
2013	349	456.9	895	832	867	224.9	38.7
2014	338	463.7	895	834	867	224.9	18.7
2015	252	415.4	888	830	861	224.9	

Page 1 of 2 pages

WASTE

y watt	Total Hours	Non-Usable Acre-Feet
245.2		
243.6		
245.1		4,352
229.5		
224.9		
225.6		
224.9		
224.9		
224.9		
225.6		
231.1		
252.1		1,368
231.6		
276.7		
224.9		
224.9		
224.9		
225.6		
228.6		
257.7		31,354
254.0		
255.9		4,975
272.3		2,959
224.9		
266.9		3,622
260.9		46,988
266.8		9,388
258.4		34,244
225.4		
247.0		
263.6		
243.7		8,624
224.9		

TABLE 7

TABLE 7
(cont'd)
TYPICAL SUMMARY OF RESERVOIR OPERATION
LONG LAKE TO ELEVATION 895
RULE CURVE OPERATION

YEAR	INFLOW 1000's of Acre-Feet	END OF YEAR Reservoir Content 1000's of Acre-Feet	POOL ELEVATION Feet Above MSL			Firm Millions of	Sec Millions of
			Maximum	Minimum	Average		
2016	266	365.5	861	798	833	225.6	
2017	358	400.6	864	752	805	224.9	
2018	332	422.7	884	786	835	224.9	
2019	296	410.8	861	808	841	224.9	
2020	278	376.5	864	787	830	225.6	
2021	313	366.6	841	759	804	224.9	
2022	308	349.2	835	756	799	224.9	
2023	327	350.8	842	748	799	224.9	
2024	309	327.3	825	738	788	225.6	
2025	343	351.5	822	720	770	224.9	
2026	392	400.3	874	735	801	224.9	
2027	325	415.6	867	788	831	224.9	
2028	311	417.9	883	809	845	225.6	
2029	299	408.6	879	793	838	224.9	
2030	344	444.5	887	791	839	224.9	
2031	326	458.6	895	816	859	224.9	
2032	291	444.5	894	831	863	225.6	
Average					843		
Minimum					720		
Average Annual						225.1	

NOTE: Starting year 1983 for Long Lake is used to be compatible with the water year 1980 for C

Page 2 of 2 pages

ENERGY

WASTE

Secondary Total
F Kilowatt Hours

Non-Usable
Acre-Feet

225.6
224.9
224.9
224.9
225.6
224.9
224.9
224.9
225.6
224.9
224.9
224.9
225.6
224.9
224.9
233.3
229.6

8.4
4.0

10.8 35.9

2,957

Water Lake.

TABLE 7

2

TABLE 8

TYPICAL SUMMARY OF RESERVOIR OPERATION
CRATER LAKE TO ELEVATION 1022
RULE CURVE OPERATION

YEAR	INFLOW	END OF YEAR Reservoir Content 1000's of Acre-Feet	POOL ELEVATION			Firm Millions of	Sec
			Maximum	Minimum	Average		
1980	213	106.9	1022	861	950	106.3	3
1981	134	105.8	1022	916	973	106.0	
1982	148	102.7	1022	920	973	106.0	2
1983	134	103.1	1022	888	959	106.0	
1984	149	110.0	1022	901	967	106.3	1
1985	148	109.9	1022	910	975	106.0	1
1986	130	104.8	1022	912	969	106.0	
1987	113	89.4	996	899	955	106.0	
1988	126	81.5	971	854	917	106.3	
1989	132	77.5	958	823	899	106.0	
1990	154	95.2	985	819	904	106.0	
1991	175	110.0	1022	882	960	106.0	15
1992	117	93.7	1010	919	968	106.3	
1993	144	109.9	1022	893	962	106.0	
1994	155	110.0	1022	908	974	106.0	14
1995	143	110.2	1022	905	971	106.0	15
1996	154	104.1	1022	920	975	106.3	24
1997	122	98.7	1015	893	954	106.0	
1998	120	89.2	980	891	941	106.0	
1999	138	96.2	1000	839	926	106.0	
2000	131	96.7	1000	862	938	106.3	
2001	196	110.0	1022	883	958	106.0	24
2002	162	107.6	1022	911	973	106.0	14
2003	159	107.3	1022	920	973	106.0	24
2004	162	110.0	1022	909	974	106.3	25
2005	156	105.4	1022	920	976	106.0	24
2006	131	103.4	1014	910	967	106.0	
2007	162	104.3	1022	918	975	106.0	25
2008	192	110.0	1022	913	972	106.3	24
2009	162	110.0	1022	919	976	106.0	25
2010	163	103.0	1022	920	974	106.0	24

ENERGY

WASTE

Secondary Kilowatt	Total Hours	Non-Usable Acre-Feet
1.5	137.8	25,203
8.6	114.6	
1.2	127.2	669
6.0	112.0	
13.5	119.8	
15.9	121.9	5,515
8.5	114.5	
1.0	107.0	
	106.3	
	106.0	
	106.0	
15.6	121.6	15,630
6.0	122.3	
2.9	108.9	
18.8	124.8	8,896
15.6	121.6	
28.8	135.1	964
	106.0	
	106.0	
	106.0	
	106.3	
24.4	130.4	27,280
18.5	124.5	18,242
21.2	127.2	1,064
25.4	131.7	5,425
24.8	130.8	6,824
1.6	107.6	
29.0	135.0	7,067
28.4	134.7	28,296
22.8	128.8	10,494
21.1	127.1	20,025

TABLE 8

2

TYPICAL SUMMARY OF RESERVOIR OPERATION
CRATER LAKE TO ELEVATION 1022
RULE CURVE OPERATION

YEAR	INFLOW	END OF YEAR Reservoir Content 1000's of Acre-Feet	POOL ELEVATION Feet Above MSL Maximum Minimum Average			Firm Millions of	Sec of
20 11	143	108.1	1022	899	968	106.0	1
20 12	151	108.8	1022	920	973	106.3	2
20 13	156	108.8	1022	918	975	106.0	2
20 14	151	110.0	1022	920	975	106.0	1
20 15	113	91.3	1010	908	964	106.0	4
20 16	118	76.6	980	859	926	106.3	
20 17	163	103.4	1008	801	900	106.0	
20 18	153	104.2	1022	901	968	106.0	1
20 19	127	104.2	999	904	959	106.0	
20 20	125	101.1	1022	892	961	106.3	
20 21	139	106.9	1010	890	955	106.0	
20 22	137	108.6	1022	914	972	106.0	
20 23	150	106.8	1022	920	974	106.0	2
20 24	140	106.1	1017	910	970	106.3	12
20 25	158	110.0	1022	914	973	106.0	2
20 26	185	103.9	1022	920	976	106.0	4
20 27	145	110.0	1022	905	969	106.0	1
20 28	149	109.3	1022	915	973	106.3	2
20 29	213	106.9	1022	920	980	106.0	4
Average					961		
Minimum					801		
Average Annual						106.1	1

Page 2 of 2 pages

GY

WASTE

dary ilowatt	Total llouts	Non-Usable Acre-Feet
5	116.5	
0	127.3	
8	127.8	4,832
8	125.8	2,539
8	150.8	
	106.3	
	106.0	
8	124.8	5,145
	106.0	
0	107.3	
9	110.9	
1	115.1	
4	128.4	
5	118.8	
2	127.2	4,546
8	152.8	11,785
9	116.9	
4	126.7	526
7	146.7	43,457
		5,088

120.0

TABLE 8

2

TABLE 9

**TYPICAL SUMMARY OF RESERVOIR OPERATION
CRATER LAKE TO ELEVATION 1022**

YEAR	INFLOW 1000's of Acre-Feet	END OF YEAR Reservoir Content 1000's of Acre-Feet	POOL ELEVATION Feet Above MSL			Firm Million
			Maximum	Minimum	Average	
1980	213	106.9	1022	861	952	106.3
1981	134	105.8	1022	918	973	106.0
1982	148	102.7	1022	921	979	106.0
1983	134	103.1	1022	888	959	106.0
1984	149	111.6	1022	901	967	106.3
1985	148	110.1	1022	917	979	106.0
1986	130	104.8	1022	921	975	106.0
1987	113	90.3	998	901	956	106.0
1988	126	82.8	973	857	920	106.3
1989	132	79.5	962	828	903	106.0
1990	154	98.3	991	828	910	106.0
1991	175	111.4	1022	891	965	106.0
1992	117	100.1	1022	928	977	106.3
1993	144	109.9	1022	911	973	106.0
1994	155	113.1	1022	922	982	106.0
1995	143	118.5	1022	914	977	106.0
1996	154	104.1	1022	960	1000	106.3
1997	122	98.7	1015	893	954	106.0
1998	120	89.2	980	891	941	106.0
1999	138	96.2	1000	839	926	106.0
2000	131	96.7	1000	862	938	106.3
2001	196	117.3	1022	883	959	106.0
2002	162	107.6	1022	933	988	106.0
2003	159	107.3	1022	927	980	106.0
2004	162	112.7	1022	909	974	106.3
2005	156	105.4	1022	932	984	106.0
2006	131	110.5	1018	910	969	106.0
2007	162	104.3	1022	929	983	106.0
2008	192	116.1	1022	913	975	106.3
2009	162	113.3	1022	943	994	106.0
2010	163	103.0	1022	930	984	106.0
2011	143	108.8	1022	899	969	106.0
2012	151	108.8	1022	924	980	106.3
2013	156	109.1	1022	920	983	106.0
2014	151	114.1	1022	921	978	106.0
2015	113	97.7	1022	919	974	106.0

ENERGY

WASTE

Boundary Kilowatt	Total Hours	Non-Usable Acre-Feet
2.1	128.4	36,722
8.7	114.7	
8.1	124.1	6,164
6.0	112.0	
2.1	118.4	
7.6	123.6	5,515
9.5	115.5	
	106.0	
	106.3	
	106.0	
	106.0	
7.8	123.8	15,630
2.9	109.2	
9.4	115.4	
7.1	123.1	8,896
1.9	117.9	
9.2	135.5	13,074
	106.0	
	106.0	
	106.0	
	106.3	
8.0	124.0	27,280
9.3	125.3	26,970
6.2	122.2	8,007
3.1	129.4	5,425
4.9	130.9	10,964
	106.0	
5.5	131.3	15,264
0.6	126.9	31,832
4.1	130.1	15,269
0.2	126.2	26,313
0.2	116.2	
4.7	121.0	9,487
6.6	122.6	12,595
6.9	122.9	2,539
3.8	109.8	

TABLE 9

2

TABLE 9 (cont'd)

TYPICAL SUMMARY OF RESERVOIR OPERATION
CRATER LAKE TO ELEVATION 1022

YEAR	INFLOW 1000's of Acre-Feet	END OF YEAR Reservoir Content 1000's of Acre-Feet	POOL ELEVATION Feet Above MSL			Firm Millions of
			Maximum	Minimum	Average	
2016	118	85.7	997	881	944	106.3
2017	163	111.0	1022	842	927	106.0
2018	153	104.2	1022	922	981	106.0
2019	127	104.2	999	904	959	106.0
2020	125	101.1	1022	892	961	106.3
2021	139	112.8	1021	890	958	106.0
2022	137	111.0	1022	930	983	106.0
2023	150	106.8	1022	934	989	106.0
2024	140	108.5	1022	910	975	106.3
2025	158	111.7	1022	920	978	106.0
2026	185	103.9	1022	937	989	106.0
2027	145	113.5	1022	905	970	106.0
2028	149	109.3	1022	940	988	106.3
2029	213	106.9	1022	932	987	106.0
Average					967	
Minimum					828	
Average Annual						106.1

Page 2 of 2 pages

ENERGY

WASTE

Secondary Kilowatt	Total Hours	Non-Usable Acre-Feet
	106.3	
5.4	111.4	
4.2	130.2	8,577
	106.0	
1.0	107.3	
	106.0	
2.0	118.0	1,695
0.7	126.7	7,707
1.6	117.9	
9.3	125.3	8,424
3.7	139.7	31,826
7.9	113.9	
5.2	121.5	12,122
3.1	109.1	56,390
2.4	118.5	8,107

TABLE 9
(cont'd)

2

TABLE 10
SUMMARY OF FINAL STUDIES

Lake	Operation	Pool Elevation			Maximum Drawdown (ft)	Design Head <u>1/</u> (ft)	Minimum Pool % Design Head	Maximum Reqr'd Disch cfs
		Maximum	Minimum	Average Elevation (ft MSL)				
Long	Normal	895	719.8	845.2	175.2	845.2	85.2	980
Long	Rule Curve	895	819.8	843.5	175.2	843.5	85.3	980
Crater	Normal	1022	827.8	967.4	194.2	967.4	85.6	400
Crater	Rule Curve	1022	801.4	960.7	220.6	960.7	83.4	410

1/ Design Head is equal to average head.

Waste Average Annual AF	Active Storage AF	Energy		Dependabl Capacity KW
		Average Annual		
		Primary KWH	Secondary KWH	
5,084	252,000	225,000,000	9,800,000	46,700
2,957	252,000	225,000,000	10,800,000	46,700
8,107	81,000	106,000,000	12,400,000	23,350
5,088	88,500	106,000,000	13,900,000	23,350

TABLE 10

2

TABLE 11
SNETTISHAM UNIT CRITERIA

<u>Item</u>	<u>Long Lake</u>	<u>Crater Lake</u>
Installed (KW) <u>1/</u>	40,600 (2 units)	20,300
Primary Energy (KWH)	225 x 10 ⁶	106 x 10 ⁶
Dependable Capacity w/overload (KW)	46,700	23,350
Maximum Pool (ft msl)	895	1022
Minimum Pool (ft msl)	720	828
Average Pool (ft msl)	845	967
Minimum TW (ft msl) <u>2/</u>	-1.0	-1.0
Maximum TW (ft msl)	16.0	16.0
Power Factor <u>3/</u>	0.9	0.9
Load Factor	0.55	0.52 <u>4/</u>
Maximum Q (cfs - normal operation)	930	410
Maximum Q (cfs - 2 unit operation)	-	530 <u>5/</u>

1/ Units were made equal size as per USBR letter of 29 Mar 65, exhibit 6.

2/ Minimum tailwater established by a sill assumed to be at elevation -1.0

3/ Power factor is as requested in USBR letter of 1 Apr 65, exhibit 7.

4/ Load factor (annual) for Long Lake is the same as used in the authorizing documents; however, the requirement for units of equal size (exhibit 6) changes Crater Lake to approximately 52%.

5/ This flow would be required only for peaking (early summer peak - 34000 KW) during closure of Long Lake penstock. Increased hydraulic losses could be tolerated for peaking during the short period of closure.

TABLE 11

TABLE 12

DETAIL RESULTS - ALL PRELIMINARY STUDIES

Lake	Pool Elevations		Average	Maximum Drawdown (feet)	Primary	Millions of KWH
	Maximum	Minimum Elevation (ft MSL)				
Long	815.0	687.0	780.3	128.0	185.1	27.3
	815.0	663.1	775.6	151.9	190.1	22.0
	815.0	643.4	770.5	171.6	192.6	19.1
	900.0	784.3	868.7	115.7	218.4	20.4
	900.0	771.0	862.3	129.0	222.7	16.7
	900.0	734.0	850.5	166.0	225.2	12.4
	950.0	850.8	919.4	99.2	237.7	17.6
	950.0	844.2	915.4	105.8	240.2	14.4
	950.0	796.7	903.3	153.3	242.6	9.8
	1020.0	942.2	1001.5	77.8	250.2	26.0
	1020.0	941.1	997.9	78.9	255.2	20.9
	1020.0	926.4	991.2	93.6	260.2	15.8
	1020.0	876.0	975.0	144.0	263.0	7.5
Crater	1020.0	860.8	984.8	159.2	94.8	20.7
	1020.0	852.9	979.4	167.1	99.8	18.2
	1020.0	845.0	970.0	175.0	104.8	13.2
	1120.0	1012.8	1093.1	107.2	109.8	20.2
	1120.0	1009.6	1089.3	110.4	114.8	17.2
	1120.0	992.7	1083.8	127.3	120.1	14.7
	1120.0	952.8	1077.6	167.2	122.6	12.5
	1220.0	1141.1	1197.8	78.9	119.8	22.5
	1220.0	1139.0	1195.2	81.0	124.7	19.6
	1220.0	1135.4	1191.8	84.6	129.7	15.9
	1220.0	1105.1	1187.0	114.9	133.0	11.4
Combined	860.0	723.4	823.7	136.6	282.7	41.6
	860.0	716.9	822.3	143.1	285.2	39.4
	860.0	710.1	820.7	149.9	287.7	37.1
	860.0	702.3	818.7	157.7	290.2	34.5
	860.0	693.3	814.0	166.7	293.2	30.8
	860.0	690.4	812.4	169.6	294.2	30.0
	900.0	741.7	854.0	158.3	316.2	26.2
	900.0	738.0	853.4	162.0	317.2	25.25
	900.0	734.2	851.6	165.8	318.2	23.4
	950.0	825.8	911.6	124.2	341.2	25.5
	950.0	828.8	910.7	126.2	342.2	24.5
	950.0	818.6	908.5	131.4	344.2	22.0
	950.0	796.1	899.3	153.9	346.2	17.3
	1020.0	935.3	994.0	84.7	375.2	25.0
	1020.0	930.5	990.9	89.5	377.7	22.5
	1020.0	928.2	990.0	91.8	380.2	19.5
	1020.0	905.0	983.2	115.0	381.3	15.7

Total	Capacity Installed KW	Dependable
212.4	38,400	43,700
212.1	39,500	44,700
211.7	40,000	45,100
238.8	45,300	51,800
239.4	46,200	52,700
237.6	46,700	53,000
253.3	49,300	56,500
254.6	49,800	57,000
252.4	50,400	57,200
276.2	51,900	59,300
276.1	52,900	60,500
276.0	54,000	61,700
270.5	55,000	62,600
115.5	19,700	22,300
118.0	20,700	23,500
118.0	21,800	24,700
130.0	22,800	26,000
132.0	23,800	27,200
134.8	24,900	28,400
135.1	25,400	28,900
142.3	25,000	28,700
144.3	25,900	29,700
145.6	26,900	30,900
144.4	27,600	31,600
324.3	58,600	66,600
324.6	59,100	67,100
324.8	59,700	67,700
324.7	60,200	68,200
324.0	60,800	68,700
324.2	61,100	69,100
342.4	65,600	74,300
342.45	65,800	74,600
341.6	66,000	74,800
366.7	70,800	80,600
366.7	71,000	80,900
366.2	71,400	81,100
363.5	71,850	81,600
400.2	77,900	89,000
400.2	78,400	89,700
400.0	78,900	90,200
397.0	79,100	90,300

TABLE 12

TABLE 13

Summary of Selected Preliminary Studies
All Alternative Plans

	Maximum Pool Elevation Ft MSL	Minimum Pool Elevation Ft MSL	Average Pool Elevation Ft MSL	Maximum Drawdown Ft	Design Head Ft	Minimum Pool % Design Head	Maximum Required Discharge CFS
Long	815	643.4	770.5	171.6	768	83.7	1000
	900	734.0	850.5	166.0	848	86.5	1025
	950	796.7	903.5	153.3	901	88.4	1015
	1020	876.0	975.0	144.0	973	90.0	1000
Crater	1020	845.0	970.0	175.0	968	87.3	415
	1120	952.8	1077.6	167.2	1075	88.6	430
	1220	1105.1	1187.5	115.0	1185	93.2	400
Combined Lakes	860	690.4	812.4	169.6	810	84.9	1420
	900	739.5	851.6	160.5	849	86.2	1450
	950	796.1	899.3	153.9	897	88.8	1450
	1020	905.0	983.7	115.0	981	92.7	1410

Project Document Plan

Long	815	647	764.0	168.0	762	84.9	
Crater	1022	820	955.0	202.0	953	86.0	
Totals for Project Document Plan							
Combined Lakes	860						

Waste	Active Storage	Primary	Secondary	Dependable Capacity
AF	AF	KWH	KWH	KW
7000	193000	192.6	19.1	45100
3040	244000	225.2	12.4	53000
2100	278000	242.6	9.8	57200
1850	350000	263.0	7.5	62000
4910	75000	104.8	13.2	24700
1880	98000	122.6	11.4	31600
1500	98000	133.0	11.4	31600
11000	264000	294.2	29.9	68900
6900	285000	318.2	23.4	74800
4020	335000	346.2	17.3	81700
3690	348000	381.3	15.7	90600
10230	186500	180.0	13.9	
8160	84000	100.0	9.4	
<u>18390</u>	<u>270500</u>	<u>280.0</u>	<u>23.3</u>	
11000	264000	294.2	29.9	

TABLE 13

2

TABLE 14
INSTALLATION SCHEDULE

<u>Item</u>	<u>Project and System Operational Conditions</u>		
	<u>1st Stage</u>	<u>2nd Stage</u>	
Period	1970-1971	1972-1978	1979-1980
Units Installed	1	2	3
Rated Capacity KW <u>1/</u>	20,300	40,600	60,900
Plant Capacity KW	23,350	46,700	70,000
Average Prime Energy millions KWH <u>2/</u>	82	161	331
Average Load Factor, percent	50	51	55
Annual Secondary Energy, millions KWH <u>3/</u>	128	114	22.2

1/ Equal sized units

2/ Average is reflected in demand not plant capability

3/ Reflects average available during growth period

NOTE: Above table shown on Chart 3

TABLE 15
LONG RIVER AT LONG LAKE OUTLET
AVERAGE MONTHLY FLOWS IN CFS

<u>Year</u>	<u>Year of Analysis</u>	<u>Jan</u>	<u>Feb</u>	<u>Mar</u>	<u>Apr</u>	<u>May</u>	<u>Jun</u>	<u>Jul</u>	<u>Aug</u>
1916	1983	47	46	47	122	239	816	807	1011
1917	1984	83	123	49	63	316	656	940	1218
1918	1985	92	39	25	67	283	703	1011	1152
1919	1986	197	52	47	118	292	515	816	992
1920	1987	170	90	43	49	222	548	867	1133
1921	1988	64	90	65	104	366	675	804	809
1922	1989	86	28	24	110	382	666	861	992
1923	1990	59	80	110	169	412	684	878	919
1924	1991	74	47	85	127	543	859	1105	992
1927	1992	95	76	59	112	432	843	907	912
1928	1993	176	129	114	207	523	770	1020	838
1929	1994	186	40	99	78	329	780	836	790
1930	1995	19	42	57	128	299	672	850	1020
1931	1996	144	250	49	117	462	908	902	1029
1932	1997	52	52	57	102	337	714	775	828
1933	1998	42	38	39	284	509	568	763	788
1934	1999	24	35	48	90	287	805	833	1173
1935	2000	43	28	55	87	263	604	1143	888
1936	2001	46	38	57	142	478	944	892	797
1937	2002	68	44	63	110	312	905	761	875
1938	2003	127	125	135	93	537	704	839	720
1939	2004	92	75	53	102	338	749	1001	1254
1940	2005	74	109	51	146	501	727	944	1162
1941	2006	44	73	64	195	424	807	954	640
1942	2007	98	95	104	120	408	887	1130	1117
1943	2008	110	61	93	219	425	751	1110	982
1944	2009	100	94	107	123	418	982	882	846
1945	2010	53	42	68	114	507	789	973	768
1946	2011	39	44	53	86	592	872	805	963
1947	2012	61	56	159	177	512	864	805	715
1948	2013	107	59	50	64	570	949	905	799
1949	2014	88	44	56	120	503	705	817	894
1950	2015	31	28	39	66	332	727	859	734
1951	2016	49	43	52	101	412	868	954	637
1952	2017	36	36	47	142	415	692	985	905
1953	2018	55	63	46	80	540	891	877	961
1954	2019	67	260	56	47	303	712	771	596
1955	2020	84	52	57	63	270	633	978	1081
1956	2021	28	28	38	66	462	558	964	1278
1957	2022	115	42	33	82	446	756	784	762
1958	2023	182	66	47	142	518	985	925	934
1959	2024	66	66	57	94	403	855	1143	845
1960	2025	86	52	68	139	446	666	986	919
1961	2026	123	119	94	191	479	960	1282	1400
1962	2027	173	74	87	80	288	753	851	787
1963	2028	137	203	103	109	366	699	892	715
1964		118	102	66	147	295	937	1135	805
Monthly Average		88	74	65	117	405	769	922	922

<u>Sep</u>	<u>Oct</u>	<u>Nov</u>	<u>Dec</u>	
982	572	137	82	
872	616	623	89	
1001	476	323	171	
944	497	181	121	
605	360	247	57	
670	650	196	263	
783	450	494	98	
1077	532	492	209	
1020	467	316	264	
907	328	119	47	
785	496	344	297	
712	1020	459	136	
774	518	529	311	
775	651	171	80	
920	660	115	70	
576	511	425	109	
763	661	238	180	
627	619	174	271	
1018	1486	715	367	
1011	1237	246	163	
1149	720	217	183	
786	808	382	246	
946	689	205	108	
471	624	416	203	
866	786	161	107	
1129	1275	425	358	
658	1018	382	277	
930	1284	144	68	
710	589	357	89	
1133	653	257	209	
1141	489	340	125	
839	521	872	124	
909	282	102	50	
699	293	122	86	
1042	1062	420	114	
832	858	135	131	
858	440	444	346	
798	319	200	52	
631	353	410	328	
967	554	424	112	
501	706	240	135	
551	535	259	190	
925	773	317	278	
679	906	194	53	
042	559	383	284	
168	710	120	206	
462	652	255	190	Annual Average
444	664	313	171	448

TABLE 15

2

TABLE 16
CRATER CREEK AT CRATER LAKE OUTLET
AVERAGE MONTHLY FLOWS IN CFS

<u>Year</u>	<u>Year of Analysis</u>	<u>Jan</u>	<u>Feb</u>	<u>Mar</u>	<u>Apr</u>	<u>May</u>	<u>Jun</u>	<u>Jul</u>	<u>Aug</u>	<u>Sep</u>
1913	1980		47	48	57	203	531	830	858	4
1914	1981	21	45	37	53	144	272	517	409	2
1915	1982	36	17	45	74	235	414	497	469	3
1916	1983	18	18	19	44	90	370	370	464	4
1917	1984	35	45	23	24	142	305	441	539	3
1918	1985	33	17	13	21	129	347	482	591	4
1919	1986	68	15	12	47	118	217	417	511	4
1920	1987	100	35	16	20	53	177	406	532	2
1921	1988	24	31	24	34	138	305	399	360	2
1922	1989	34	10	10	35	145	287	437	471	3
1923	1990	21	28	38	47	160	297	452	483	5
1924	1991	28	17	30	39	229	400	584	566	5
1927	1992	35	27	25	38	161	350	377	357	3
1928	1993	89	31	40	42	193	381	528	377	3
1929	1994	76	19	49	29	92	382	419	404	3
1930	1995	5	9	15	34	104	308	420	484	3
1931	1996	68	102	22	45	211	402	417	474	3
1932	1997	20	20	15	33	105	284	362	366	4
1933	1998	14	13	15	66	211	230	379	367	2
1934	1999	7	12	18	31	100	371	420	565	3
1935	2000	15	10	20	30	90	251	606	418	2
1936	2001	16	13	21	42	194	462	454	371	4
1937	2002	26	16	23	35	112	436	379	411	4
1938	2003	55	43	46	32	225	309	423	333	5
1939	2004	37	26	20	33	124	337	520	608	3
1940	2005	28	38	15	43	205	323	485	560	4
1941	2006	15	25	24	51	166	372	491	293	2
1942	2007	40	33	36	37	158	425	538	536	3
1943	2008	45	21	33	56	166	337	585	466	5
1944	2009	41	33	37	38	163	488	448	396	2
1945	2010	19	15	25	36	209	361	502	357	4
1946	2011	13	15	20	30	255	414	403	457	3
1947	2012	23	19	53	48	212	409	403	330	5
1948	2013	44	20	19	25	243	466	461	372	5
1949	2014	35	16	21	37	207	310	410	421	3
1950	2015	10	10	15	25	122	323	436	339	4
1951	2016	17	15	19	33	160	411	489	291	3
1952	2017	12	13	18	42	161	302	511	426	4
1953	2018	20	22	17	29	227	428	446	456	3
1954	2019	25	89	21	20	108	314	384	271	3
1955	2020	33	18	21	24	93	267	506	518	3
1956	2021	9	10	15	25	185	226	497	621	2
1957	2022	48	15	13	29	177	342	392	354	4
1958	2023	84	23	18	42	216	490	474	442	2
1959	2024	25	23	21	32	156	404	606	396	2
1960	2025	34	18	25	41	177	361	511	434	4
1961	2026	52	41	33	52	194	473	691	684	3
1962	2027	79	26	31	29	101	340	431	367	4
1963	2028	59	70	36	35	138	306	454	330	5
1964		50	35	24	43	104	458	599	375	1
Monthly Average		35	27	25	38	160	355	473	446	3

	<u>Oct</u>	<u>Nov</u>	<u>Dec</u>
1	460	108	38
6	313	104	24
9	185	45	33
0	270	51	33
1	251	250	34
1	202	133	65
0	209	67	45
2	140	92	25
7	290	75	95
2	202	158	41
2	230	198	77
1	197	124	95
2	135	48	25
3	194	113	82
7	463	222	60
9	225	256	146
1	334	73	28
9	316	42	27
2	219	170	43
2	296	92	67
6	274	66	97
7	762	294	127
5	615	96	62
5	327	84	68
3	373	151	89
3	311	79	43
2	276	166	75
3	361	61	42
4	637	170	125
1	489	151	99
4	644	55	28
6	258	141	36
5	291	100	77
0	208	134	48
9	224	361	76
3	110	38	22
1	114	46	35
0	515	167	45
6	402	51	51
9	184	178	120
9	126	77	22
8	142	163	115
4	241	169	44
6	320	93	52
0	231	101	71
2	355	125	99
1	428	74	23
0	243	152	101
3	321	45	76
8	291	99	76
3	300	121	63

Annual Average

203

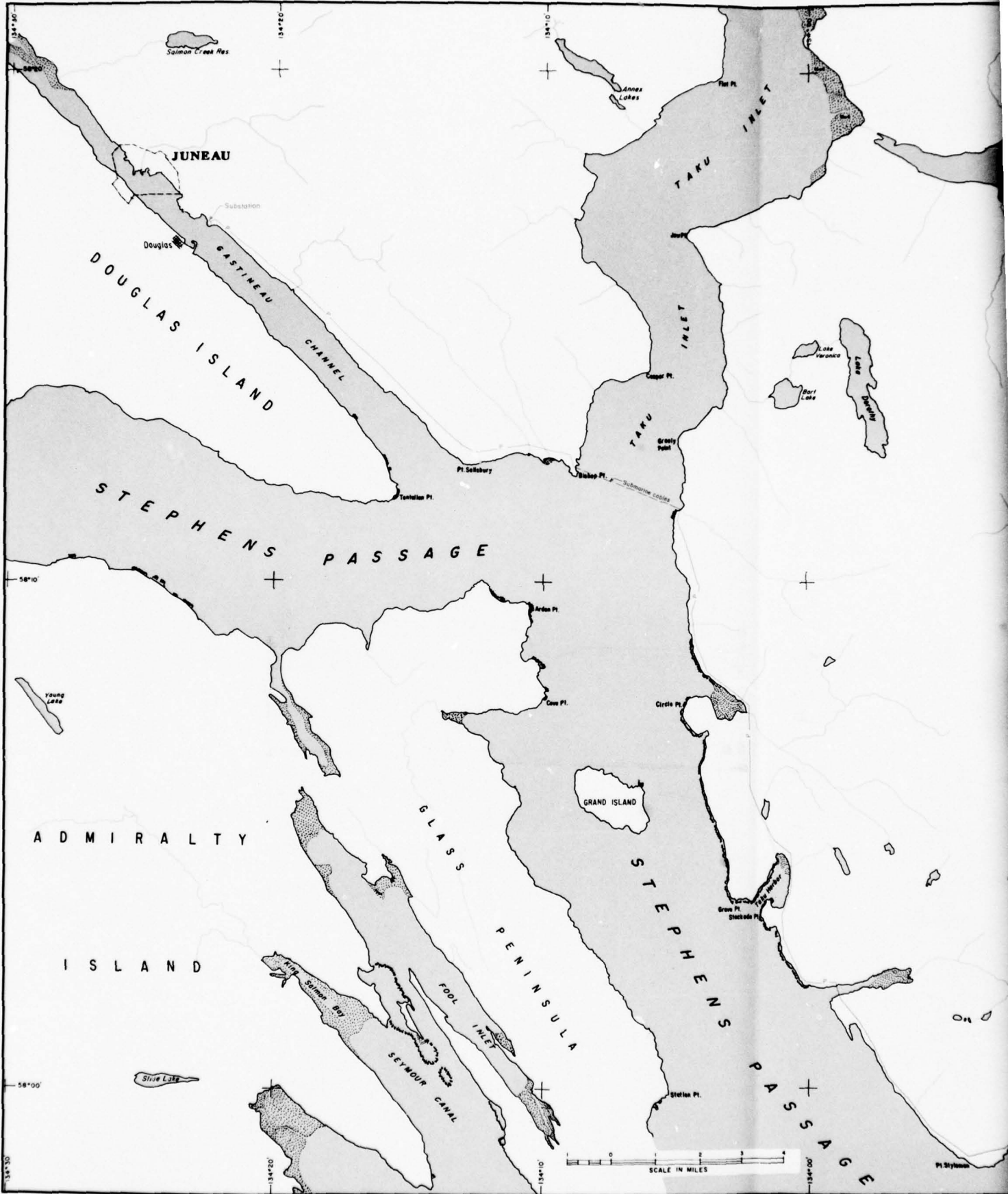
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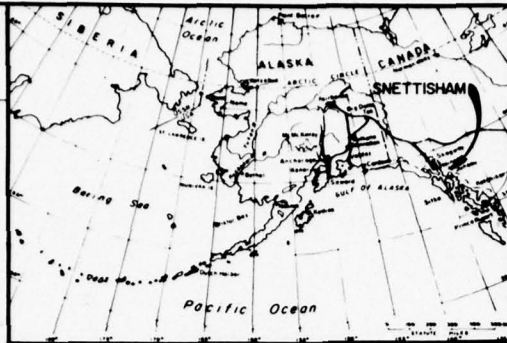
895-66

3

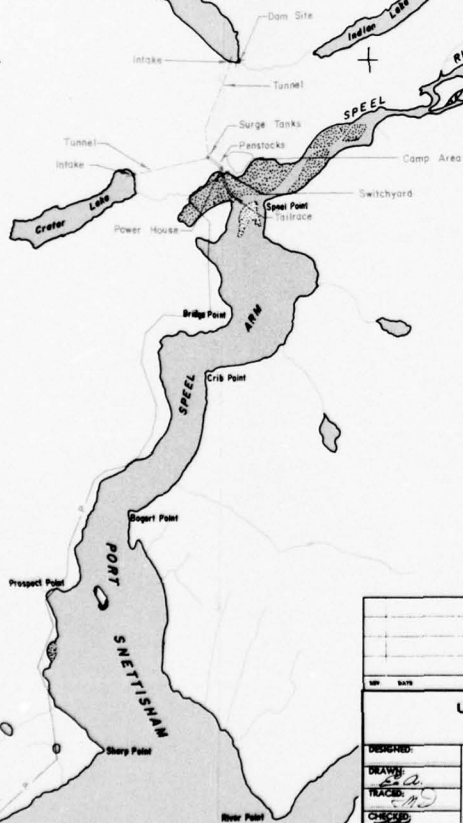
PLATES

CORPS OF ENGINEERS

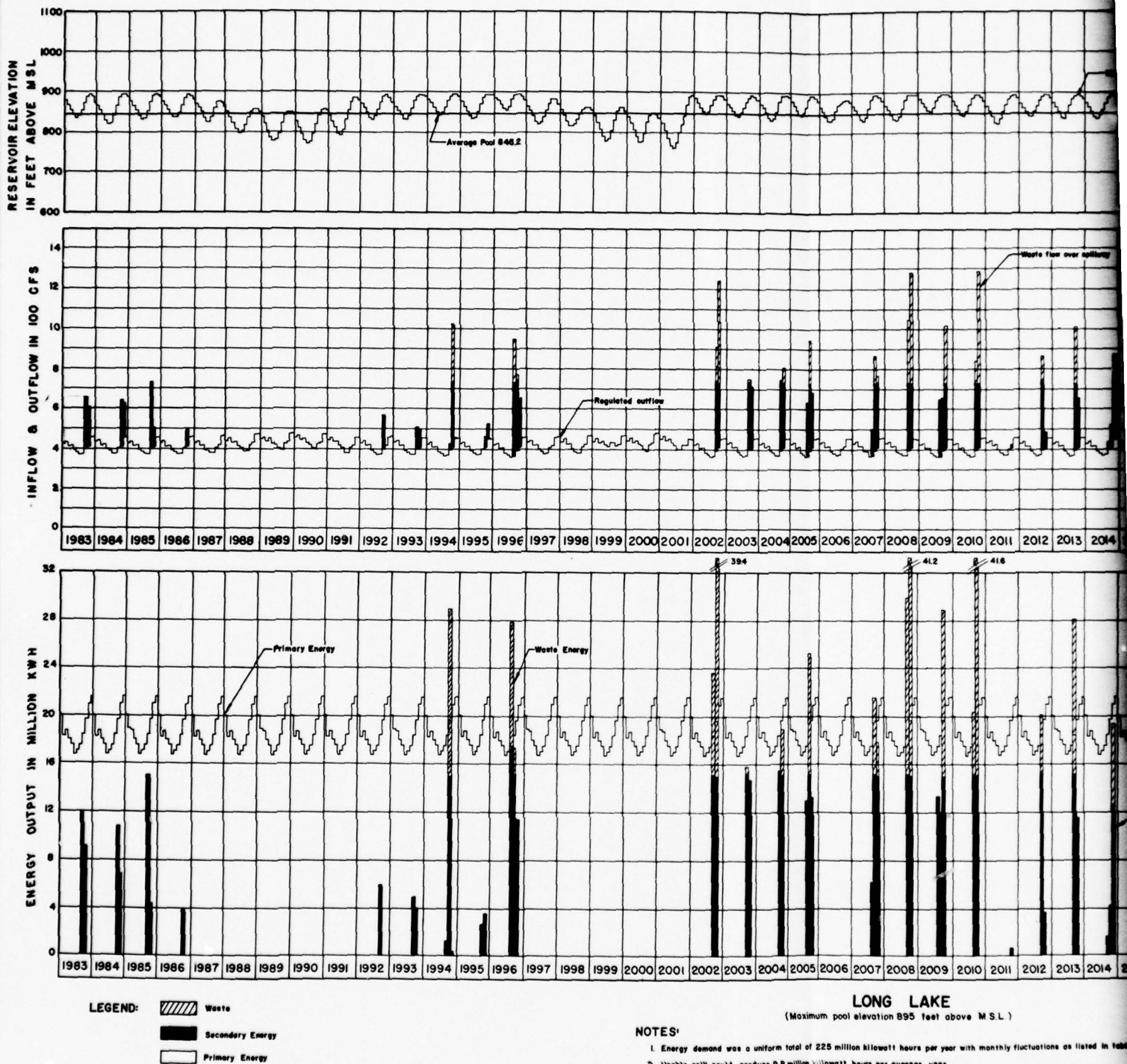




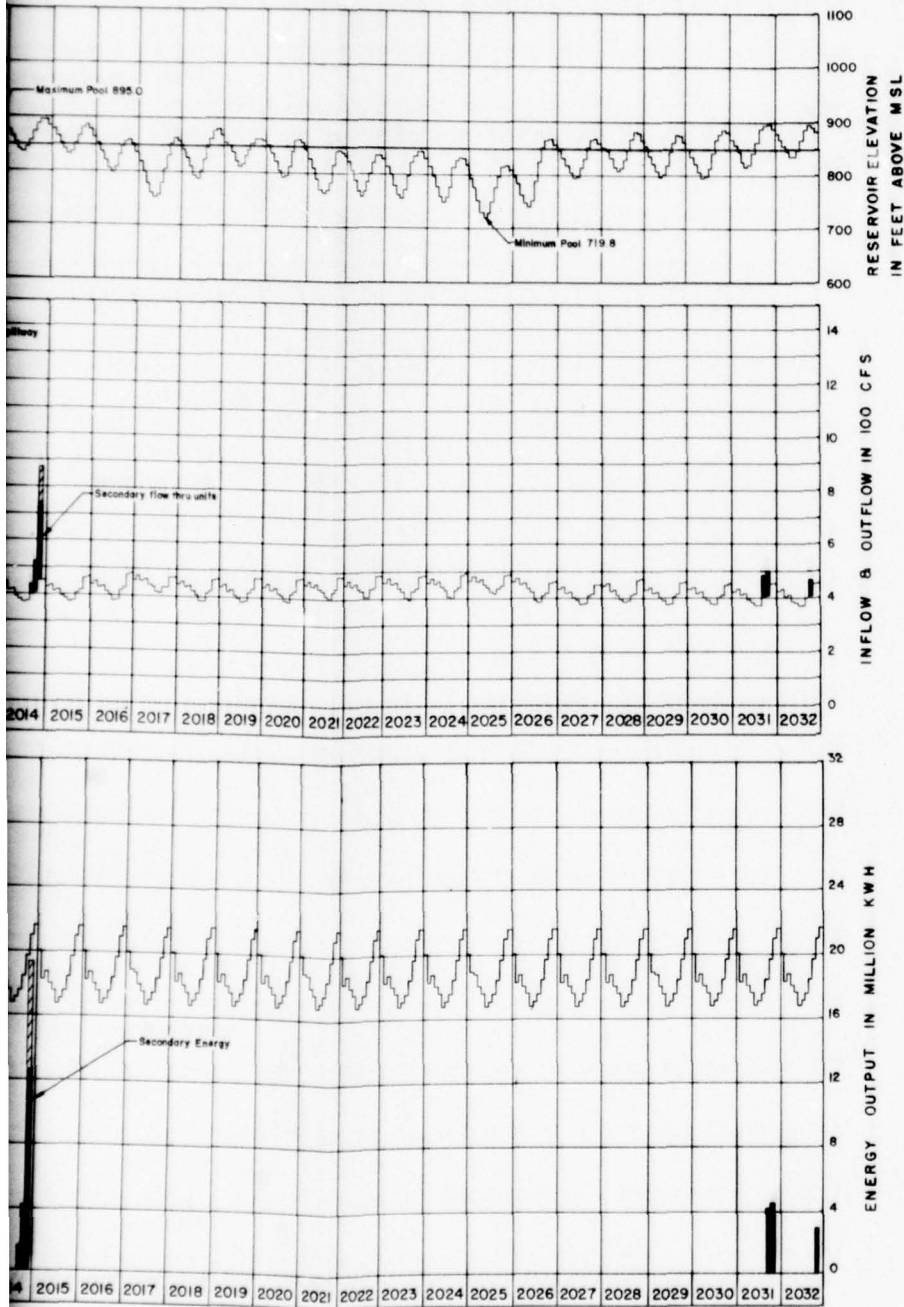
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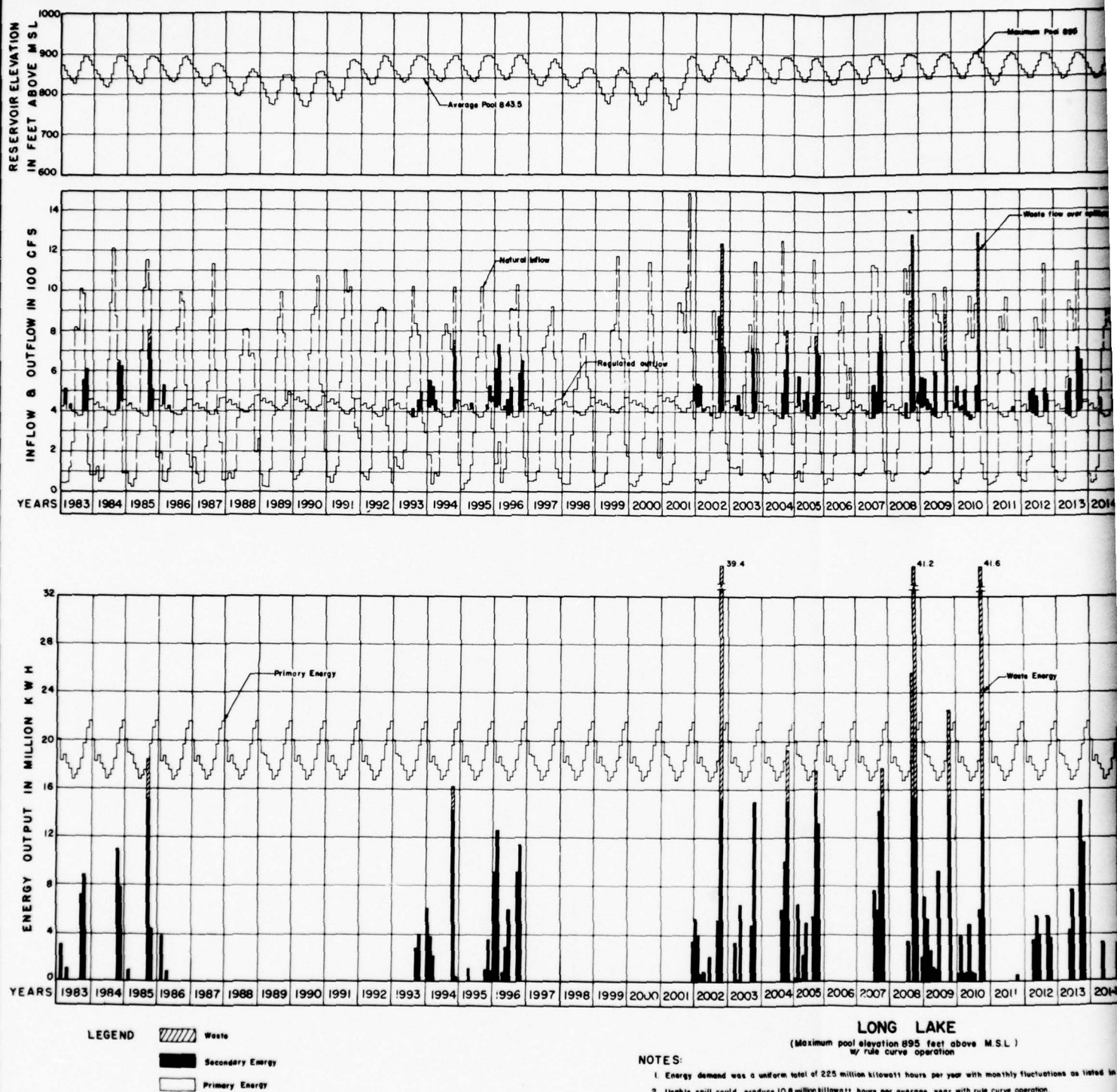
NO.	DATE	DESCRIPTION	BY
U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA			
DESIGNED: <i>[Signature]</i>			
DRAWN: <i>[Signature]</i>			
TRACED: <i>[Signature]</i>			
CHECKED: <i>[Signature]</i>			
PREPARED: <i>[Signature]</i>			
SUPERVISED: <i>[Signature]</i>			
SUBMITTED: <i>[Signature]</i>			
RECOMMENDED: <i>[Signature]</i>			
APPROVED: <i>[Signature]</i>			
SCALE AS SHOWN		DATE 29 SEPT. 64	
FILE NUMBER		0-45-90	
SHEET 1 OF 1			



U. S. ARMY



U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: _____ DRAWN: _____ CHECKED: _____ PREPARED: <u>222</u> SUPERVISED: <u>M. B. Wade Jr.</u> SUBMITTED: <u>[Signature]</u> RECOMMENDED: <u>[Signature]</u>	
SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT LONG LAKE TYPICAL REGULATION RESULTS	
APPROVED: <u>[Signature]</u> DATE: _____ SCALE: _____ FILE: _____ SHEET: <u>Q-4-8-113</u>	DATE: _____ OF: _____



U. S. ARMY

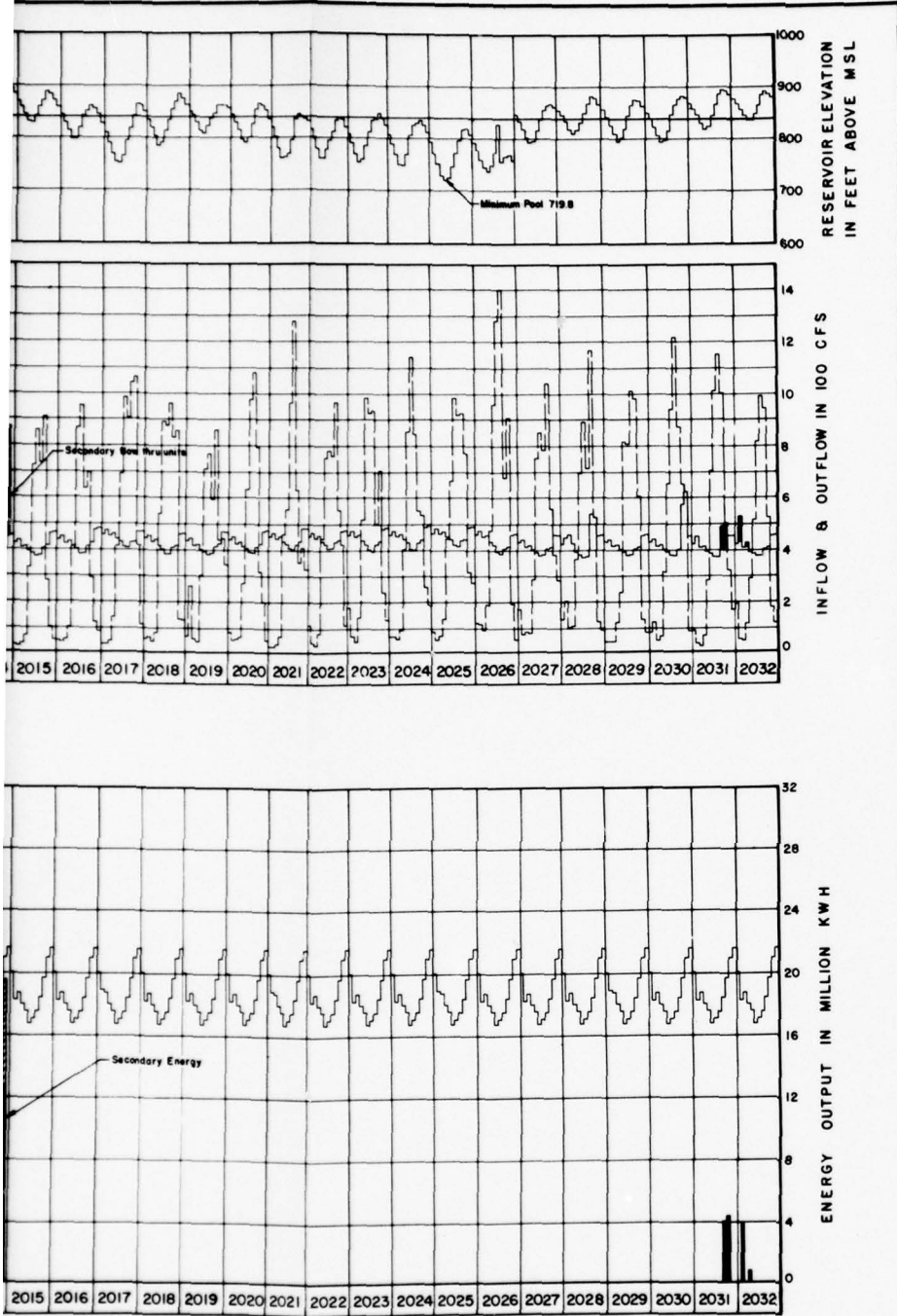
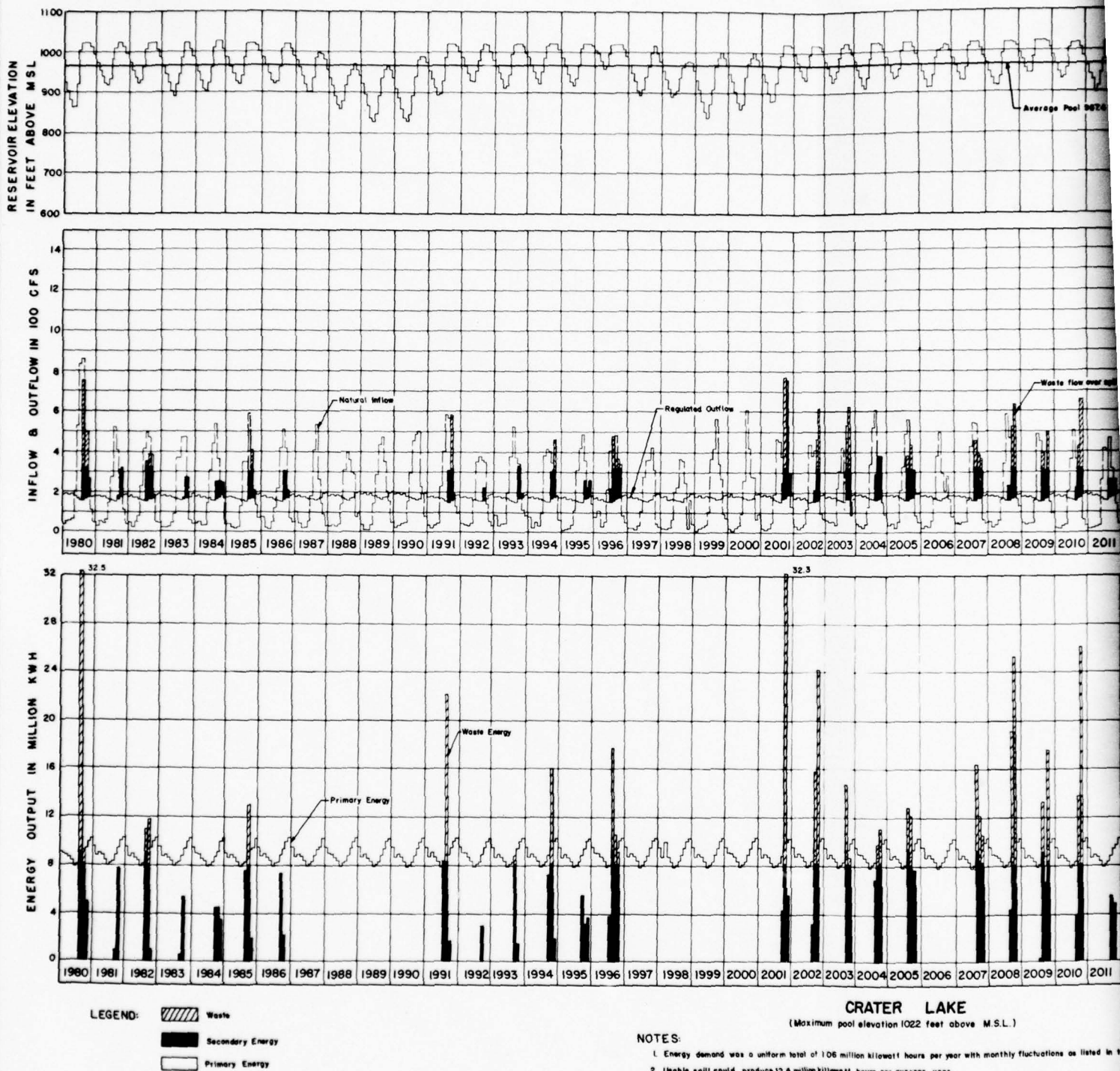
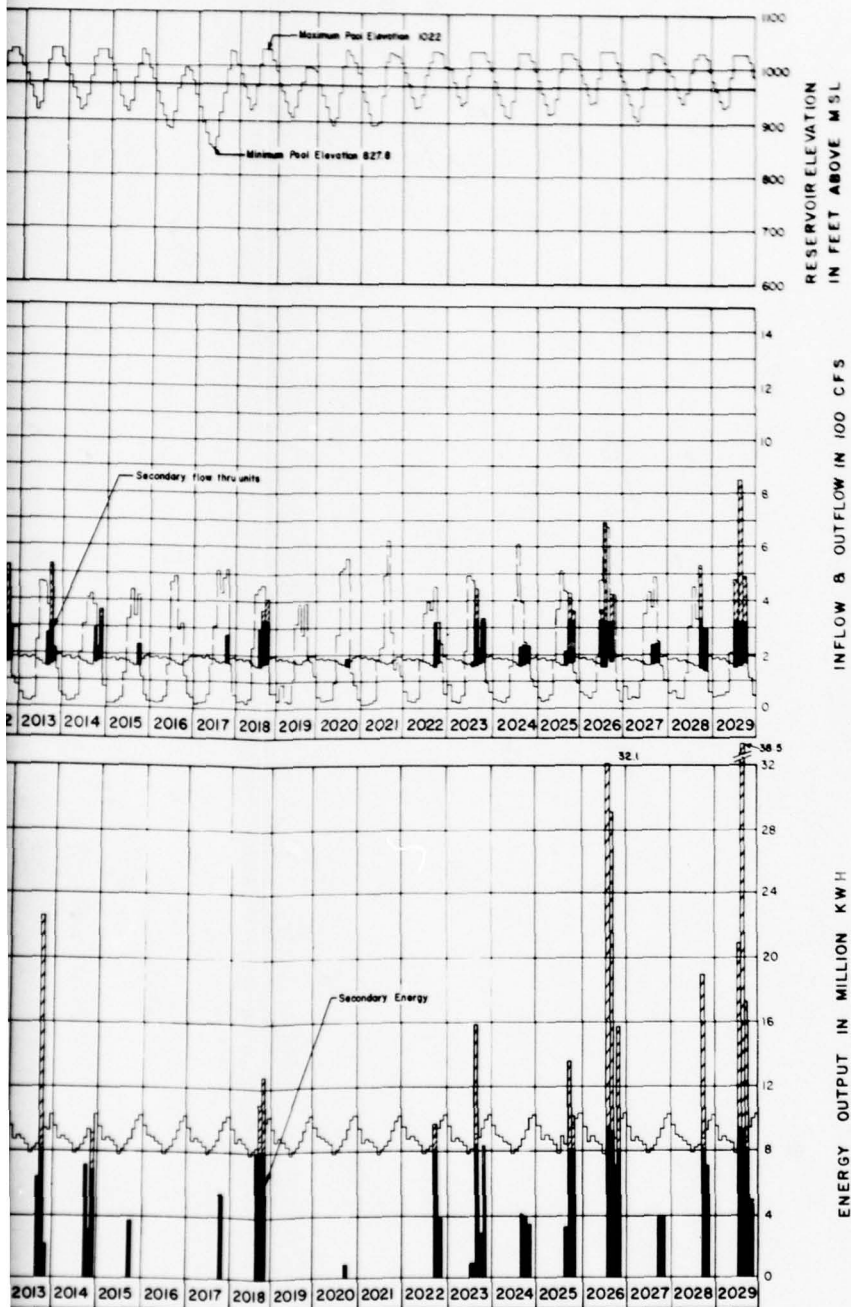


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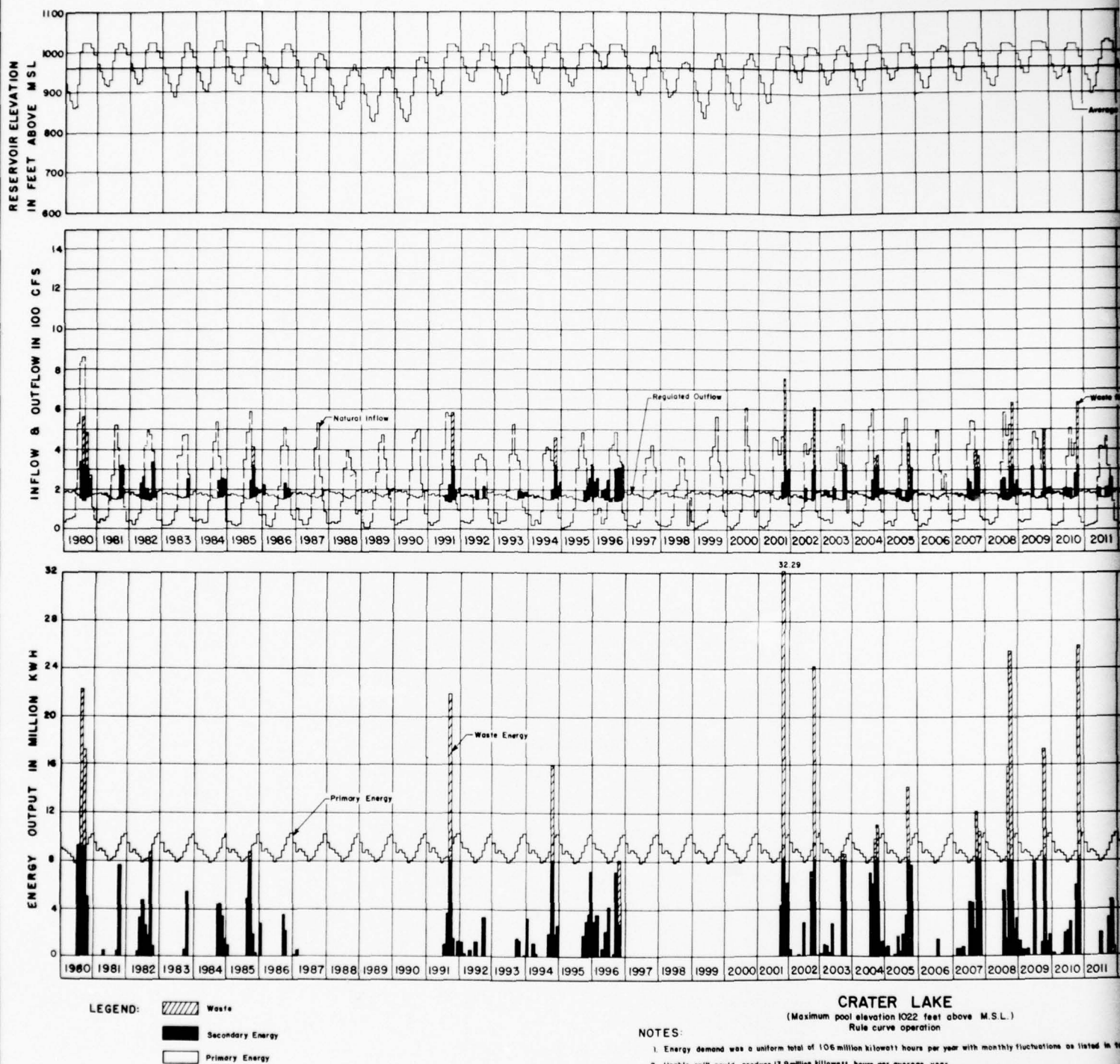
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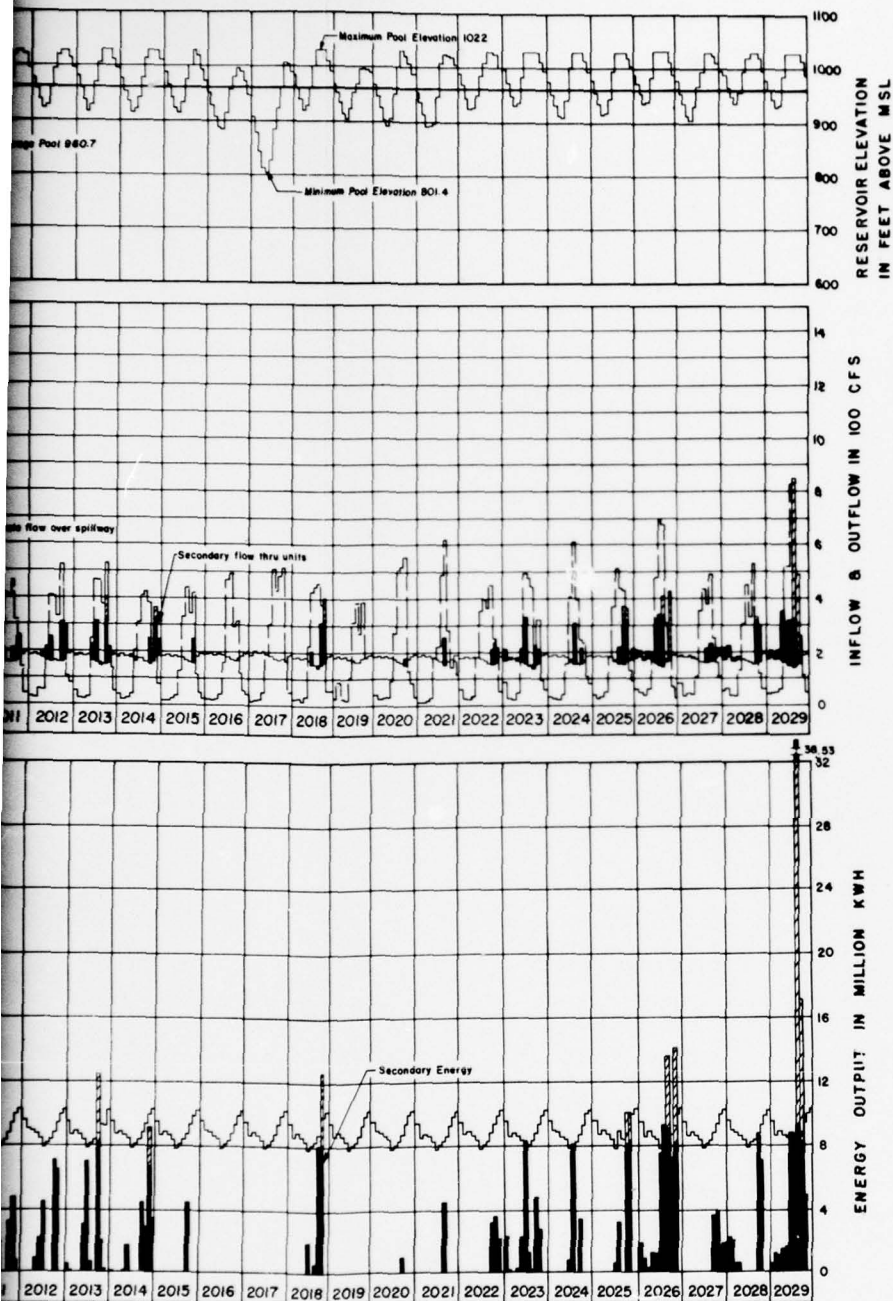
U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: _____ DRAWN: T.S. READER: _____ CHECKED: _____ PREPARED: B.L.L. SUPERVISED: <i>P. DeLada Jr.</i> SUBMITTED: <i>James St. John</i> RECOMMENDED: <i>Robert G. Jones</i>	
SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT LONG LAKE TYPICAL REGULATION RESULTS	
APPROVED: <i>John J. Farley</i> SCALE: AS SHOWN DATE: 31 JULY 1989 FILE NUMBER: Q-4-5-113 SHEET 2 OF 2	





U. S. ARMY ENGINEER DISTRICT ALASKA CORPS OF ENGINEERS ANCHORAGE ALASKA	
DESIGNED:	SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT CRATER LAKE TYPICAL REGULATION RESULTS
DRAWN:	
CHECKED:	
PREPARED:	
SUPERVISED:	
SUBMITTED:	APPROVED:
RECOMMENDED:	SCALE AS SHOWN
DATE: 31 JULY 1985	FILE NO. 04-5-113
SHEET 1 OF 2	





U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: DRAWN: TS CHECKED: PREPARED: <i>ELL</i>	SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT CRATER LAKE TYPICAL REGULATION RESULTS
SUPERVISED: <i>W. C. Edwards Jr</i> CHECKED POWER INVESTIGATING SECTION SUBMITTED: <i>W. C. Edwards Jr</i> UNDER POWER DIV. DIRECTOR RECOMMENDED: <i>James G. Gandy</i> MAJOR, ENGINEER CORPS	APPROVED: <i>Wm. F. Farley</i> CHIEF, CIVIL DISTRICT ENGINEER SCALE AS SHOWN DATE 31 JULY 1960 FILE NUMBER Q-4-5-113 SHEET 2 OF 2

STORAGE EOM	SECONDARY ENERGY		PRIMARY POWER		WASTE WATER	TAIL WATER
AF	SPILL AF	KWH	ENERGY KWH	KW	KWH/AF	AF
1001						
011284.	0.	0.	20043300.	26940.	62.06 750.	0.
019019	0.	0.	18265400.	27180.	61.36 742.	0.
021281	0.	0.	18783000.	25246.	60.56 736.	0.
026684	0.	0.	18041100.	25057.	59.90 729.	0.
036521	0.	0.	17591400.	23644.	59.77 723.	0.
046425	0.	0.	16781000.	23307.	60.51 732.	0.
053905	0.	0.	17096000.	22978.	61.04 740.	0.
067890	0.	0.	17546200.	23583.	63.20 764.	0.
084000.	6797.	1288570.	18446000.	25619.	64.10 775.	0.
094000.	5149.	4021980.	19705200.	26485.	64.29 778.	0.
107400.	0.	0.	21032600.	29212.	64.23 777.	0.
107743.	0.	0.	21618100.	29056.	63.97 774.	0.
1094						
012079	0.	0.	20043300.	26940.	63.46 767.	0.
031249	0.	0.	18265400.	27180.	62.76 759.	0.
042279	0.	0.	18783000.	25246.	61.96 749.	0.
052512	0.	0.	18041100.	25057.	61.13 739.	0.
062794	0.	0.	17591400.	23644.	60.65 733.	0.
072499	0.	0.	16781000.	23307.	61.07 739.	0.
081174	0.	0.	17096000.	22978.	62.16 752.	0.
096614	0.	0.	17546200.	23583.	63.22 765.	0.
104000.	1149.	1065170.	18446000.	25619.	64.03 774.	0.
104000.	19350.	15032800.	19705200.	26485.	64.21 777.	18011. 2.0
104000.	280.	217888.	21032600.	29212.	64.30 778.	0.
104428.	0.	0.	21618100.	29056.	63.95 773.	0.
1095						
019757.	0.	0.	20043300.	26940.	63.12 763.	0.
037426	0.	0.	18265400.	27180.	62.21 752.	0.
056420	0.	0.	18783000.	25246.	61.33 742.	0.
075977	0.	0.	18041100.	25057.	60.51 732.	0.
092728	0.	0.	17591400.	23644.	59.96 725.	0.
106677	0.	0.	16781000.	23307.	60.19 728.	0.
108655	0.	0.	17096000.	22978.	61.22 740.	0.
109425	0.	0.	17546200.	23583.	62.63 757.	0.
109402	0.	0.	18446000.	25619.	63.83 772.	0.
109400.	3113.	2422410.	19705200.	26485.	64.27 777.	0.
109400.	4442.	1456130.	21032600.	29212.	64.29 777.	0.
109400.	0.	0.	21618100.	29056.	64.14 776.	0.
1096						
010122.	0.	0.	20043300.	26940.	63.70 770.	0.
027754	0.	0.	18917700.	27180.	63.17 764.	0.
035911	0.	0.	18783000.	25246.	62.55 756.	0.
03764	0.	0.	18041100.	25057.	61.74 747.	0.
043543	0.	0.	17591400.	23644.	61.93 744.	0.
0505300.	0.	0.	16781000.	23307.	62.26 753.	0.
057632	0.	0.	17096000.	22978.	63.56 769.	0.
064000.	22129.	17191800.	17546200.	23583.	64.17 776.	13048. 2.0
064000.	19528.	15171400.	18446000.	25619.	64.21 777.	2851. 2.0
064000.	14672.	11405900.	19705200.	26485.	64.23 777.	0.
07017.	0.	0.	21032600.	29212.	64.00 774.	0.
083693.	0.	0.	21618100.	29056.	63.25 765.	0.
1097						
010326.	0.	0.	20043300.	26940.	62.35 754.	0.
026951	0.	0.	18265400.	27180.	61.45 743.	0.
035049	0.	0.	18783000.	25246.	60.52 732.	0.
037326	0.	0.	18041100.	25057.	59.51 720.	0.
039377	0.	0.	17591400.	23644.	58.93 713.	0.
042674	0.	0.	16781000.	23307.	59.29 717.	0.
049424	0.	0.	17096000.	22978.	60.37 730.	0.
052598	0.	0.	17546200.	23583.	61.50 744.	0.
054476	0.	0.	18446000.	25619.	62.46 758.	0.
05628	0.	0.	19705200.	26485.	63.54 768.	0.
05660	0.	0.	21032600.	29212.	63.41 767.	0.
06001.	0.	0.	21618100.	29056.	62.54 756.	0.
1098						
010486	0.	0.	20043300.	26940.	61.58 745.	0.
02583.	0.	0.	18265400.	27180.	60.56 733.	0.
03971.	0.	0.	18783000.	25246.	59.45 719.	0.
05424	0.	0.	18041100.	25057.	58.59 708.	0.
06077	0.	0.	17591400.	23644.	58.54 708.	0.
062187	0.	0.	16781000.	23307.	59.32 714.	0.
06468	0.	0.	17096000.	22978.	59.83 723.	0.
06145	0.	0.	17546200.	23583.	60.94 737.	0.
0704.	0.	0.	18446000.	25619.	61.48 746.	0.
0846.	0.	0.	19705200.	26485.	61.97 749.	0.
09119.	0.	0.	21032600.	29212.	62.04 750.	0.
09776.	0.	0.	21618100.	29056.	61.51 744.	0.
1099						
01067.	0.	0.	20043300.	26940.	60.48 731.	0.
02578.	0.	0.	18265400.	27180.	59.23 716.	0.
03676.	0.	0.	18783000.	25246.	57.91 700.	0.
04560.	0.	0.	18041100.	25057.	56.52 684.	0.
0558.	0.	0.	17591400.	23644.	55.60 672.	0.
06109.	0.	0.	16781000.	23307.	54.03 678.	0.
0792.	0.	0.	17096000.	22978.	57.58 696.	0.
0804.	0.	0.	17546200.	23583.	59.44 721.	0.
09123.	0.	0.	18446000.	25619.	61.26 741.	0.
0960.	0.	0.	19705200.	26485.	61.97 749.	0.
0999.	0.	0.	21032600.	29212.	61.96 749.	0.
1025.	0.	0.	21618100.	29056.	61.30 741.	0.
1100						
01791.	0.	0.	20043300.	26940.	60.41 731.	0.
0291.	0.	0.	18917700.	27180.	59.15 715.	0.
0391.	0.	0.	18783000.	25246.	57.78 699.	0.
0494.	0.	0.	18041100.	25057.	56.40 682.	0.
0529.	0.	0.	17591400.	23644.	55.41 670.	0.
0532.	0.	0.	16781000.	23307.	55.38 670.	0.
0589.	0.	0.	17096000.	22978.	57.14 691.	0.
0757.	0.	0.	17546200.	23583.	59.35 719.	0.
0866.	0.	0.	18446000.	25619.	60.44 731.	0.
0909.	0.	0.	19705200.	26485.	60.96 737.	0.
0942.	0.	0.	21032600.	29212.	60.80 735.	0.
0981.	0.	0.	21618100.	29056.	60.06 726.	0.
1001						
0101.	0.	0.	20043300.	26940.	59.09 715.	0.
0277.	0.	0.	17546200.	23583.	57.49 698.	0.
0313.	0.	0.	18783000.	25246.	56.20 680.	0.
0356.	0.	0.	18041100.	25057.	54.80 663.	0.
0406.	0.	0.	17591400.	23644.	54.23 656.	0.
0433.	0.	0.	17096000.	22978.	55.30 669.	0.
0457.	0.	0.	17546200.	23583.	56.89 712.	0.
0495.	0.	0.	18446000.	25619.	60.40 730.	0.
0503.	0.	0.	19705200.	26485.	62.51 756.	0.
0509.	49.	38130.	21032600.	29212.	64.04 774.	0.
0508.	0.	0.	21618100.	29056.	64.22 777.	0.
1002						
0103.	0.	0.	20043300.	26940.	63.76 771.	0.
0204.	0.	0.	18265400.	27180.	62.81 761.	0.
0270.	0.	0.	18783000.	25246.	62.07 751.	0.
0377.	0.	0.	18041100.	25057.	61.26 741.	0.
0407.	0.	0.	17591400.	23644.	60.84 736.	0.
0458.	0.	0.	16781000.	23307.	61.36 742.	0.
051.	0.	0.	17096000.	22978.	62.49 756.	0.
0509.	0.	0.	17546200.	23583.	63.59 769.	0.
0509.	19528.	15171400.	18446000.	25619.	64.17 776.	11044. 2.0
0509.	19350.	15032800.	19705200.	26485.	64.21 777.	31354. 2.0
0515.	0.	0.	21032600.	29212.	64.08 775.	0.
0517.	0.	0.	21618100.	29056.	63.55 768.	0.

DESIGNED:		SCALE: NONE		DATE: AUGUST 1968	
DRAWN:		FILE NUMBER:		SHEET 1 OF 3	
CHECKED:		DATE: AUGUST 1968		FILE NUMBER: Q-4-5-113	
APPROVED:		DATE: AUGUST 1968		FILE NUMBER: Q-4-5-113	
SUBMITTED:		DATE: AUGUST 1968		FILE NUMBER: Q-4-5-113	
RECEIVED:		DATE: AUGUST 1968		FILE NUMBER: Q-4-5-113	

LONG LAKE

FORAGE EOM	SECONDARY ENERGY	PRIMARY POWER	WASTE AF	TAIL WATER				
AF	SPILL AF	KWH	KWH/AF	AF				
2013	0.	0.	20043300.	26940.	62.96	761.	0.	4
27990	0.	0.	18265400.	27180.	62.13	751.	0.	4
18075	0.	0.	18783000.	25246.	61.26	741.	0.	4
93811	0.	0.	18041100.	25057.	60.33	730.	0.	4
72908	0.	0.	17591400.	23644.	60.04	726.	0.	4
83744	0.	0.	17810100.	23307.	61.04	738.	0.	4
17496	0.	0.	17096000.	22978.	62.45	755.	0.	4
90518	0.	0.	17546200.	23583.	63.62	769.	0.	4
76955	0.	0.	18446000.	25619.	63.82	776.	17467.	2.0
94000.	14928.	15171400.	19705200.	26485.	64.23	777.	0.	1.4
94000.	14795.	11501400.	21032600.	29212.	64.06	775.	0.	5
72162	0.	0.	21618100.	29056.	63.61	769.	0.	5
94000.	0.	0.	20043300.	26940.	62.92	761.	0.	4
2014	0.	0.	18265400.	27180.	62.87	761.	0.	4
14142.	0.	0.	18783000.	25246.	61.15	749.	0.	4
92203.	0.	0.	18041100.	25057.	60.36	730.	0.	4
76435	0.	0.	17591400.	23644.	58.73	726.	0.	4
81345	0.	0.	16781000.	23307.	60.44	733.	0.	4
90428	0.	0.	17096000.	22978.	61.65	746.	0.	4
97636	0.	0.	17546200.	23583.	62.86	760.	0.	4
94000.	1702.	1324510.	18446000.	25619.	63.92	773.	0.	4
94000.	6706.	5217680.	19705200.	26485.	64.29	777.	0.	4
13684.	16199.	12586800.	21032600.	29212.	64.21	777.	8624.	2.0
94000.	0.	0.	21618100.	29056.	63.96	773.	0.	5
2015	0.	0.	20043300.	26940.	63.11	763.	0.	4
99345.	0.	0.	18265400.	27180.	62.19	752.	0.	4
16630.	0.	0.	18783000.	25246.	61.27	741.	0.	4
94093	0.	0.	18041100.	25057.	60.33	730.	0.	4
72908	0.	0.	17591400.	23644.	59.72	722.	0.	4
68979.	0.	0.	16781000.	23307.	60.09	727.	0.	4
99160	0.	0.	17096000.	22978.	61.21	740.	0.	4
18096	0.	0.	17546200.	23583.	62.28	753.	0.	4
80745	0.	0.	18446000.	25619.	63.28	765.	0.	4
90767	0.	0.	19705200.	26485.	63.67	770.	0.	4
67512	0.	0.	21032600.	29212.	63.12	763.	0.	5
11045	0.	0.	21618100.	29056.	62.20	752.	0.	5
15396	0.	0.	20043300.	26940.	61.19	760.	0.	4
2016	0.	0.	18265400.	27180.	60.14	727.	0.	4
93395	0.	0.	18783000.	25246.	58.46	713.	0.	4
17019	0.	0.	18041100.	25057.	57.49	698.	0.	4
16681	0.	0.	17591400.	23644.	57.09	690.	0.	4
10847	0.	0.	16781000.	23307.	57.88	700.	0.	4
10718	0.	0.	17096000.	22978.	59.61	721.	0.	4
12008	0.	0.	17546200.	23583.	60.83	736.	0.	4
17367	0.	0.	18446000.	25619.	61.53	746.	0.	4
12099	0.	0.	19705200.	26485.	61.69	746.	0.	4
9517	0.	0.	21032600.	29212.	61.65	738.	0.	5
1137	0.	0.	21618100.	29056.	60.06	726.	0.	5
9927	0.	0.	20043300.	26940.	58.75	710.	0.	4
9462	0.	0.	18265400.	27180.	57.21	692.	0.	4
2017	0.	0.	18783000.	25246.	55.60	673.	0.	4
9667	0.	0.	18041100.	25057.	54.23	656.	0.	4
9085	0.	0.	17591400.	23644.	53.47	647.	0.	4
3100	0.	0.	16781000.	23307.	53.94	652.	0.	4
1060	0.	0.	17096000.	22978.	55.60	672.	0.	4
9390	0.	0.	17546200.	23583.	57.66	698.	0.	4
9977	0.	0.	18446000.	25619.	58.55	720.	0.	4
9919	0.	0.	19705200.	26485.	61.28	741.	0.	4
7923	0.	0.	21032600.	29212.	62.06	751.	0.	5
9051	0.	0.	21618100.	29056.	61.53	744.	0.	5
2018	0.	0.	20043300.	26940.	60.56	732.	0.	4
9048	0.	0.	18265400.	27180.	59.44	719.	0.	4
915	0.	0.	18783000.	25246.	58.17	703.	0.	4
9008	0.	0.	18041100.	25057.	56.79	687.	0.	4
9909	0.	0.	17591400.	23644.	56.33	681.	0.	4
1077	0.	0.	16781000.	23307.	57.43	694.	0.	4
9008	0.	0.	17096000.	22978.	59.14	715.	0.	4
9909	0.	0.	17546200.	23583.	60.75	734.	0.	4
1077	0.	0.	18446000.	25619.	62.02	750.	0.	4
7222	0.	0.	19705200.	26485.	63.04	762.	0.	5
7935	0.	0.	21032600.	29212.	63.17	764.	0.	5
9064	0.	0.	21618100.	29056.	62.36	754.	0.	5
909	0.	0.	20043300.	26940.	61.53	746.	0.	4
929	0.	0.	18265400.	27180.	60.96	736.	0.	4
967	0.	0.	18783000.	25246.	60.07	726.	0.	4
173	0.	0.	18041100.	25057.	58.92	713.	0.	4
968	0.	0.	17591400.	23644.	58.09	702.	0.	4
979	0.	0.	16781000.	23307.	58.41	706.	0.	4
996	0.	0.	17096000.	22978.	59.57	720.	0.	4
104	0.	0.	17546200.	23583.	60.50	732.	0.	4
102	0.	0.	18446000.	25619.	61.32	742.	0.	4
197	0.	0.	19705200.	26485.	61.87	748.	0.	4
129	0.	0.	21032600.	29212.	61.88	748.	0.	5
979	0.	0.	21618100.	29056.	61.67	746.	0.	5
10	0.	0.	20043300.	26940.	61.02	738.	0.	4
994	0.	0.	18265400.	27180.	60.02	726.	0.	4
122	0.	0.	18783000.	25246.	58.83	711.	0.	4
109	0.	0.	18041100.	25057.	57.51	697.	0.	4
911	0.	0.	17591400.	23644.	56.54	67.	0.	4
95	0.	0.	16781000.	23307.	56.62	67.	0.	4
106	0.	0.	17096000.	22978.	58.11	701.	0.	4
100	0.	0.	17546200.	23583.	60.22	728.	0.	4
100	0.	0.	18446000.	25619.	61.66	746.	0.	4
135	0.	0.	19705200.	26485.	61.98	750.	0.	4
95	0.	0.	21032600.	29212.	61.48	744.	0.	5
96	0.	0.	21618100.	29056.	60.99	733.	0.	5
11	0.	0.	20043300.	26940.	59.29	717.	0.	4
97	0.	0.	18265400.	27180.	57.88	700.	0.	4
99	0.	0.	18783000.	25246.	56.36	681.	0.	4
94	0.	0.	18041100.	25057.	54.78	662.	0.	4
90	0.	0.	17591400.	23644.	54.03	653.	0.	4
94	0.	0.	16781000.	23307.	54.29	656.	0.	4
97	0.	0.	17096000.	22978.	55.70	675.	0.	4
10	0.	0.	17546200.	23583.	57.10	691.	0.	4
10	0.	0.	18446000.	25619.	58.75	710.	0.	4
10	0.	0.	19705200.	26485.	59.76	725.	0.	4
10	0.	0.	21032600.	29212.	59.85	726.	0.	4
10	0.	0.	21618100.	29056.	59.17	716.	0.	4
2019	0.	0.	20043300.	26940.	58.96	713.	0.	4
999	0.	0.	18265400.	27180.	57.63	697.	0.	4
172	0.	0.	18783000.	25246.	56.10	678.	0.	4
106	0.	0.	18041100.	25057.	54.52	659.	0.	4
99	0.	0.	17591400.	23644.	53.75	650.	0.	4
95	0.	0.	16781000.	23307.	54.39	658.	0.	4
10	0.	0.	17096000.	22978.	55.70	675.	0.	4
10	0.	0.	17546200.	23583.	57.10	691.	0.	4
10	0.	0.	18446000.	25619.	58.75	710.	0.	4
10	0.	0.	19705200.	26485.	59.76	725.	0.	4
10	0.	0.	21032600.	29212.	59.85	726.	0.	4
10	0.	0.	21618100.	29056.	59.17	716.	0.	4
2020	0.	0.	20043300.	26940.	58.96	713.	0.	4
999	0.	0.	18265400.	27180.	57.63	697.	0.	4
172	0.	0.	18783000.	25246.	56.10	678.	0.	4
106	0.	0.	18041100.	25057.	54.52	659.	0.	4
99	0.	0.	17591400.	23644.	53.75	650.	0.	4
95	0.	0.	16781000.	23307.	54.39	658.	0.	4
10	0.	0.	17096000.	22978.	55.70	675.	0.	4
10	0.	0.	17546200.	23583.	57.10	691.	0.	4
10	0.	0.	18446000.	25619.	58.75	710.	0.	4
10	0.	0.	19705200.	26485.	59.76	725.	0.	4
10	0.	0.	21032600.	29212.	59.85	726.	0.	4
10	0.	0.	21618100.	29056.	59.17	716.	0.	4
2021	0.	0.	20043300.	26940.	58.96	713.	0.	4
999	0.	0.	18265400.	27180.	57.63	697.	0.	4
172	0.	0.	18783000.	25246.	56.10	678.	0.	4
106	0.	0.	18041100.	25057.	54.52	659.	0.	4
99	0.	0.	17591400.	23644.	53.75	650.	0.	4
95	0.	0.	16781000.	23307.	54.39	658.	0.	4
10	0.	0.	17096000.	22978.	55.70	675.	0.	4
10	0.	0.	17546200.	23583.	57.10	691.	0.	4
10	0.	0.	18446000.	25619.	58.75	710.	0.	4
10	0.	0.	19705200.	26485.	59.76	725.	0.	4
10	0.	0.	21032600.	29212.	59.85	726.	0.	4
10	0.	0.	21618100.	29056.	59.17	716.	0.	4

DESIGNED		DATE		APPROVED		BY	
<p align="center">U.S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA</p>							
DRAWN				<p align="center">SNETTISHAM, ALASKA</p>			
CHECKED				<p align="center">LONG LAKE</p>			
<p align="center">MONTHLY REGULATION COMPUTATIONS</p>							
SUPERVISOR		DATE		SCALE		DATE	
SUBMITTER		DATE		SCALE		DATE	
FILE NUMBER		DATE		SCALE		DATE	
FILE NUMBER		DATE		SCALE		DATE	
FILE NUMBER		DATE		SCALE		DATE	

FLOW	ELEVATION	EOM	HEAD	STORAGE	EOM	SECONDARY	ENERGY	PRIMARY	POWER	KWH/AF
NATL	REG	AVERAGE	EOM	AF	SPILL	ENERGY	ENERGY	POWER	KW/AF	
CFS	CFS	FT/MSL	FT/MSL	FT	AF	AF	KWH	KW	KW/AF	
YEAR OF STUDY 2023										
JAN 182.	465.	809.87	801.19	805.37	331816.	0.	0.	20043300.	26940.	58.04 702.
FEB 86.	465.	794.24	785.32	793.44	308027.	0.	0.	18245400.	27180.	56.80 687.
MAR 47.	456.	774.81	764.50	774.28	283734.	0.	0.	18783000.	25246.	55.27 668.
APR 142.	465.	758.09	747.88	755.53	264458.	0.	0.	18041100.	25057.	53.77 650.
MAY 918.	465.	749.77	739.46	749.27	248999.	0.	0.	17591400.	23644.	53.23 644.
JUN 985.	427.	765.89	750.12	765.40	302144.	0.	0.	16781000.	23307.	54.50 659.
JUL 925.	405.	792.55	805.06	792.12	334090.	0.	0.	17096000.	22778.	56.62 685.
AUG 934.	407.	818.83	824.59	818.35	366751.	0.	0.	17544200.	23583.	58.54 708.
SEP 501.	429.	829.91	831.23	829.42	370882.	0.	0.	18446000.	25619.	59.59 721.
OCT 704.	440.	836.43	841.63	835.93	387209.	0.	0.	19705200.	26485.	60.08 727.
NOV 240.	465.	856.96	852.29	854.35	372673.	0.	0.	21032600.	29212.	60.15 727.
DEC 135.	490.	825.00	817.71	824.39	350066.	0.	0.	21618100.	29056.	59.22 714.
YEAR OF STUDY 2024										
JAN 182.	465.	808.24	798.76	807.64	332455.	0.	0.	20043300.	26940.	57.91 700.
FEB 86.	465.	789.51	780.27	788.91	302354.	0.	0.	18917700.	27180.	56.42 682.
MAR 57.	465.	769.53	758.80	768.99	277563.	0.	0.	18783000.	25246.	56.86 683.
APR 94.	470.	749.66	740.12	748.88	255147.	0.	0.	18041100.	25057.	53.23 644.
MAY 403.	451.	738.87	737.62	738.35	252147.	0.	0.	17591400.	23644.	53.23 644.
JUN 855.	439.	747.92	758.23	747.41	276880.	0.	0.	16781000.	23307.	53.04 641.
JUL 1143.	416.	776.85	785.47	776.37	321657.	0.	0.	17096000.	22778.	55.39 670.
AUG 845.	408.	805.76	816.05	805.28	368475.	0.	0.	17544200.	23583.	57.68 697.
SEP 551.	436.	818.49	820.93	817.99	355109.	0.	0.	18446000.	25619.	58.74 710.
OCT 535.	448.	822.83	824.74	822.32	360640.	0.	0.	19705200.	26485.	59.08 714.
NOV 259.	490.	819.64	814.58	819.02	366497.	0.	0.	21032600.	29212.	58.81 711.
DEC 190.	502.	807.03	799.48	806.39	327277.	0.	0.	21618100.	29056.	57.81 699.
YEAR OF STUDY 2025										
JAN 86.	476.	790.21	780.99	789.62	303230.	0.	0.	20043300.	26940.	56.48 683.
FEB 52.	494.	770.32	759.69	769.70	278638.	0.	0.	18245400.	27180.	56.91 684.
MAR 88.	474.	749.28	738.87	748.70	253451.	0.	0.	18783000.	25246.	53.21 643.
APR 139.	465.	729.90	720.93	729.24	233022.	0.	0.	18041100.	25057.	51.59 624.
MAY 446.	465.	720.37	719.81	719.81	231800.	0.	0.	17591400.	23644.	52.11 630.
JUN 886.	455.	725.48	731.10	725.93	246322.	0.	0.	16781000.	23307.	51.16 619.
JUL 984.	435.	745.21	755.33	744.72	278107.	0.	0.	17096000.	22778.	52.81 639.
AUG 819.	428.	772.10	784.87	771.61	308137.	0.	0.	17544200.	23583.	54.99 665.
SEP 825.	450.	795.85	804.96	794.78	334881.	0.	0.	18446000.	25619.	56.88 688.
OCT 773.	453.	814.20	821.57	813.68	356107.	0.	0.	19705200.	26485.	58.34 705.
NOV 317.	490.	817.62	813.68	817.00	365423.	0.	0.	21032600.	29212.	58.84 709.
DEC 276.	501.	808.31	802.95	807.68	331543.	0.	0.	21618100.	29056.	57.81 699.
YEAR OF STUDY 2026										
JAN 123.	476.	794.53	784.12	793.55	309556.	0.	0.	20043300.	26940.	56.82 687.
FEB 119.	489.	777.77	769.42	777.15	283344.	0.	0.	18245400.	27180.	55.49 671.
MAR 94.	466.	759.47	749.52	758.50	264430.	0.	0.	18783000.	25246.	54.06 654.
APR 191.	475.	742.46	735.39	741.87	246475.	0.	0.	18041100.	25057.	52.44 637.
MAY 478.	473.	736.04	736.49	735.51	251031.	0.	0.	17591400.	23644.	52.11 630.
JUN 960.	438.	749.74	762.80	749.25	282080.	0.	0.	16781000.	23307.	53.18 643.
JUL 1282.	410.	784.50	804.20	784.02	335479.	0.	0.	17096000.	22778.	56.00 677.
AUG 1400.	397.	827.24	845.33	826.78	379143.	0.	0.	17544200.	23583.	58.37 718.
SEP 679.	418.	852.74	857.15	852.25	412649.	0.	0.	18446000.	25619.	61.28 761.
OCT 904.	425.	865.35	871.55	864.86	462392.	0.	0.	19705200.	26485.	62.19 752.
NOV 194.	467.	849.02	844.49	846.45	426085.	0.	0.	21032600.	29212.	62.42 755.
DEC 53.	471.	857.34	850.19	854.76	400345.	0.	0.	21618100.	29056.	61.04 748.
YEAR OF STUDY 2027										
JAN 173.	443.	844.69	839.19	844.19	383708.	0.	0.	20043300.	26940.	60.73 734.
FEB 74.	454.	832.58	825.98	832.06	362576.	0.	0.	18245400.	27180.	59.80 723.
MAR 87.	430.	818.19	810.40	817.70	341684.	0.	0.	18783000.	25246.	56.71 710.
APR 80.	436.	802.40	794.41	801.91	320295.	0.	0.	18041100.	25057.	57.45 695.
MAY 288.	418.	791.14	787.92	790.48	312296.	0.	0.	17591400.	23644.	56.55 689.
JUN 753.	409.	795.43	803.94	795.45	332330.	0.	0.	16781000.	23307.	56.90 688.
JUL 851.	393.	814.43	824.89	813.95	360855.	0.	0.	17096000.	22778.	58.37 718.
AUG 787.	394.	832.44	839.99	831.97	384989.	0.	0.	17544200.	23583.	59.78 723.
SEP 1042.	418.	851.12	862.24	856.44	422009.	0.	0.	18446000.	25619.	61.14 760.
OCT 958.	447.	864.52	864.78	864.03	430222.	0.	0.	19705200.	26485.	62.13 751.
NOV 383.	469.	845.36	843.93	844.79	425073.	0.	0.	21032600.	29212.	62.19 752.
DEC 284.	469.	840.75	837.58	840.18	413447.	0.	0.	21618100.	29056.	61.04 748.
YEAR OF STUDY 2028										
JAN 157.	440.	852.23	846.88	851.73	395012.	0.	0.	20043300.	26940.	61.21 748.
FEB 101.	449.	842.14	837.41	841.83	380854.	0.	0.	18917700.	27180.	60.51 732.
MAR 103.	422.	831.26	825.11	830.77	361108.	0.	0.	18245400.	27180.	59.49 721.
APR 109.	426.	818.09	811.06	817.60	342275.	0.	0.	18783000.	25246.	59.70 722.
MAY 386.	427.	810.08	805.05	805.60	339734.	0.	0.	18041100.	25057.	58.70 710.
JUN 699.	390.	815.83	822.58	815.36	357615.	0.	0.	17591400.	23644.	58.05 702.
JUL 892.	384.	832.65	842.73	832.19	388833.	0.	0.	16781000.	23307.	58.47 707.
AUG 715.	384.	848.87	855.01	846.93	409024.	0.	0.	17096000.	22778.	59.80 723.
SEP 1148.	410.	867.52	880.03	867.04	454078.	0.	0.	17544200.	23583.	61.00 738.
OCT 927.	417.	881.71	883.39	881.23	460782.	0.	0.	18446000.	25619.	62.35 754.
NOV 128.	456.	878.04	872.62	877.49	448689.	0.	0.	19705200.	26485.	63.94 764.
DEC 93.	444.	866.30	859.92	865.74	417864.	0.	0.	21032600.	29212.	63.09 763.
YEAR OF STUDY 2029										
JAN 47.	439.	853.00	846.08	852.50	393730.	0.	0.	20043300.	26940.	61.29 741.
FEB 46.	450.	838.74	831.40	838.22	371248.	0.	0.	18245400.	27180.	60.29 729.
MAR 47.	427.	823.51	815.82	823.02	347880.	0.	0.	18783000.	25246.	59.11 715.
APR 122.	432.	808.39	801.32	807.90	329602.	0.	0.	18041100.	25057.	57.93 700.
MAY 239.	414.	797.08	793.00	796.60	318602.	0.	0.	17591400.	23644.	57.02 689.
JUN 816.	405.	802.33	811.66	801.86	343001.	0.	0.	16781000.	23307.	57.41 694.
JUL 1007.	390.	820.71	828.76	820.25	390428.	0.	0.	17096000.	22778.	58.88 712.
AUG 1011.	389.	841.77	853.78	841.31	404820.	0.	0.	17544200.	23583.	60.49 731.
SEP 982.	412.	863.19	872.40	862.71	440681.	0.	0.	18446000.	25619.	62.04 750.
OCT 605.	420.	875.74	878.89	875.26	452005.	0.	0.	19705200.	26485.	62.93 761.
NOV 137.	465.	873.46	868.03	872.90	432462.	0.	0.	21032600.	29212.	62.76 759.
DEC 82.	469.	861.41	854.80	860.84	406419.	0.	0.	21618100.	29056.	61.89 748.
YEAR OF STUDY 2030										
JAN 83.	442.	847.95	841.11	847.45	365758.	0.	0.	20043300.	26940.	60.93 737.
FEB 123.	452.	835.31	829.52	834.79	348238.	0.	0.	18245400.	27180.	60.01 725.
MAR 49.	428.	821.39	813.27	820.90	344925.	0.	0.	18783000.	25246.	58.95 715.
APR 53.	434.	809.78	798.29	804.28	322806.	0.	0.	18041100.	25057.	57.44 697.
MAY 316.	416.	793.62	791.35	793.34	316423.	0.	0.	17591400.	23644.	56.97 689.
JUN 650.	409.	797.05	802.76	796.58	331317.	0.	0.	16781000.	23307.	56.97 689.
JUL 940.	393.	815.11	827.46	814.64	364955.	0.	0.	17096000.	22778.	58.43 707.
AUG 1218.	389.	841.14	858.83	842.49	415895.	0.	0.	17544200.	23583.	60.49 731.
SEP 872.	411.	864.44	874.05	865.96	443302.	0.	0.	18446000.	25619.	62.04 750.
OCT 652.	419.	877.92	881.79	877.43	457581.	0.	0.	19705200.	26485.	62.93 76

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1. STORAGE EOW EQUALS THE STORAGE IN THE RESERVOIR AT THE END OF THE MONTH.
2. WASTE IS THE NONUSABLE OUTFLOW WASTED OVER THE SPILLWAY.
3. YEARS ARE PROJECTED INTO THE ASSUMED TIME OF PROJECT OPERATION WITH 1903 EQUIVALENT TO THE FIRST YEAR OF PERIOD OF RECORD.
4. LONG LAKE PRODUCES 225 MILLION KWH/YEAR PRIMARY ENERGY AND 9.8 MILLION KWH/YEAR OF SECONDARY ENERGY WITHOUT BENEFIT OF RULE CURVE OPERATION. SEE PLATES 2 & 3 AND TABLES 6 & 10.

REPORTING OFFICE		REPORTING DATE	
U. S. ARMY ENGINEER DISTRICT. ALASKA		CORPS OF ENGINEERS	
ANCHORAGE, ALASKA			
DESIGNED	SNETTISHAM, ALASKA LONG LAKE MONTHLY REGULATION COMPUTATIONS		
DRAWN			
CHECKED			
REVIEWED			
APPROVED			
SPECIAL INSTRUCTIONS 1. <i>See</i> 2. <i>See</i> 3. <i>See</i> 4. <i>See</i> 5. <i>See</i> 6. <i>See</i> 7. <i>See</i> 8. <i>See</i> 9. <i>See</i> 10. <i>See</i> 11. <i>See</i> 12. <i>See</i> 13. <i>See</i> 14. <i>See</i> 15. <i>See</i> 16. <i>See</i> 17. <i>See</i> 18. <i>See</i> 19. <i>See</i> 20. <i>See</i> 21. <i>See</i> 22. <i>See</i> 23. <i>See</i> 24. <i>See</i> 25. <i>See</i> 26. <i>See</i> 27. <i>See</i> 28. <i>See</i> 29. <i>See</i> 30. <i>See</i> 31. <i>See</i> 32. <i>See</i> 33. <i>See</i> 34. <i>See</i> 35. <i>See</i> 36. <i>See</i> 37. <i>See</i> 38. <i>See</i> 39. <i>See</i> 40. <i>See</i> 41. <i>See</i> 42. <i>See</i> 43. <i>See</i> 44. <i>See</i> 45. <i>See</i> 46. <i>See</i> 47. <i>See</i> 48. <i>See</i> 49. <i>See</i> 50. <i>See</i> 51. <i>See</i> 52. <i>See</i> 53. <i>See</i> 54. <i>See</i> 55. <i>See</i> 56. <i>See</i> 57. <i>See</i> 58. <i>See</i> 59. <i>See</i> 60. <i>See</i> 61. <i>See</i> 62. <i>See</i> 63. <i>See</i> 64. <i>See</i> 65. <i>See</i> 66. <i>See</i> 67. <i>See</i> 68. <i>See</i> 69. <i>See</i> 70. <i>See</i> 71. <i>See</i> 72. <i>See</i> 73. <i>See</i> 74. <i>See</i> 75. <i>See</i> 76. <i>See</i> 77. <i>See</i> 78. <i>See</i> 79. <i>See</i> 80. <i>See</i> 81. <i>See</i> 82. <i>See</i> 83. <i>See</i> 84. <i>See</i> 85. <i>See</i> 86. <i>See</i> 87. <i>See</i> 88. <i>See</i> 89. <i>See</i> 90. <i>See</i> 91. <i>See</i> 92. <i>See</i> 93. <i>See</i> 94. <i>See</i> 95. <i>See</i> 96. <i>See</i> 97. <i>See</i> 98. <i>See</i> 99. <i>See</i> 100. <i>See</i> 101. <i>See</i> 102. <i>See</i> 103. <i>See</i> 104. <i>See</i> 105. <i>See</i> 106. <i>See</i> 107. <i>See</i> 108. <i>See</i> 109. <i>See</i> 110. <i>See</i> 111. <i>See</i> 112. <i>See</i> 113. <i>See</i> 114. <i>See</i> 115. <i>See</i> 116. <i>See</i> 117. <i>See</i> 118. <i>See</i> 119. <i>See</i> 120. <i>See</i> 121. <i>See</i> 122. <i>See</i> 123. <i>See</i> 124. <i>See</i> 125. <i>See</i> 126. <i>See</i> 127. <i>See</i> 128. <i>See</i> 129. <i>See</i> 130. <i>See</i> 131. <i>See</i> 132. <i>See</i> 133. <i>See</i> 134. <i>See</i> 135. <i>See</i> 136. <i>See</i> 137. <i>See</i> 138. <i>See</i> 139. <i>See</i> 140. <i>See</i> 141. <i>See</i> 142. <i>See</i> 143. <i>See</i> 144. <i>See</i> 145. <i>See</i> 146. <i>See</i> 147. <i>See</i> 148. <i>See</i> 149. <i>See</i> 150. <i>See</i> 151. <i>See</i> 152. <i>See</i> 153. <i>See</i> 154. <i>See</i> 155. <i>See</i> 156. <i>See</i> 157. <i>See</i> 158. <i>See</i> 159. <i>See</i> 160. <i>See</i> 161. <i>See</i> 162. <i>See</i> 163. <i>See</i> 164. <i>See</i> 165. <i>See</i> 166. <i>See</i> 167. <i>See</i> 168. <i>See</i> 169. <i>See</i> 170. <i>See</i> 171. <i>See</i> 172. <i>See</i> 173. <i>See</i> 174. <i>See</i> 175. <i>See</i> 176. <i>See</i> 177. <i>See</i> 178. <i>See</i> 179. <i>See</i> 180. <i>See</i> 181. <i>See</i> 182. <i>See</i> 183. <i>See</i> 184. <i>See</i> 185. <i>See</i> 186. <i>See</i> 187. <i>See</i> 188. <i>See</i> 189. <i>See</i> 190. <i>See</i> 191. <i>See</i> 192. <i>See</i> 193. <i>See</i> 194. <i>See</i> 195. <i>See</i> 196. <i>See</i> 197. <i>See</i> 198. <i>See</i> 199. <i>See</i> 200. <i>See</i> 201. <i>See</i> 202. <i>See</i> 203. <i>See</i> 204. <i>See</i> 205. <i>See</i> 206. <i>See</i> 207. <i>See</i> 208. <i>See</i> 209. <i>See</i> 210. <i>See</i> 211. <i>See</i> 212. <i>See</i> 213. <i>See</i> 214. <i>See</i> 215. <i>See</i> 216. <i>See</i> 217. <i>See</i> 218. <i>See</i> 219. <i>See</i> 220. <i>See</i> 221. <i>See</i> 222. <i>See</i> 223. <i>See</i> 224. <i>See</i> 225. <i>See</i> 226. <i>See</i> 227. <i>See</i> 228. <i>See</i> 229. <i>See</i> 230. <i>See</i> 231. <i>See</i> 232. <i>See</i> 233. <i>See</i> 234. <i>See</i> 235. <i>See</i> 236. <i>See</i> 237. <i>See</i> 238. <i>See</i> 239. <i>See</i> 240. <i>See</i> 241. <i>See</i> 242. <i>See</i> 243. <i>See</i> 244. <i>See</i> 245. <i>See</i> 246. <i>See</i> 247. <i>See</i> 248			

FLOW										FLOW									
NATL CFS		REG CFS		AVERAGE		EOM		HEAD		EOM		NATL CFS		REG CFS		AVERAGE		EOM	
FTMSL		FTMSL		FTMSL		FTMSL		FT		FT		FTMSL		REG CFS		FTMSL		FTMSL	
YEAR OF STUDY		YEAR OF STUDY		YEAR OF STUDY		YEAR OF STUDY		YEAR OF STUDY		YEAR OF STUDY		YEAR OF STUDY		YEAR OF STUDY		YEAR OF STUDY		YEAR OF STUDY	
1987		1988		1989		1990		1991		1992		1993		1994		1995		1996	
1997		1998		1999		2000		2001		2002		2003		2004		2005		2006	
2007		2008		2009		2010		2011		2012		2013		2014		2015		2016	
2017		2018		2019		2020		2021		2022		2023		2024		2025		2026	
2027		2028		2029		2030		2031		2032		2033		2034		2035		2036	
2037		2038		2039		2040		2041		2042		2043		2044		2045		2046	
2047		2048		2049		2050		2051		2052		2053		2054		2055		2056	
2057		2058		2059		2060		2061		2062		2063		2064		2065		2066	
2067		2068		2069		2070		2071		2072		2073		2074		2075		2076	
2077		2078		2079		2080		2081		2082		2083		2084		2085		2086	
2087		2088		2089		2090		2091		2092		2093		2094		2095		2096	
2097		2098		2099		2100		2101		2102		2103		2104		2105		2106	
2107		2108		2109		2110		2111		2112		2113		2114		2115		2116	
2117		2118		2119		2120		2121		2122		2123		2124		2125		2126	
2127		2128		2129		2130		2131		2132		2133		2134		2135		2136	
2137		2138		2139		2140		2141		2142		2143		2144		2145		2146	
2147		2148		2149		2150		2151		2152		2153		2154		2155		2156	
2157		2158		2159		2160		2161		2162		2163		2164		2165		2166	
2167		2168		2169		2170		2171		2172		2173		2174		2175		2176	
2177		2178		2179		2180		2181		2182		2183		2184		2185		2186	
2187		2188		2189		2190		2191		2192		2193		2194		2195		2196	
2197		2198		2199		2200		2201		2202		2203		2204		2205		2206	
2207		2208		2209		2210		2211		2212		2213		2214		2215		2216	
2217		2218		2219		2220		2221		2222		2223		2224		2225		2226	
2227		2228		2229		2230		2231		2232		2233		2234		2235		2236	
2237		2238		2239		2240		2241		2242		2243		2244		2245		2246	
2247		2248		2249		2250		2251		2252		2253		2254		2255		2256	
2257		2258		2259		2260		2261		2262		2263		2264		2265		2266	
2267		2268		2269		2270		2271		2272		2273		2274		2275		2276	
2277		2278		2279		2280		2281		2282		2283		2284		2285		2286	
2287		2288		2289		2290		2291		2292		2293		2294		2295		2296	
2297		2298		2299		2300		2301		2302		2303		2304		2305		2306	
2307		2308		2309		2310		2311		2312		2313		2314		2315		2316	
2317		2318		2319		2320		2321		2322		2323		2324		2325		2326	
2327		2328		2329		2330		2331		2332		2333		2334		2335		2336	
2337		2338		2339		2340		2341		2342		2343		2344		2345		2346	
2347		2348		2349		2350		2351		2352		2353		2354		2355		2356	
2357		2358		2359		2360		2361		2362		2363		2364		2365		2366	
2367		2368		2369		2370		2371		2372		2373		2374		2375		2376	
2377		2378		2379		2380		2381		2382		2383		2384		2385		2386	
2387		2388		2389		2390		2391		2392		2393		2394		2395		2396	
2397		2398		2399		2400		2401		2402		2403		2404		2405		2406	
2407		2408		2409		2410		2411		2412		2413		2414		2415		2416	
2417		2418		2419		2420		2421		2422		2423		2424		2425		2426	
2427		2428		2429		2430		2431		2432		2433		2434		2435		2436	
2437		2438		2439		2440		2441		2442		2443		2444		2445		2446	
2447		2448		2449		2450		2451		2452		2453		2454		2455		2456	
2457		2458		2459		2460		2461		2462		2463		2464		2465		2466	
2467		2468		2469		2470		2471		2472		2473		2474		2475		2476	
2477		2478		2479		2480		2481		2482		2483		2484		2485		2486	
2487		2488		2489		2490		2491		2492		2493		2494		2495		2496	
2497		2498		2499		2500		2501		2502		2503		2504		2505		2506	
2507		2508		2509		2510		2511		2512		2513		2514		2515		2516	
2517		2518		2519		2520		2521		2522		2523		2524		2525		2526	
2527		2528		2529		2530		2531		2532		2533		2534		2535		2536	
2537		2538		2539		2540		2541		2542		2543		2544		2545		2546	
2547		2548		2549		2550		2551		2552		2553		2554		2555		2556	
2557		2558		2559		2560		2561		2562		2563		2564		2565		2566	
2567		2568		2569		2570		2571		2572		2573		2574		2575		2576	
2577		2578		2579		2580		2581		2582		2583		2584		2585		2586	
2587		2588		2589		2590		2591		2592		2593		2594		2595		2596	
2597		2598		2599		2600		2601		2602		2603		2604		2605		2606	
2607		2608		2609		2610		2611		2612		2613		2614		2615		2616	
2617		2618		2619		2620		2621		2622		2623		2624		2625		2626	
2627		2628		2629		2630		2631		2632		2633		2634		2635		2636	
2637		2638		2639		2640		2641		2642		2643		2644		2645		2646	
2647		2648		2649		2650		2651		2652		2653		2654		2655		2656	
2657		2658		2659		2660		2661		2662		2663		2664		2665		2666	
2667		2668		2669		2670		2671		2672		2673		2674		2675		2676	
2677		2678		2679		2680		2681		2682		2683		2684		2685		2686	
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2697		2698		2699		2700		2701		2702		2703		2704		2705		2706	
2707		2708		2709		2710		2711		2712		2713		2714		2715		2716	
2717		2718		2719		2720		2721		2722		2723		2724		2725		2726	
2727		2728		2729		2730		2731		2732		2733		2734		2735		2736	
2737		2738		2739		2740		2741		2742		2743		2744		2745		2746	
2747		2748		2749		2750		2751		2752		2753		2754		2755		2756	
2757		2758		2759		2760		2761		2762		2763		2764		2765		2766	
2767		2768		2769		2770		2771		2772		2773		2774		2775		2776	
2777		2778		2779		2780		2781		2782		2783		2784		2785		2786	
2787		2788		2789		2790		2791		2792		2793		2794		2795		2796	
2797		2798		2799		2800		2801		2802		2803		2804		2805		2806	
2807		2808		2809		2810		2811		2812		2813		2814		2815		2816	
2817		2818		2819		2820		2821		2822		2823		2824		2825		2826	
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2897		2898		2899		2900		2901		2902		2903		2904		2905		2906	
2907		2908		2909		2910		2911		2912		2913		2914		2915		2916	
2917		2918		2919		2920		2921		2922		2923		2924		2925		2926	
2927		2928		2929		2930		2931		2932		2933		2934		2935		2936	
2937		2938		2939		2940		2941		2942		2943		2944		2945		2946	
2947		2948		2949		2950		2951		2952		2953		2954		2955			

STAGE COM	SECONDARY SPILL AF	ENERGY KWH	PRIMARY ENERGY KWH	POWER KW	KWH/AF	WASTE AF	TAIL WATER
1003	0.	0.	20043300.	26940.	63.32 763.	0.	.4
1012	0.	0.	18265400.	27180.	62.33 754.	0.	.4
1021	0.	0.	18783000.	25246.	57.78 715.	0.	.4
1030	0.	0.	18041100.	25057.	56.55 708.	0.	.4
1039	0.	0.	17591400.	23644.	59.88 724.	0.	.4
1048	0.	0.	16781000.	23307.	58.02 714.	0.	.4
1057	0.	0.	17096000.	22978.	59.83 723.	0.	.4
1066	0.	0.	17546200.	23583.	57.25 692.	0.	.4
1075	0.	0.	18446000.	25619.	58.89 712.	0.	.4
1084	0.	0.	19705200.	26485.	62.51 756.	0.	.4
1093	0.	0.	21032600.	29212.	64.06 774.	0.	.5
1102	0.	0.	21618100.	29056.	63.35 773.	0.	.5
1111	8011.	6135720.	20043300.	26940.	63.32 763.	0.	.7
1120	1005.	1774840.	18265400.	27180.	62.33 754.	0.	.4
1129	2746.	2074420.	18783000.	25246.	61.37 742.	0.	.4
1138	0.	0.	18041100.	25057.	60.45 731.	0.	.4
1147	0.	0.	17591400.	23644.	59.88 724.	0.	.4
1156	0.	0.	16781000.	23307.	60.32 729.	0.	.4
1165	0.	0.	17096000.	22978.	61.14 739.	0.	.4
1174	0.	0.	17546200.	23583.	62.56 757.	0.	.4
1183	0.	0.	18446000.	25619.	63.78 771.	0.	.4
1192	19150.	17032600.	19705200.	26485.	64.25 775.	1348.	2.6
1201	280.	217888.	21032600.	29212.	64.30 778.	0.	.5
1210	0.	0.	21618100.	29056.	63.35 773.	0.	.5
1219	0.	0.	20043300.	26940.	63.32 763.	0.	.4
1228	0.	0.	18265400.	27180.	62.33 754.	0.	.4
1237	0.	0.	18783000.	25246.	61.37 742.	0.	.4
1246	1572.	1149800.	18041100.	25057.	60.45 731.	0.	.4
1255	0.	0.	17591400.	23644.	59.88 724.	0.	.4
1264	0.	0.	16781000.	23307.	60.32 729.	0.	.4
1273	0.	0.	17096000.	22978.	61.14 739.	0.	.4
1282	0.	0.	17546200.	23583.	62.56 757.	0.	.4
1291	0.	0.	18446000.	25619.	63.78 771.	0.	.4
1300	1365.	1062790.	19705200.	26485.	64.25 775.	0.	.4
1309	4442.	1456130.	21032600.	29212.	64.30 778.	0.	.5
1318	1269.	966892.	21618100.	29056.	63.35 773.	0.	.5
1327	0.	0.	20043300.	26940.	63.32 763.	0.	.4
1336	0.	0.	18265400.	27180.	62.33 754.	0.	.4
1345	0.	0.	18783000.	25246.	61.37 742.	0.	.4
1354	0.	0.	18041100.	25057.	60.45 731.	0.	.4
1363	0.	0.	17591400.	23644.	59.88 724.	0.	.4
1372	0.	0.	16781000.	23307.	60.32 729.	0.	.4
1381	0.	0.	17096000.	22978.	61.14 739.	0.	.4
1390	0.	0.	17546200.	23583.	62.56 757.	0.	.4
1399	0.	0.	18446000.	25619.	63.78 771.	0.	.4
1408	0.	0.	19705200.	26485.	64.25 775.	0.	.4
1417	0.	0.	21032600.	29212.	64.30 778.	0.	.5
1426	0.	0.	21618100.	29056.	63.35 773.	0.	.5
1435	0.	0.	20043300.	26940.	63.32 763.	0.	.4
1444	0.	0.	18265400.	27180.	62.33 754.	0.	.4
1453	0.	0.	18783000.	25246.	61.37 742.	0.	.4
1462	0.	0.	18041100.	25057.	60.45 731.	0.	.4
1471	0.	0.	17591400.	23644.	59.88 724.	0.	.4
1480	0.	0.	16781000.	23307.	60.32 729.	0.	.4
1489	0.	0.	17096000.	22978.	61.14 739.	0.	.4
1498	0.	0.	17546200.	23583.	62.56 757.	0.	.4
1507	0.	0.	18446000.	25619.	63.78 771.	0.	.4
1516	0.	0.	19705200.	26485.	64.25 775.	0.	.4
1525	0.	0.	21032600.	29212.	64.30 778.	0.	.5
1534	0.	0.	21618100.	29056.	63.35 773.	0.	.5
1543	0.	0.	20043300.	26940.	63.32 763.	0.	.4
1552	0.	0.	18265400.	27180.	62.33 754.	0.	.4
1561	0.	0.	18783000.	25246.	61.37 742.	0.	.4
1570	0.	0.	18041100.	25057.	60.45 731.	0.	.4
1579	0.	0.	17591400.	23644.	59.88 724.	0.	.4
1588	0.	0.	16781000.	23307.	60.32 729.	0.	.4
1597	0.	0.	17096000.	22978.	61.14 739.	0.	.4
1606	0.	0.	17546200.	23583.	62.56 757.	0.	.4
1615	0.	0.	18446000.	25619.	63.78 771.	0.	.4
1624	0.	0.	19705200.	26485.	64.25 775.	0.	.4
1633	0.	0.	21032600.	29212.	64.30 778.	0.	.5
1642	0.	0.	21618100.	29056.	63.35 773.	0.	.5
1651	0.	0.	20043300.	26940.	63.32 763.	0.	.4
1660	0.	0.	18265400.	27180.	62.33 754.	0.	.4
1669	0.	0.	18783000.	25246.	61.37 742.	0.	.4
1678	0.	0.	18041100.	25057.	60.45 731.	0.	.4
1687	0.	0.	17591400.	23644.	59.88 724.	0.	.4
1696	0.	0.	16781000.	23307.	60.32 729.	0.	.4
1705	0.	0.	17096000.	22978.	61.14 739.	0.	.4
1714	0.	0.	17546200.	23583.	62.56 757.	0.	.4
1723	0.	0.	18446000.	25619.	63.78 771.	0.	.4
1732	0.	0.	19705200.	26485.	64.25 775.	0.	.4
1741	0.	0.	21032600.	29212.	64.30 778.	0.	.5
1750	0.	0.	21618100.	29056.	63.35 773.	0.	.5
1759	0.	0.	20043300.	26940.	63.32 763.	0.	.4
1768	0.	0.	18265400.	27180.	62.33 754.	0.	.4
1777	0.	0.	18783000.	25246.	61.37 742.	0.	.4
1786	0.	0.	18041100.	25057.	60.45 731.	0.	.4
1795	0.	0.	17591400.	23644.	59.88 724.	0.	.4
1804	0.	0.	16781000.	23307.	60.32 729.	0.	.4
1813	0.	0.	17096000.	22978.	61.14 739.	0.	.4
1822	0.	0.	17546200.	23583.	62.56 757.	0.	.4
1831	0.	0.	18446000.	25619.	63.78 771.	0.	.4
1840	0.	0.	19705200.	26485.	64.25 775.	0.	.4
1849	0.	0.	21032600.	29212.	64.30 778.	0.	.5
1858	0.	0.	21618100.	29056.	63.35 773.	0.	.5
1867	0.	0.	20043300.	26940.	63.32 763.	0.	.4
1876	0.	0.	18265400.	27180.	62.33 754.	0.	.4
1885	0.	0.	18783000.	25246.	61.37 742.	0.	.4
1894	0.	0.	18041100.	25057.	60.45 731.	0.	.4
1903	0.	0.	17591400.	23644.	59.88 724.	0.	.4
1912	0.	0.	16781000.	23307.	60.32 729.	0.	.4
1921	0.	0.	17096000.	22978.	61.14 739.	0.	.4
1930	0.	0.	17546200.	23583.	62.56 757.	0.	.4
1939	0.	0.	18446000.	25619.	63.78 771.	0.	.4
1948	0.	0.	19705200.	26485.	64.25 775.	0.	.4
1957	0.	0.	21032600.	29212.	64.30 778.	0.	.5
1966	0.	0.	21618100.	29056.	63.35 773.	0.	.5
1975	0.	0.	20043300.	26940.	63.32 763.	0.	.4
1984	0.	0.	18265400.	27180.	62.33 754.	0.	.4
1993	0.	0.	18783000.	25246.	61.37 742.	0.	.4
2002	0.	0.	18041100.	25057.	60.45 731.	0.	.4
2011	0.	0.	17591400.	23644.	59.88 724.	0.	.4
2020	0.	0.	16781000.	23307.	60.32 729.	0.	.4
2029	0.	0.	17096000.	22978.	61.14 739.	0.	.4
2038	0.	0.	17546200.	23583.	62.56 757.	0.	.4
2047	0.	0.	18446000.	25619.	63.78 771.	0.	.4
2056	0.	0.	19705200.	26485.	64.25 775.	0.	.4
2065	0.	0.	21032600.	29212.	64.30 778.	0.	.5
2074	0.	0.	21618100.	29056.	63.35 773.	0.	.5
2083	0.	0.	20043300.	26940.	63.32 763.	0.	.4
2092	0.	0.	18265400.	27180.	62.33 754.	0.	.4
2101	0.	0.	18783000.	25246.	61.37 742.	0.	.4
2110	0.	0.	18041100.	25057.	60.45 731.	0.	.4
2119	0.	0.	17591400.	23644.	59.88 724.	0.	.4
2128	0.	0.	16781000.	23307.	60.32 729.	0.	.4
2137	0.	0.	17096000.	22978.	61.14 739.	0.	.4
2146	0.	0.	17546200.	23583.	62.56 757.	0.	.4
2155	0.	0.	18446000.	25619.	63.78 771.	0.	.4
2164	0.	0.	19705200.	26485.	64.25 775.	0.	.4
2173	0.	0.	21032600.	29212.	64.30 778.	0.	.5
2182	0.	0.	21618100.	29056.	63.35 773.	0.	.5
2191	0.	0.	20043300.	26940.	63.32 763.	0.	.4
2200	0.	0.	18265400.	27180.	62.33 754.	0.	.4
2209	0.	0.	18783000.	25246.	61.37 742.	0.	.4
2218	0.	0.	18041100.	25057.	60.45 731.	0.	.4
2227	0.	0.	17591400.	23644.	59.88 724.	0.	.4
2236	0.	0.	16781000.	23307.	60.32 729.	0.	.4
2245	0.	0.	17096000.	22978.	61.14 739.	0.	.4
2254	0.	0.	17546200.	23583.	62.56 757.	0.	.4
2263	0.	0.	18446000.	25619.	63.78 771.	0.	.4
2272	0.	0.	19705200.	26485.	64.25 775.	0.	.4
2281	0.	0.	21032600.	29212.	64.30 778.	0.	.5
2290	0.	0.	21618100.	29056.	63.35 773.	0.	.5
2299	0.	0.	20043300.	26940.	63.32 763.	0.	.4
2308	0.	0.	18265400.	27180.	62.33 754.	0.	.4
2317	0.	0.	18783000.	25246.	61.37 742.	0.	.4
2326	0.	0.	18041100.	25057.	60.45 731.	0.	.4
2335	0.	0.	17591400.	23644.	59.88 724.	0.	.4
2344	0.	0.	16781000.	23307.	60.32 729.	0.	.4
2353	0.	0.	17096000.	22978.	61.14 739.	0.	.4
2362	0.	0.	17546200.	23583.	62.56 757.	0.	.4
2371	0.	0.	18446000.	25619.	63.78 771.	0.	.4
2380	0.	0.	19705200.	26485.	64.25 775.	0.	.4
2389	0.	0.	21032600.	29212.	64.30 778.	0.	.5
2398	0.	0.	21618100.	29056.	63.35 773.	0.	.5
2407	0.	0.	20043300.	26940.	63.32 763.	0.	.4
2416	0.	0.	18265400.	27180.	62.33 754.	0.	.4
2425	0.	0.	18783000.	25246.	61.37 742.	0.	.4
2434	0.	0.	18041100.	25057.	60.45 731.	0.	.4
2443	0.	0.	17591400.	23644.	59.88 724.	0.	.4
2452	0.	0.	16781000.	23307.	60.32 729.	0.	.4
2461	0.	0.	17096				

CORPS OF ENGINEERS

FLOW										FLOW									
POOL ELEVATION										POOL ELEVATION									
HEAD										HEAD									
STORAGE										STORAGE									
SECONDARY										SECONDARY									
ENERGY										ENERGY									
PRIMARY										PRIMARY									
POWER										POWER									
KW/CFS										KW/CFS									
KWH/AF										KWH/AF									
AF										AF									
WASTE										WASTE									
TAL WATER										TAL WATER									
NATL CFS	RES CFS	AVERAGE FT/BL	FT/BL	FT	AF	SPILL AF	ENERGY KWH	ENERGY KWH	POWER KW	NATL CFS	RES CFS	AVERAGE FT/BL	FT/BL	FT	AF	SPILL AF	ENERGY KWH	ENERGY KWH	POWER KW
PROJECT NO. 2										PROJECT NO. 2									
YEAR OF STUDY 2003										YEAR OF STUDY 2003									
JAN 127.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
FEB 128.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
MAR 129.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
APR 130.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
MAY 131.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
JUN 132.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
JUL 133.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
AUG 134.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
SEP 135.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
OCT 136.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
NOV 137.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
DEC 138.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
PROJECT NO. 2										PROJECT NO. 2									
YEAR OF STUDY 2004										YEAR OF STUDY 2004									
JAN 127.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
FEB 128.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
MAR 129.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
APR 130.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
MAY 131.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
JUN 132.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
JUL 133.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
AUG 134.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
SEP 135.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
OCT 136.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
NOV 137.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
DEC 138.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
PROJECT NO. 2										PROJECT NO. 2									
YEAR OF STUDY 2005										YEAR OF STUDY 2005									
JAN 127.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
FEB 128.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
MAR 129.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
APR 130.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
MAY 131.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
JUN 132.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
JUL 133.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
AUG 134.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
SEP 135.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
OCT 136.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
NOV 137.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.
DEC 138.	228.	874.52	869.37	874.03	430867.	0.	0.	20043300.	26940.	62.84	760.	0.	0.	0.	0.	0.	0.	0.	0.

LONG LAKE

MONTHLY REGULATION COMPUTATIONS
RULE CURVE OPERATION

BRIDGE	SECONDARY	PRIMARY	WASTE	TAIL				
COM	SPILL	ENERGY	POWER	WATER				
AF	AF	KWH	KW	AF				
013	0.	0.	20043300.	26940.	62.96	761.	0.	4
014	0.	0.	18265400.	27180.	62.19	752.	0.	4
015	0.	0.	18783000.	25246.	61.19	751.	0.	4
016	0.	0.	18041100.	25057.	60.33	730.	0.	4
017	0.	0.	17591400.	23644.	59.79	697.	0.	4
018	0.	0.	16781000.	23307.	58.89	727.	0.	4
019	0.	0.	17096000.	22978.	61.21	760.	0.	4
020	0.	0.	17546200.	23583.	60.73	734.	0.	4
021	0.	0.	18446000.	25619.	61.87	768.	0.	4
022	0.	0.	19705200.	26485.	60.32	729.	0.	4
023	0.	0.	21032600.	29212.	61.68	766.	0.	4
024	0.	0.	21618100.	29056.	61.53	744.	0.	4
025	0.	0.	20043300.	26940.	62.96	761.	0.	4
026	0.	0.	18265400.	27180.	62.19	752.	0.	4
027	0.	0.	18783000.	25246.	61.19	751.	0.	4
028	0.	0.	18041100.	25057.	60.33	730.	0.	4
029	0.	0.	17591400.	23644.	59.79	697.	0.	4
030	0.	0.	16781000.	23307.	58.89	727.	0.	4
031	0.	0.	17096000.	22978.	61.21	760.	0.	4
032	0.	0.	17546200.	23583.	60.73	734.	0.	4
033	0.	0.	18446000.	25619.	61.87	768.	0.	4
034	0.	0.	19705200.	26485.	60.32	729.	0.	4
035	0.	0.	21032600.	29212.	61.68	766.	0.	4
036	0.	0.	21618100.	29056.	61.53	744.	0.	4
037	0.	0.	20043300.	26940.	62.96	761.	0.	4
038	0.	0.	18265400.	27180.	62.19	752.	0.	4
039	0.	0.	18783000.	25246.	61.19	751.	0.	4
040	0.	0.	18041100.	25057.	60.33	730.	0.	4
041	0.	0.	17591400.	23644.	59.79	697.	0.	4
042	0.	0.	16781000.	23307.	58.89	727.	0.	4
043	0.	0.	17096000.	22978.	61.21	760.	0.	4
044	0.	0.	17546200.	23583.	60.73	734.	0.	4
045	0.	0.	18446000.	25619.	61.87	768.	0.	4
046	0.	0.	19705200.	26485.	60.32	729.	0.	4
047	0.	0.	21032600.	29212.	61.68	766.	0.	4
048	0.	0.	21618100.	29056.	61.53	744.	0.	4
049	0.	0.	20043300.	26940.	62.96	761.	0.	4
050	0.	0.	18265400.	27180.	62.19	752.	0.	4
051	0.	0.	18783000.	25246.	61.19	751.	0.	4
052	0.	0.	18041100.	25057.	60.33	730.	0.	4
053	0.	0.	17591400.	23644.	59.79	697.	0.	4
054	0.	0.	16781000.	23307.	58.89	727.	0.	4
055	0.	0.	17096000.	22978.	61.21	760.	0.	4
056	0.	0.	17546200.	23583.	60.73	734.	0.	4
057	0.	0.	18446000.	25619.	61.87	768.	0.	4
058	0.	0.	19705200.	26485.	60.32	729.	0.	4
059	0.	0.	21032600.	29212.	61.68	766.	0.	4
060	0.	0.	21618100.	29056.	61.53	744.	0.	4
061	0.	0.	20043300.	26940.	62.96	761.	0.	4
062	0.	0.	18265400.	27180.	62.19	752.	0.	4
063	0.	0.	18783000.	25246.	61.19	751.	0.	4
064	0.	0.	18041100.	25057.	60.33	730.	0.	4
065	0.	0.	17591400.	23644.	59.79	697.	0.	4
066	0.	0.	16781000.	23307.	58.89	727.	0.	4
067	0.	0.	17096000.	22978.	61.21	760.	0.	4
068	0.	0.	17546200.	23583.	60.73	734.	0.	4
069	0.	0.	18446000.	25619.	61.87	768.	0.	4
070	0.	0.	19705200.	26485.	60.32	729.	0.	4
071	0.	0.	21032600.	29212.	61.68	766.	0.	4
072	0.	0.	21618100.	29056.	61.53	744.	0.	4
073	0.	0.	20043300.	26940.	62.96	761.	0.	4
074	0.	0.	18265400.	27180.	62.19	752.	0.	4
075	0.	0.	18783000.	25246.	61.19	751.	0.	4
076	0.	0.	18041100.	25057.	60.33	730.	0.	4
077	0.	0.	17591400.	23644.	59.79	697.	0.	4
078	0.	0.	16781000.	23307.	58.89	727.	0.	4
079	0.	0.	17096000.	22978.	61.21	760.	0.	4
080	0.	0.	17546200.	23583.	60.73	734.	0.	4
081	0.	0.	18446000.	25619.	61.87	768.	0.	4
082	0.	0.	19705200.	26485.	60.32	729.	0.	4
083	0.	0.	21032600.	29212.	61.68	766.	0.	4
084	0.	0.	21618100.	29056.	61.53	744.	0.	4
085	0.	0.	20043300.	26940.	62.96	761.	0.	4
086	0.	0.	18265400.	27180.	62.19	752.	0.	4
087	0.	0.	18783000.	25246.	61.19	751.	0.	4
088	0.	0.	18041100.	25057.	60.33	730.	0.	4
089	0.	0.	17591400.	23644.	59.79	697.	0.	4
090	0.	0.	16781000.	23307.	58.89	727.	0.	4
091	0.	0.	17096000.	22978.	61.21	760.	0.	4
092	0.	0.	17546200.	23583.	60.73	734.	0.	4
093	0.	0.	18446000.	25619.	61.87	768.	0.	4
094	0.	0.	19705200.	26485.	60.32	729.	0.	4
095	0.	0.	21032600.	29212.	61.68	766.	0.	4
096	0.	0.	21618100.	29056.	61.53	744.	0.	4
097	0.	0.	20043300.	26940.	62.96	761.	0.	4
098	0.	0.	18265400.	27180.	62.19	752.	0.	4
099	0.	0.	18783000.	25246.	61.19	751.	0.	4
100	0.	0.	18041100.	25057.	60.33	730.	0.	4
101	0.	0.	17591400.	23644.	59.79	697.	0.	4
102	0.	0.	16781000.	23307.	58.89	727.	0.	4
103	0.	0.	17096000.	22978.	61.21	760.	0.	4
104	0.	0.	17546200.	23583.	60.73	734.	0.	4
105	0.	0.	18446000.	25619.	61.87	768.	0.	4
106	0.	0.	19705200.	26485.	60.32	729.	0.	4
107	0.	0.	21032600.	29212.	61.68	766.	0.	4
108	0.	0.	21618100.	29056.	61.53	744.	0.	4
109	0.	0.	20043300.	26940.	62.96	761.	0.	4
110	0.	0.	18265400.	27180.	62.19	752.	0.	4
111	0.	0.	18783000.	25246.	61.19	751.	0.	4
112	0.	0.	18041100.	25057.	60.33	730.	0.	4
113	0.	0.	17591400.	23644.	59.79	697.	0.	4
114	0.	0.	16781000.	23307.	58.89	727.	0.	4
115	0.	0.	17096000.	22978.	61.21	760.	0.	4
116	0.	0.	17546200.	23583.	60.73	734.	0.	4
117	0.	0.	18446000.	25619.	61.87	768.	0.	4
118	0.	0.	19705200.	26485.	60.32	729.	0.	4
119	0.	0.	21032600.	29212.	61.68	766.	0.	4
120	0.	0.	21618100.	29056.	61.53	744.	0.	4

DESIGNED:		CHECKED:	
DRAWN:		NOTED:	
K.H.		K.H.	
SUPERVISOR:		DATE: 14 JUL 1966	
SCALE: NONE		DATE: AUGUST 1966	
FILE NUMBER:		0-4-5-113	
SHEET 2 OF 3		DESIGN MEMORANDUM NO 7 APPENDIX A PLATE 10	

FLOW		ELEVATION		HEAD		STORAGE		SECONDARY ENERGY		PRIMARY POWER		KWH/AF	
NATL CFS		REG CFS		AVERAGE FTMSL		AVERAGE FT		SPILL AF		ENERGY KWH		KW	
PROJECT NO 2				YEAR OF STUDY 2023									
JAN	182	444	809.87	803.19	809.32	331816	0	0	20043300	26940	58.04	702	
FEB	68	478	784.26	785.32	793.32	308277	0	0	18265400	27180	56.80	687	
MAR	47	456	774.81	784.30	774.28	284716	0	0	18783000	25246	55.27	668	
APR	142	463	756.08	747.22	755.41	264456	0	0	18041100	25057	53.77	650	
MAY	318	444	749.77	751.64	749.17	248999	0	0	17591400	23644	53.23	644	
JUN	885	427	745.88	740.12	745.40	202166	0	0	16781000	23307	54.50	658	
JUL	925	403	742.59	805.06	742.12	334090	0	0	17096000	22978	56.62	685	
AUG	934	402	816.83	824.59	816.35	366751	0	0	17544200	23583	58.54	708	
SEP	501	429	824.91	831.23	829.42	370882	0	0	18446000	25619	58.59	721	
OCT	706	440	836.43	841.63	835.93	370789	0	0	19705200	26485	60.08	727	
NOV	240	489	856.96	852.29	856.35	372673	0	0	21032600	29212	60.19	727	
DEC	135	480	825.00	817.71	824.39	350806	0	0	21618100	29056	59.22	716	
PROJECT NO 3				YEAR OF STUDY 2024									
JAN	86	465	808.24	788.76	807.68	322265	0	0	20043300	26940	57.91	700	
FEB	68	481	789.51	780.27	788.81	302156	0	0	18917700	27180	56.42	682	
MAR	57	469	769.53	758.80	768.99	277563	0	0	18783000	25246	56.86	663	
APR	86	470	749.48	740.12	748.88	255167	0	0	18041100	25057	53.23	644	
MAY	403	451	738.87	737.62	738.35	232127	0	0	17591400	23644	52.33	639	
JUN	855	439	747.92	758.23	747.43	274880	0	0	16781000	23307	53.04	641	
JUL	1143	414	776.85	795.47	776.37	321657	0	0	17096000	22978	55.19	670	
AUG	845	408	805.76	814.05	805.28	348475	0	0	17544200	23583	57.68	697	
SEP	551	430	818.49	820.93	817.99	355303	0	0	18446000	25619	58.74	710	
OCT	555	448	822.83	824.74	822.32	360640	0	0	19705200	26485	59.08	716	
NOV	259	496	819.68	814.58	819.03	364697	0	0	21032600	29212	58.81	711	
DEC	190	502	807.03	799.48	806.39	327277	0	0	21618100	29056	57.81	699	
PROJECT NO 2				YEAR OF STUDY 2025									
JAN	86	476	790.21	780.95	789.62	303236	0	0	20043300	26940	56.48	683	
FEB	52	478	770.32	759.69	769.70	278638	0	0	18265400	27180	56.91	684	
MAR	68	479	749.28	738.87	748.70	251485	0	0	18783000	25246	55.21	663	
APR	139	485	729.90	720.93	729.29	233022	0	0	18041100	25057	51.59	624	
MAY	448	465	720.37	719.81	719.81	231800	0	0	17591400	23644	50.75	616	
JUN	880	452	725.46	731.10	724.93	284722	0	0	16781000	23307	51.14	619	
JUL	986	435	745.21	759.33	744.72	321657	0	0	17096000	22978	52.81	639	
AUG	916	428	772.10	784.87	771.61	308117	0	0	17544200	23583	54.99	665	
SEP	925	430	795.85	806.84	795.34	320331	0	0	18446000	25619	56.88	688	
OCT	773	453	816.20	821.57	815.68	336581	0	0	19705200	26485	58.34	705	
NOV	317	498	817.62	813.68	817.00	345423	0	0	21032600	29212	58.64	709	
DEC	276	501	806.31	803.95	807.68	331543	0	0	21618100	29056	57.91	706	
PROJECT NO 2				YEAR OF STUDY 2026									
JAN	123	474	794.53	784.12	793.95	309956	0	0	20043300	26940	56.82	687	
FEB	119	469	777.77	769.42	777.15	289364	0	0	18265400	27180	55.45	671	
MAR	94	466	759.47	749.52	758.90	264430	0	0	18783000	25246	54.06	654	
APR	136	475	742.48	735.39	741.87	249475	0	0	18041100	25057	52.64	637	
MAY	479	453	736.04	736.49	735.51	231031	0	0	17591400	23644	52.11	630	
JUN	960	438	749.74	762.80	749.25	282080	0	0	16781000	23307	52.48	631	
JUL	1282	410	786.50	806.20	784.02	335478	0	0	17096000	22978	56.00	677	
AUG	1000	397	827.26	814.13	824.78	340128	0	0	17544200	23583	59.37	718	
SEP	679	418	852.74	857.15	852.25	412849	0	0	18446000	25619	61.28	741	
OCT	906	429	865.55	873.55	864.86	442922	0	0	19705200	26485	62.19	752	
NOV	194	467	868.02	864.49	866.53	426897	0	0	21032600	29212	62.42	755	
DEC	53	471	857.34	850.19	856.76	400345	0	0	21618100	29056	61.68	747	
PROJECT NO 2				YEAR OF STUDY 2027									
JAN	173	443	844.69	839.19	844.19	383708	0	0	20043300	26940	60.73	736	
FEB	74	450	832.58	825.98	832.06	362578	0	0	18265400	27180	59.80	723	
MAR	87	430	818.19	810.40	817.70	341488	0	0	18783000	25246	58.71	710	
APR	80	436	802.40	794.41	801.91	320295	0	0	18041100	25057	57.45	695	
MAY	288	418	791.16	787.92	790.68	312766	0	0	17591400	23644	56.55	684	
JUN	753	405	785.93	803.94	785.43	360851	0	0	16781000	23307	56.90	688	
JUL	851	391	814.41	824.89	813.95	332710	0	0	17096000	22978	59.37	718	
AUG	787	394	832.44	839.90	831.97	384989	0	0	17544200	23583	59.78	723	
SEP	1042	418	851.12	862.26	850.64	422069	0	0	18446000	25619	61.16	740	
OCT	559	426	864.52	866.79	864.03	430328	0	0	19705200	26485	62.13	751	
NOV	383	469	865.36	863.93	864.79	425073	0	0	21032600	29212	62.19	752	
DEC	284	469	860.75	857.58	860.18	413647	0	0	21618100	29056	61.84	748	
PROJECT NO 2				YEAR OF STUDY 2028									
JAN	117	452	852.23	846.88	851.71	395912	0	0	20043300	26940	61.21	740	
FEB	203	449	842.14	837.41	841.63	380555	0	0	18265400	27180	60.51	732	
MAR	103	422	831.26	825.11	830.77	361188	0	0	18783000	25246	59.70	722	
APR	109	426	818.08	809.09	817.60	342275	0	0	18041100	25057	58.70	710	
MAY	366	407	810.08	804.09	809.60	334716	0	0	17591400	23644	58.05	702	
JUN	699	398	815.83	822.58	815.36	357613	0	0	16781000	23307	58.47	707	
JUL	892	384	832.65	842.73	832.19	388853	0	0	17096000	22978	59.80	723	
AUG	715	386	848.87	855.01	848.41	410926	0	0	17544200	23583	61.00	738	
SEP	1168	410	867.52	880.03	867.04	450708	0	0	18446000	25619	62.35	754	
OCT	527	417	881.71	883.39	881.23	460782	0	0	19705200	26485	63.36	766	
NOV	128	467	878.04	872.49	877.45	440897	0	0	21032600	29212	63.09	761	
DEC	93	466	866.30	859.92	865.74	417864	0	0	21618100	29056	62.24	753	
PROJECT NO 2				YEAR OF STUDY 2029									
JAN	47	439	859.00	846.08	852.50	393730	0	0	20043300	26940	61.29	741	
FEB	46	450	838.74	831.40	838.27	371248	0	0	18265400	27180	60.29	728	
MAR	47	427	823.91	815.62	821.02	347880	0	0	18783000	25246	59.11	715	
APR	122	432	808.39	801.16	807.90	329402	0	0	18041100	25057	57.93	700	
MAY	239	414	797.08	793.00	796.60	318602	0	0	17591400	23644	57.02	689	
JUN	816	405	802.33	816.66	801.84	403001	0	0	16781000	23307	57.41	694	
JUL	807	390	820.71	829.76	820.25	368628	0	0	17096000	22978	58.88	712	
AUG	1011	389	841.77	853.78	841.31	406820	0	0	17544200	23583	60.49	731	
SEP	982	412	863.19	872.60	862.71	420581	0	0	18446000	25619	62.04	750	
OCT	605	420	875.74	878.89	875.26	432462	0	0	19705200	26485	62.93	761	
NOV	137	465	873.46	868.83	872.90	432462	0	0	21032600	29212	63.76	759	
DEC	82	469	861.41	854.80	860.18	409499	0	0	21618100	29056	61.84	748	
PROJECT NO 2				YEAR OF STUDY 2030									
JAN	83	442	847.95	841.11	847.45	385599	0	0	20043300	26940	60.93	737	
FEB	123	452	835.31	829.52	834.79	368238	0	0	18265400	27180	60.01	726	
MAR	49	428	821.27	813.02	820.90	342275	0	0	18783000	25246	58.93	713	
APR	63	434	804.78	797.18	802.34	322866	0	0	18041100	25057	57.84	697	
MAY	316	416	793.82	791.35	793.34	316623	0	0	17591400	23644	56.84	688	
JUN	656	409	797.05	802.76	796.58	333137	0	0	16781000	23307	56.97	689	
JUL	984	393	811.13	821.74	812.19	381946	0	0	17096000				

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NOTES

1. STORAGE EOM EQUALS THE STORAGE IN THE RESERVOIR AT THE END OF THE MONTH
2. WASTE IS THE NONUSABLE OUTFLOW WASTED OVER THE SPILLWAY
3. YEARS ARE PROJECTED INTO THE ASSUMED TIME OF PROJECT OPERATION WITH 1983 EQUIVALENT TO THE FIRST YEAR OF PERIOD OF RECORD
4. LONG LAKE PRODUCES 225 MILLION KWH YEAR PRIMARY ENERGY AND 10.8 MILLION KWH YEAR OF SECONDARY ENERGY WITH BENEFIT OF RULE CURVE OPERATION. SEE PLATES 2 & 3 AND TABLES 7 & 10

DESIGNED:		DATE:		DESCRIPTION:		BY:	
U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA							
DRAWN:		SNETTISHAM, ALASKA					
CHECKED:		LONG LAKE					
PREPARED:		MONTHLY REGULATION COMPUTATIONS					
SUPERVISED:		APPROVED:		SCALE:		DATE:	
SUBMITTED:		FILE NUMBER:		Q-4-5-113			
RECOMMENDED:		SHEET:		3 OF 3			

CORPS OF ENGINEERS

[illegible]

CRATER LAKE
MONTHLY REGULATION COMPUTATIONS:
W/RULE CURVE OPERATION

SECONDARY	ENERGY	PRIMARY	POWER	WASTE	TAIL
WELL	NUM	ENERGY	KW	W/AF	WATER
NO		NUM			
1	9444700.	12694.	69.77	795.	0.
2	8606900.	12807.	67.74	771.	0.
3	8850840.	11876.	61.66	766.	0.
4	8501250.	11807.	59.10	718.	0.
5	8289350.	11141.	58.08	702.	0.
6	7907470.	10982.	58.44	709.	0.
7	8055950.	10827.	61.46	763.	0.
8	8268070.	11113.	65.23	788.	0.
9	8692050.	12072.	68.69	831.	0.
10	9285410.	12480.	70.49	852.	0.
11	9910870.	13765.	70.82	856.	0.
12	10186800.	13692.	70.26	850.	0.
13	9444700.	12694.	69.97	834.	0.
14	8606900.	12807.	67.92	814.	0.
15	8850840.	11876.	65.59	793.	0.
16	8501250.	11807.	63.78	771.	0.
17	8289350.	11141.	63.18	764.	0.
18	7907470.	10982.	64.91	785.	0.
19	8055950.	10827.	66.18	827.	0.
20	8268070.	11113.	72.45	876.	0.
21	8692050.	12072.	73.35	887.	15030.
22	9285410.	12480.	73.43	888.	0.
23	9910870.	13765.	73.60	885.	0.
24	10186800.	13692.	72.41	876.	0.
25	9444700.	12694.	71.24	862.	0.
26	8606900.	12807.	70.37	851.	0.
27	8850840.	11876.	68.22	829.	0.
28	8501250.	11807.	66.68	806.	0.
29	8289350.	11141.	65.88	797.	0.
30	7907470.	10982.	66.63	804.	0.
31	8055950.	10827.	68.63	830.	0.
32	8268070.	11113.	70.68	855.	0.
33	8692050.	12072.	72.32	872.	0.
34	9285410.	12480.	72.46	876.	0.
35	9910870.	13765.	71.74	868.	0.
36	10186800.	13692.	70.26	850.	0.
37	9444700.	12694.	69.01	835.	0.
38	8606900.	12807.	67.76	819.	0.
39	8850840.	11876.	66.25	801.	0.
40	8501250.	11807.	64.59	781.	0.
41	8289350.	11141.	63.84	772.	0.
42	7907470.	10982.	65.22	789.	0.
43	8055950.	10827.	68.37	827.	0.
44	8268070.	11113.	71.30	862.	0.
45	8692050.	12072.	72.50	882.	0.
46	9285410.	12480.	73.43	888.	0.
47	9910870.	13765.	73.33	886.	0.
48	10186800.	13692.	72.42	876.	0.
49	9444700.	12694.	71.22	861.	0.
50	8606900.	12807.	69.70	815.	0.
51	8850840.	11876.	68.23	825.	0.
52	8501250.	11807.	66.69	807.	0.
53	8289350.	11141.	65.44	791.	0.
54	7907470.	10982.	66.14	800.	0.
55	8055950.	10827.	68.67	831.	0.
56	8268070.	11113.	71.18	861.	0.
57	8692050.	12072.	72.92	882.	0.
58	9285410.	12480.	73.35	887.	15030.
59	9910870.	13765.	73.43	888.	0.
60	10186800.	13692.	72.45	876.	0.
61	9444700.	12694.	71.20	861.	0.
62	8606900.	12807.	69.57	815.	0.
63	8850840.	11876.	67.95	822.	0.
64	8501250.	11807.	66.36	802.	0.
65	8289350.	11141.	65.15	788.	0.
66	7907470.	10982.	65.56	791.	0.
67	8055950.	10827.	67.72	819.	0.
68	8268070.	11113.	70.57	853.	0.
69	8692050.	12072.	72.87	887.	0.
70	9285410.	12480.	73.43	888.	0.
71	9910870.	13765.	73.51	888.	0.
72	10186800.	13692.	72.58	876.	1.9
73	9444700.	12694.	71.25	862.	0.
74	8606900.	12807.	69.69	813.	0.
75	8850840.	11876.	68.20	825.	0.
76	8501250.	11807.	66.67	806.	0.
77	8289350.	11141.	65.92	797.	0.
78	7907470.	10982.	66.66	806.	0.
79	8055950.	10827.	68.81	832.	0.
80	8268070.	11113.	71.62	866.	0.
81	8692050.	12072.	73.22	886.	1.2
82	9285410.	12480.	73.35	887.	15030.
83	9910870.	13765.	72.56	882.	0.
84	10186800.	13692.	71.84	865.	0.
85	9444700.	12694.	78.30	859.	0.
86	8606900.	12807.	68.63	832.	0.
87	8850840.	11876.	67.18	812.	0.
88	8501250.	11807.	65.46	792.	0.
89	8289350.	11141.	64.15	778.	0.
90	7907470.	10982.	64.39	776.	0.
91	8055950.	10827.	66.20	801.	0.
92	8268070.	11113.	68.37	827.	0.
93	8692050.	12072.	70.37	853.	0.
94	9285410.	12480.	72.37	875.	0.
95	9910870.	13765.	72.38	875.	0.
96	10186800.	13692.	71.05	859.	0.
97	9444700.	12694.	69.43	840.	0.
98	8606900.	12807.	67.76	819.	0.
99	8850840.	11876.	66.90	798.	0.
100	8501250.	11807.	64.90	778.	0.
101	8289350.	11141.	63.19	771.	0.
102	7907470.	10982.	64.15	778.	0.
103	8055950.	10827.	65.99	798.	0.
104	8268070.	11113.	68.27	826.	0.
105	8692050.	12072.	69.20	843.	0.
106	9285410.	12480.	70.25	840.	0.
107	9910870.	13765.	70.35	851.	0.
108	10186800.	13692.	69.46	840.	0.
109	9444700.	12694.	67.79	829.	0.
110	8606900.	12807.	65.92	797.	0.
111	8850840.	11876.	63.88	772.	0.
112	8501250.	11807.	61.72	744.	0.
113	8289350.	11141.	59.98	715.	0.
114	7907470.	10982.	60.75	735.	0.
115	8055950.	10827.	63.85	770.	0.
116	8268070.	11113.	67.19	815.	0.
117	8692050.	12072.	70.28	850.	0.
118	9285410.	12480.	71.54	865.	0.
119	9910870.	13765.	71.54	865.	0.
120	10186800.	13692.	70.45	852.	0.

DESIGNED		DATE		REVISION		BY
<p align="center">U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA</p>						
<p align="center">SNETTISHAM, ALASKA CRATER LAKE MONTHLY REGULATION COMPUTATIONS</p>						
SUPERVISOR <i>[Signature]</i> ENGINEER <i>[Signature]</i> DRAFTER <i>[Signature]</i> CHECKED <i>[Signature]</i> REVISIONS <i>[Signature]</i>	APPROVED <i>[Signature]</i> SCALE NONE DATE AUGUST 1965 FILE NUMBER Q-4-5-113 SHEET 1 OF 5					

POOL													POOL																		
FLOW		ELEVATION		HEAD		STORAGE		ENERGY		POWER		WASTE		TAIL		ELEVATION		HEAD		STORAGE		ENERGY		POWER		WASTE		TAIL			
NATL CFS		REG CFS		AVERAGE FT/MBL		AVERAGE FT/MBL		SPILL AF		KWH		KWH/AF		AF		NATL CFS		REG CFS		AVERAGE FT/MBL		AVERAGE FT/MBL		SPILL AF		KWH		KWH/AF		AF	
PROJECT NO. 4													PROJECT NO. 4																		
YEAR OF STUDY 2000													YEAR OF STUDY 2001																		
JAN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	JAN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
FEB	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	FEB	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
MAR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	MAR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
APR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	APR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
MAY	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	MAY	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
JUN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	JUN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
JUL	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	JUL	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
AUG	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	AUG	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
SEP	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	SEP	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
OCT	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	OCT	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
NOV	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	NOV	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
DEC	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	DEC	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
PROJECT NO. 4													PROJECT NO. 4																		
YEAR OF STUDY 2001													YEAR OF STUDY 2002																		
JAN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	JAN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
FEB	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	FEB	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
MAR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	MAR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
APR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	APR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
MAY	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	MAY	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
JUN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	JUN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
JUL	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	JUL	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
AUG	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	AUG	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
SEP	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	SEP	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
OCT	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	OCT	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
NOV	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	NOV	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
DEC	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	DEC	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
PROJECT NO. 4													PROJECT NO. 4																		
YEAR OF STUDY 2002													YEAR OF STUDY 2003																		
JAN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	JAN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
FEB	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	FEB	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
MAR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	MAR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
APR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	APR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
MAY	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	MAY	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
JUN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	JUN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
JUL	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	JUL	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
AUG	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	AUG	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
SEP	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	SEP	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
OCT	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	OCT	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
NOV	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	NOV	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
DEC	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	DEC	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
PROJECT NO. 4													PROJECT NO. 4																		
YEAR OF STUDY 2003													YEAR OF STUDY 2004																		
JAN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	JAN	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
FEB	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	FEB	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
MAR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	MAR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
APR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	APR	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0		
MAY	13	183	980.00	980.00	980.00	980.00	980.00	0.0	0.0	980.00	126.94	69.07	855.0	0.0	0.0	MAY	13	183	980.00	980.00											

STORAGE EOM	SECONDARY		PRIMARY		WASTE		TAIL WATER
	SPILL AF	ENERGY KWH	ENERGY KWH	POWER KW	KWH/AF KWCPS	AF	
JULY 2010							
100001.	21%	18157.	7444700.	12694.	71.25	862.	0.4
800001.	0.	0.	8000900.	12807.	69.69	843.	0.4
81413.	0.	0.	8850840.	11876.	68.10	874.	0.4
71000.	0.	0.	8501250.	11807.	66.67	806.	0.4
71000.	24%	190174.	8289150.	11141.	65.92	797.	0.4
87000.	24%	190174.	7907470.	10982.	64.57	806.	0.4
100001.	121%	7081270.	8055950.	10827.	64.96	834.	0.4
112301.	0.	0.	8289150.	11141.	71.41	864.	0.4
121000.	0.	0.	8692050.	12072.	73.02	883.	0.4
121000.	541%	659920.	8289150.	11141.	73.41	888.	20025. 1.0
115742.	0.	0.	9910870.	13765.	73.07	864.	0.4
104184.	0.	0.	10186800.	13692.	71.68	867.	0.4
JULY 2011							
82677.	0.	0.	7444700.	12694.	70.36	848.	0.4
81126.	0.	0.	8000900.	12807.	68.90	828.	0.4
73113.	0.	0.	8850840.	11876.	66.83	808.	0.4
84478.	0.	0.	8501250.	11807.	65.11	787.	0.4
84478.	0.	0.	8289150.	11141.	64.70	782.	0.4
82000.	20%	183300.	7907470.	10982.	64.34	802.	0.4
97495.	0.	0.	8055950.	10827.	66.75	831.	0.4
111474.	0.	0.	8289150.	11141.	71.43	864.	0.4
121000.	35%	3173020.	8692050.	12072.	73.13	884.	0.4
121000.	541%	4607170.	8289150.	11141.	73.41	888.	0.4
117501.	70%	824632.	9910870.	13765.	73.22	866.	0.4
108074.	0.	0.	10186800.	13692.	72.32	879.	0.4
JULY 2012							
88490.	0.	0.	7444700.	12694.	70.47	858.	0.4
88977.	0.	0.	8914290.	12807.	69.66	840.	0.4
81484.	0.	0.	8850840.	11876.	68.23	823.	0.4
73000.	801%	646225.	8501250.	11807.	66.65	806.	0.4
73000.	24%	190174.	8289150.	11141.	65.92	797.	0.4
87000.	24%	190174.	7907470.	10982.	64.55	806.	0.4
97095.	0.	0.	8055950.	10827.	66.75	831.	0.4
107737.	0.	0.	8289150.	11141.	70.81	856.	0.4
121000.	502%	7154330.	8692050.	12072.	72.85	881.	0.4
121000.	743%	659920.	8289150.	11141.	73.36	887.	0.4
115742.	0.	0.	9910870.	13765.	73.07	864.	0.4
108824.	0.	0.	10186800.	13692.	72.25	874.	0.4
JULY 2013							
100001.	302%	483330.	7444700.	12694.	71.15	860.	0.4
83909.	17%	14444.	8000900.	12807.	69.82	844.	0.4
81484.	0.	0.	8850840.	11876.	68.17	824.	0.4
73000.	0.	0.	8501250.	11807.	66.58	805.	0.4
73000.	381%	1040810.	8289150.	11141.	65.82	786.	0.4
87000.	891%	7181420.	7907470.	10982.	64.58	805.	0.4
100001.	893%	578189.	8055950.	10827.	66.47	834.	0.4
113314.	0.	0.	8289150.	11141.	71.47	864.	0.4
121000.	914%	8119440.	8692050.	12072.	73.05	884.	4832. 1.0
121000.	234%	2078570.	8289150.	11141.	73.43	888.	0.4
117501.	288%	253172.	9910870.	13765.	73.22	866.	0.4
108820.	0.	0.	10186800.	13692.	72.36	879.	0.4
JULY 2014							
100001.	3.	2582.	7444700.	12694.	71.15	860.	0.4
83909.	0.	0.	8000900.	12807.	69.69	843.	0.4
81484.	0.	0.	8850840.	11876.	68.13	824.	0.4
73000.	15.	13043.	8501250.	11807.	66.75	807.	0.4
73000.	223%	1786820.	8289150.	11141.	65.94	797.	0.4
87000.	0.	0.	7907470.	10982.	64.55	806.	0.4
97165.	0.	0.	8055950.	10827.	66.75	831.	0.4
113465.	0.	0.	8289150.	11141.	71.28	862.	0.4
121000.	516%	5584630.	8692050.	12072.	73.07	884.	0.4
121000.	332%	2951900.	8289150.	11141.	73.43	888.	0.4
121000.	775%	6901120.	9910870.	13765.	73.35	867.	2539. 1.0
110001.	408%	1589760.	10186800.	13692.	72.64	878.	0.4
JULY 2015							
89457.	0.	0.	7444700.	12694.	71.22	861.	0.4
89995.	0.	0.	8000900.	12807.	69.62	842.	0.4
80163.	0.	0.	8850840.	11876.	68.01	822.	0.4
71000.	0.	0.	8501250.	11807.	66.37	803.	0.4
68072.	0.	0.	8289150.	11141.	65.27	789.	0.4
73000.	0.	0.	7907470.	10982.	64.55	786.	0.4
73000.	0.	0.	8055950.	10827.	66.17	824.	0.4
84411.	0.	0.	8289150.	11141.	70.40	851.	0.4
105551.	0.	0.	8692050.	12072.	73.07	884.	0.4
115000.	514%	4481910.	8289150.	11141.	73.43	888.	0.4
111162.	0.	0.	9910870.	13765.	73.22	866.	0.4
101965.	0.	0.	10186800.	13692.	72.36	879.	0.4
91271.	0.	0.	10186800.	13692.	69.86	845.	0.4
JULY 2016							
80000.	0.	0.	7444700.	12694.	68.17	824.	0.4
70632.	0.	0.	8914290.	12807.	64.39	803.	0.4
80000.	0.	0.	8850840.	11876.	65.63	779.	0.4
51156.	0.	0.	8501250.	11807.	62.41	755.	0.4
49812.	0.	0.	8289150.	11141.	61.26	741.	0.4
61815.	0.	0.	7907470.	10982.	62.54	756.	0.4
83794.	0.	0.	8055950.	10827.	65.96	796.	0.4
91705.	0.	0.	8289150.	11141.	68.45	828.	0.4
92913.	0.	0.	8692050.	12072.	69.75	844.	0.4
95986.	0.	0.	8289150.	11141.	70.18	849.	0.4
86875.	0.	0.	9910870.	13765.	69.13	774.	0.4
76553.	0.	0.	10186800.	13692.	67.48	818.	0.4
JULY 2017							
65379.	0.	0.	7444700.	12694.	65.52	792.	0.4
54872.	0.	0.	8000900.	12807.	63.34	764.	0.4
43988.	0.	0.	8850840.	11876.	61.00	738.	0.4
54462.	0.	0.	8501250.	11807.	58.42	706.	0.4
32288.	0.	0.	8289150.	11141.	56.74	685.	0.4
34818.	0.	0.	7907470.	10982.	57.32	693.	0.4
54317.	0.	0.	8055950.	10827.	60.73	734.	0.4
74953.	0.	0.	8289150.	11141.	64.72	783.	0.4
92913.	0.	0.	8692050.	12072.	67.60	820.	0.4
111765.	0.	0.	8289150.	11141.	70.30	857.	0.4
112384.	0.	0.	9910870.	13765.	72.36	879.	0.4
103405.	0.	0.	10186800.	13692.	71.67	867.	0.4
JULY 2018							
92515.	0.	0.	7444700.	12694.	70.19	849.	0.4
84183.	0.	0.	8000900.	12807.	68.70	831.	0.4
74523.	0.	0.	8850840.	11876.	67.07	811.	0.4
83445.	0.	0.	8501250.	11807.	65.33	790.	0.4
88867.	0.	0.	8289150.	11141.	64.71	783.	0.4
82000.	24%	1985450.	7907470.	10982.	64.28	802.	0.4
93768.	0.	0.	8055950.	10827.	68.95	834.	0.4
117501.	784%	880730.	8289150.	11141.	71.75	868.	0.4
121000.	904%	8027710.	8692050.	12072.	73.21	885.	5145. 1.0
121000.	911%	8086980.	8289150.	11141.	73.35	887.	0.4
112794.	0.	0.	9910870.	13765.	72.48	861.	0.4
104198.	0.	0.	10186800.	13692.	71.75	868.	0.4
JULY 2019							
98460.	0.	0.	7444700.	12694.	70.34	851.	0.4
89303.	0.	0.	8000900.	12807.	69.29	837.	0.4
73828.	0.	0.	8850840.	11876.	67.93	822.	0.4
70418.	0.	0.	8501250.	11807.	66.29	802.	0.4
86924.	0.	0.	8289150.	11141.	65.01	786.	0.4
75229.	0.	0.	7907470.	10982.	65.48	792.	0.4
88977.	0.	0.	8055950.	10827.	67.50	819.	0.4
97774.	0.	0.	8289150.	11141.	69.25	837.	0.4
105772.	0.	0.	8692050.	12072.	70.18	846.	0.4
109401.	0.	0.	8289150.	11141.	71.82	869.	0.4
108590.	0.	0.	9910870.	13765.	71.83	869.	0.4
104184.	0.	0.	10186800.	13692.	71.44	864.	0.4

DESIGNED

DRAWN

CHECKED

APPROVED

DATE

DATE

DATE

DATE

REVISION

REVISION

REVISION

REVISION

BY

BY

BY

BY

U.S. ARMY ENGINEER DISTRICT, ALASKA

CORPS OF ENGINEERS

ANCHORAGE, ALASKA

SNETTISHAM, ALASKA

CRATER LAKE

MONTHLY REGULATION COMPUTATIONS

SHEET 2 OF 3

Q-4-5-113

DESIGN MEMORANDUM NO 7 APPENDIX A PLATE

DESIGNED:		U. S. ARMY ENGINEER DISTRICT, ALASKA	
DRAWN:		CORPS OF ENGINEERS	
CHECKED:		ANCHORAGE, ALASKA	
APPROVED:		SNETTISHAM, ALASKA	
SUBMITTED:		CRATER LAKE	
RECOMMENDED:		MONTHLY REGULATION COMPUTATIONS	
SCALE: NONE		DATE: AUGUST 1965	
SHEET: 2 OF 3		FILE NUMBER: Q-4-5-113	

PROJECT NO.	DATE OF SURV.	ELEVATION	CON		EOM		SECONDARY		ENERGY SURV.
			FEET	FT	FEET	FT	FEET	FT	
JAN 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
FEB 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAR 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
APR 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAY 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUN 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUL 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
AUG 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
SEP 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
OCT 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
NOV 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
DEC 33	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
PROJECT NO. 4									
JAN 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
FEB 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAR 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
APR 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAY 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUN 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUL 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
AUG 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
SEP 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
OCT 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
NOV 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
DEC 34	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
PROJECT NO. 4									
JAN 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
FEB 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAR 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
APR 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAY 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUN 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUL 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
AUG 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
SEP 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
OCT 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
NOV 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
DEC 35	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
PROJECT NO. 4									
JAN 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
FEB 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAR 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
APR 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAY 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUN 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUL 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
AUG 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
SEP 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
OCT 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
NOV 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
DEC 36	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
PROJECT NO. 4									
JAN 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
FEB 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAR 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
APR 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAY 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUN 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUL 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
AUG 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
SEP 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
OCT 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
NOV 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
DEC 37	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
PROJECT NO. 4									
JAN 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
FEB 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAR 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
APR 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAY 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUN 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUL 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
AUG 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
SEP 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
OCT 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
NOV 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
DEC 38	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
PROJECT NO. 4									
JAN 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
FEB 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAR 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
APR 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAY 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUN 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUL 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
AUG 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
SEP 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
OCT 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
NOV 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
DEC 39	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
PROJECT NO. 4									
JAN 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
FEB 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAR 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
APR 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAY 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUN 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUL 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
AUG 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
SEP 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
OCT 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
NOV 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
DEC 40	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
PROJECT NO. 4									
JAN 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
FEB 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAR 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
APR 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAY 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUN 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUL 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
AUG 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
SEP 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
OCT 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
NOV 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
DEC 41	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
PROJECT NO. 4									
JAN 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
FEB 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAR 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
APR 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAY 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUN 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
JUL 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
AUG 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
SEP 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
OCT 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
NOV 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
DEC 42	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
PROJECT NO. 4									
JAN 43	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
FEB 43	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
MAR 43	180	970.11	970.11	970.11	970.11	970.11	970.11	970.11	
APR									

CRATER LAKE

MONTHLY REGULATION COMPUTATION
W/RULE CURVE OPERATION

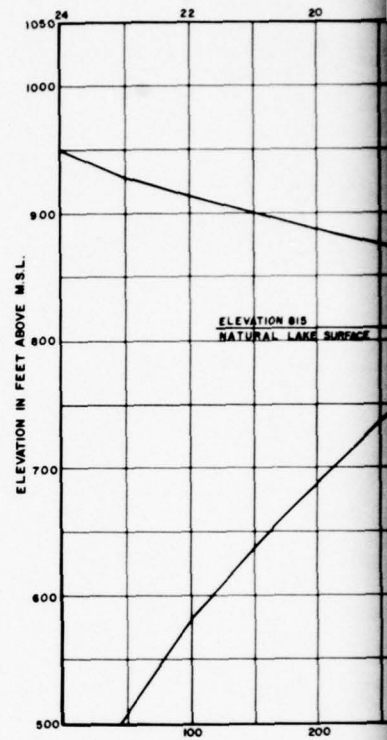
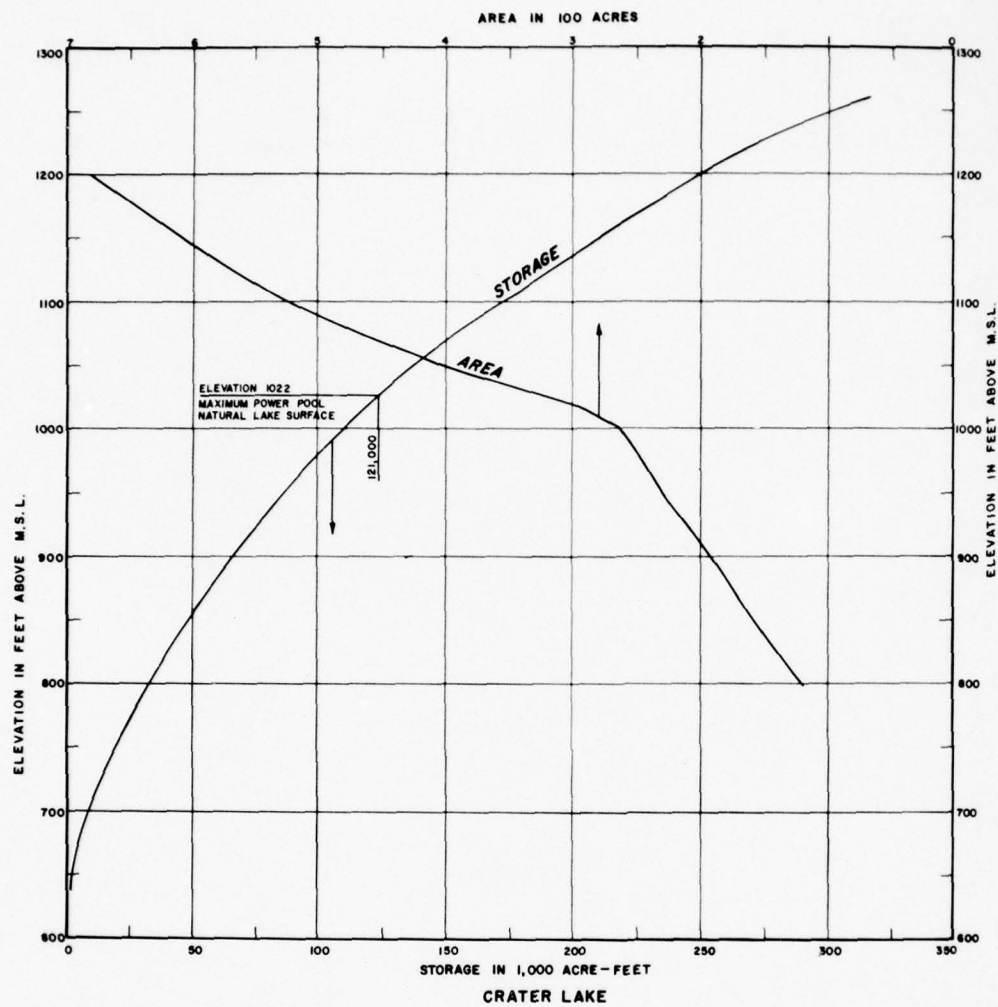
PRIMARY
POWER
KW
KWH/AF
WASTE
AF
TAIL
WATER

12694.	70.38	851.	0.	.4
12697.	68.95	834.	0.	.4
11899.	67.31	814.	0.	.4
11807.	65.58	798.	0.	.4
11141.	64.14	778.	0.	.4
10982.	64.20	776.	0.	.4
10827.	66.70	807.	0.	.4
11113.	70.27	890.	0.	.4
12072.	72.77	880.	0.	.4
12480.	73.27	886.	0.	.4
13765.	72.58	878.	0.	.4
13692.	71.48	864.	0.	.4
12694.	69.76	844.	0.	.4
12807.	68.08	823.	0.	.4
11899.	66.36	802.	0.	.4
11807.	64.68	780.	0.	.4
11141.	63.56	769.	0.	.4
10982.	63.94	773.	0.	.4
10827.	66.22	801.	0.	.4
11113.	70.27	890.	0.	.4
12072.	72.48	877.	0.	.8
12480.	72.49	877.	0.	.4
13765.	72.27	876.	0.	.4
13692.	71.85	860.	0.	.4
12694.	70.89	857.	0.	.4
12807.	69.51	841.	0.	.4
11899.	67.91	821.	0.	.4
11807.	66.27	801.	0.	.4
11141.	65.47	792.	0.	.4
10982.	64.45	804.	0.	.4
10827.	68.62	830.	0.	.4
11113.	70.73	855.	0.	.4
12072.	72.75	880.	0.	.4
12480.	73.42	888.	0.	.4
13765.	73.21	885.	0.	.4
13692.	72.95	875.	0.	.4
12694.	71.12	860.	0.	.4
12807.	69.71	843.	0.	.4
11899.	68.18	825.	0.	.4
11807.	66.65	808.	0.	.4
11141.	65.92	797.	0.	.4
10982.	66.54	805.	0.	2.2
10827.	68.97	834.	0.	.4
11113.	71.77	868.	0.	.4
12072.	72.76	880.	0.	.8
12480.	73.10	884.	0.	.4
13765.	73.04	883.	0.	.4
13692.	72.11	872.	0.	.4
12694.	70.77	856.	0.	.4
12807.	69.31	838.	0.	.4
11899.	67.74	819.	0.	.4
11807.	66.13	800.	0.	.4
11141.	65.20	789.	0.	.4
10982.	66.28	802.	0.	.4
10827.	68.87	833.	0.	1.7
11113.	71.58	866.	0.	.4
12072.	72.59	876.	0.	.4
12480.	72.82	881.	0.	.4
13765.	72.76	880.	0.	.4
13692.	71.93	870.	0.	.4
12694.	70.69	855.	0.	.4
12807.	69.28	838.	0.	.4
11899.	67.73	819.	0.	.4
11807.	66.19	800.	0.	.4
11141.	65.44	791.	0.	.4
10982.	66.47	804.	0.	.4
10827.	68.96	834.	0.	.5
11113.	71.75	868.	0.	.4
12072.	73.19	885.	2291.	1.8
12480.	73.35	887.	2255.	1.8
13765.	73.21	885.	0.	.4
13692.	72.41	876.	0.	.5
12694.	71.24	862.	0.	.5
12807.	69.71	843.	0.	.5
11899.	68.22	825.	0.	.4
11807.	66.69	807.	0.	.5
11141.	65.92	797.	0.	.4
10982.	66.57	805.	0.	1.9
10827.	69.10	836.	0.	2.2
11113.	72.77	880.	5041.	1.9
12072.	73.36	887.	0.	1.5
12480.	73.35	887.	6744.	1.8
13765.	72.96	882.	0.	.4
13692.	71.83	869.	0.	.4
12694.	70.55	853.	0.	.4
12807.	69.19	839.	0.	.4
11899.	67.91	821.	0.	.4
11807.	66.37	803.	0.	.4
11141.	65.12	787.	0.	.4
10982.	65.70	795.	0.	.4
10827.	68.10	824.	0.	.4
11113.	70.43	852.	0.	.4
12072.	72.73	880.	0.	.4
12480.	73.42	888.	0.	.4
13765.	73.22	885.	0.	.4
13692.	72.43	876.	0.	.4
12694.	71.23	861.	0.	.4
12807.	69.70	843.	0.	.4
11899.	68.22	825.	0.	.4
11807.	66.69	807.	0.	.4
11141.	65.75	795.	0.	.4
10982.	66.30	802.	0.	.4
10827.	68.59	829.	0.	.4
11113.	70.89	857.	0.	.4
12072.	72.88	881.	524.	1.8
12480.	73.36	887.	0.	1.5
13765.	73.20	885.	0.	.4
13692.	72.37	875.	0.	.4

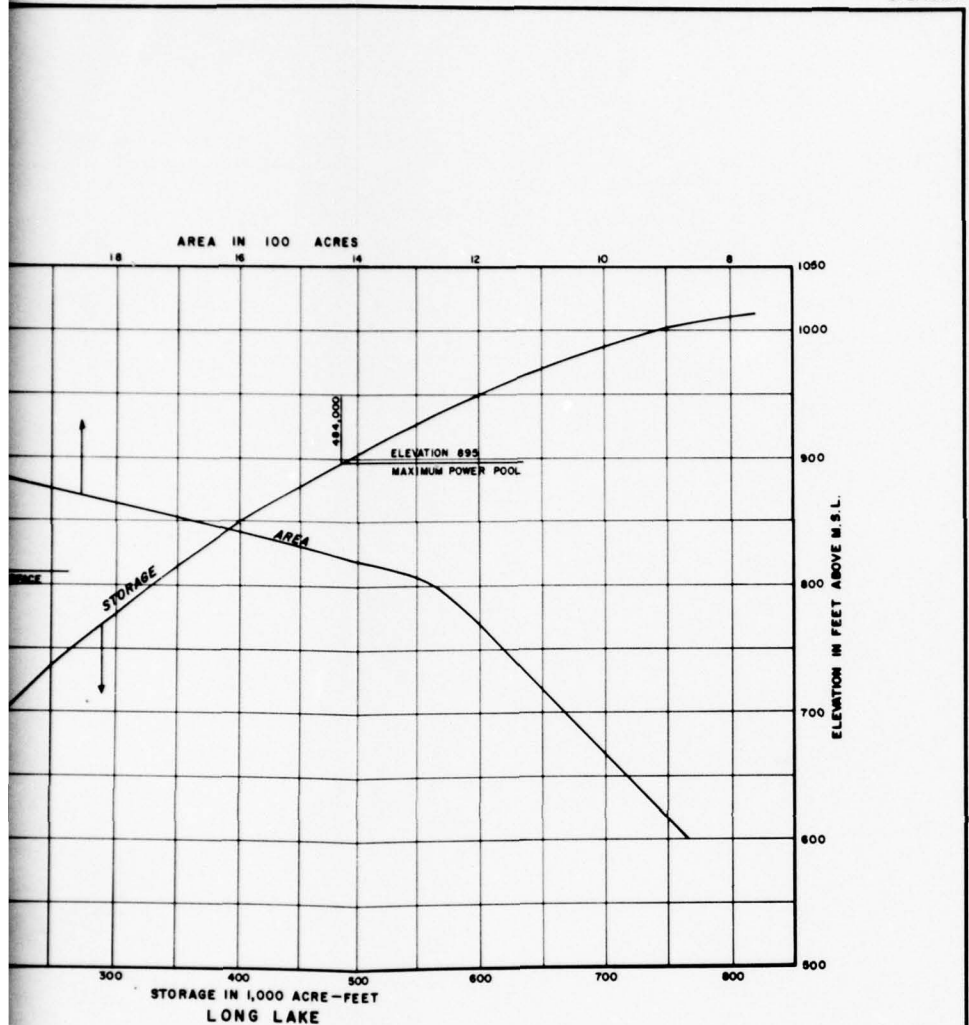
- NOTES:
1. STORAGE EOH EQUALS THE STORAGE IN THE RESERVOIR AT THE END OF THE MONTH.
 2. WASTE IS THE NONUSABLE OUTFLOW WASTED OVER THE SPILLWAY.
 3. YEARS ARE PROJECTED INTO THE ASSUMED TIME OF PROJECT OPERATION WITH 1980 EQUIVALENT TO THE FIRST YEAR OF PERIOD OF RECORD.
 4. CRATER LAKE PRODUCES 108 MILLION KWH/YEAR PRIMARY ENERGY AND 13.9 MILLION KWH/YEAR OF SECONDARY ENERGY WITH BENEFIT OF RULE CURVE OPERATION. SEE PLATES 4 & 5 AND TABLES 8 & 10.

12694.	71.19	863.	0.	.4
12807.	69.71	843.	0.	.4
11899.	68.22	825.	0.	.4
11807.	66.69	807.	0.	.5
11141.	65.92	797.	0.	.5
10982.	66.49	807.	0.	.4
10827.	71.52	865.	0.	.4
11113.	73.35	887.	33182.	1.8
12072.	73.35	887.	10275.	1.8
12480.	73.41	888.	0.	.8
13765.	73.10	884.	0.	.4
13692.	72.17	873.	0.	.4

U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
SNETTISHAM, ALASKA CRATER LAKE MONTHLY REGULATION COMPUTATIONS	
DESIGNED BY DRAWN BY CHECKED BY APPROVED BY DATE	SCALE DATE AUGUST 1988 FILE NUMBER SHEET 3 OF 3 0-4-5-113



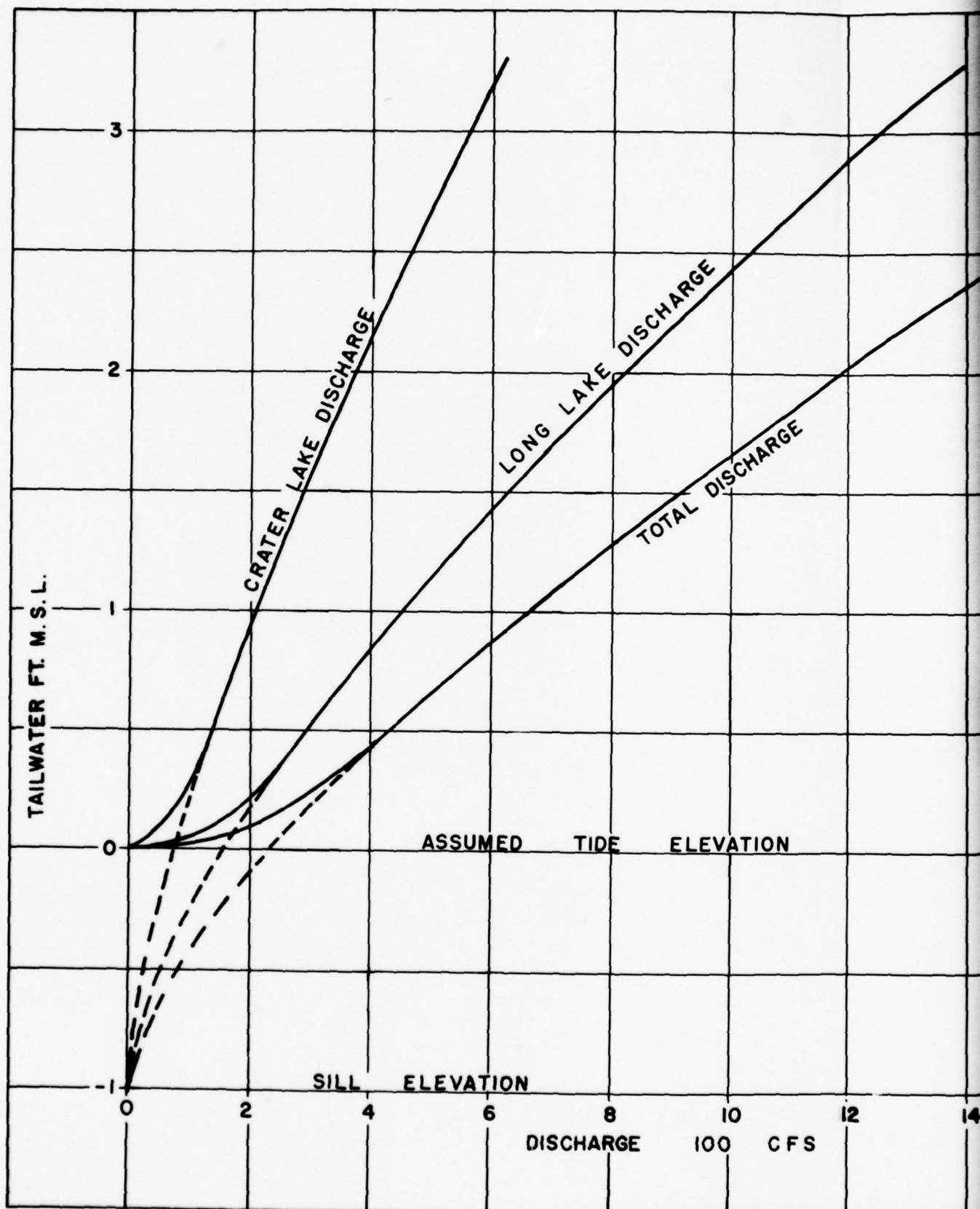
AREA - STORAGE CURVES



NOTES:

- The AREA-STORAGE curves are taken from House Document No. 40, 87th Congress, 1st Session. Updated with recent survey data.

U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: _____ DRAWN: _____ TRACED: _____ CHECKED: _____ PREPARED: _____ SUPERVISED: _____ SUBMITTED: _____ RECOMMENDED: _____ CORP. ENGINEERING DIVISION	
SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT AREA STORAGE CURVES	
APPROVED: _____ CHIEF, ALASKA DISTRICT ENGINEER	DATE 31 JULY 1968 FILE NUMBER Q-4-5-113
SHEET 1 OF 1	





NOTES:

1. Assumed sill elevation = - 1.0 MSL.
2. Assumed sill width = 76 Feet.
3. Rating curve based upon $Q = 3LH^{3/2}$.
4. Assumed discharge balanced in proportion to ave. annual inflow.
5. Assumed tide at 0.0.

U. S. ARMY ENGINEER DISTRICT, ALASKA
CORPS OF ENGINEERS
ANCHORAGE, ALASKA

SNETTISHAM PROJECT, ALASKA

**TAILWATER RATING
FOR
LONG & CRATER LAKES**

DESIGNED

DRAWN

TRACED

CHECKED

PREPARED

ell

SUPERVISED

M. D. DeLoe
CHIEF, POWER INVESTIGATIONS SECTION

SUBMITTED *James J. Smith*
CHIEF, PLANNING AND REPORTS BR.

RECOMMENDED *James J. Smith*
CHIEF, ENGINEERING DIVISION

APPROVED

Clare J. Farley
COLONEL, CE, DISTRICT ENGINEER

SCALE NONE

DATE 31 JULY 65

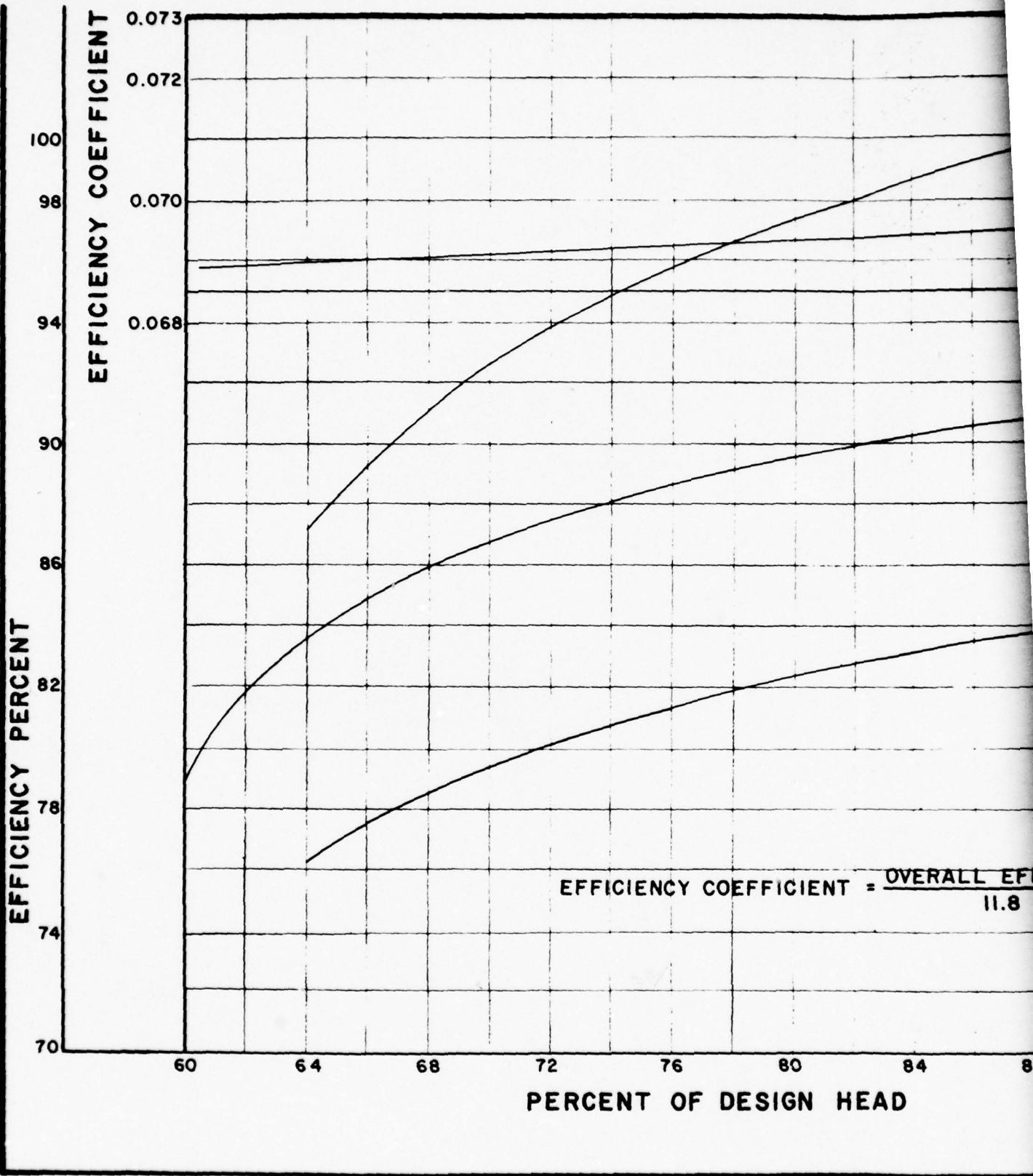
DRAWING NUMBER

Q-4-5-113

SHEET

OF

DESIGN MEMORANDUM NO. 7 APPENDIX A PLATE 16



EFFICIENCY COEFFICIENT

HYDRAULIC EFFICIENCY

GENERATOR EFFICIENCY

TURBINE EFFICIENCY

OVERALL EFFICIENCY

BASED ON EM1110-2-1701
& USBR ENG. MONOGRAPH #20

100 104 108 112

U. S. ARMY ENGINEER DISTRICT, ALASKA
CORPS OF ENGINEERS
ANCHORAGE, ALASKA

DESIGNED MDW

DRAWN

TRACES RTH D.

CHECKED MDW

PREPARED

CHIEF

SNETTISHAM PROJECT, ALASKA

FIRST STAGE DEVELOPMENT

PLANT EFFICIENCIES

92

96

SUPERVISED

CHIEF, POWER INVESTIGATIONS SECTION

SUBMITTED

CHIEF, PLANNING AND REPORTS BR.

RECOMMENDED

CHIEF, ENGINEERING DIVISION

APPROVED

COLONEL, CE, DISTRICT ENGINEER

SCALE NONE

DATE 31 Oct 64

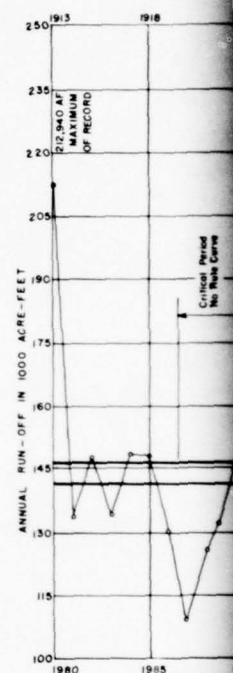
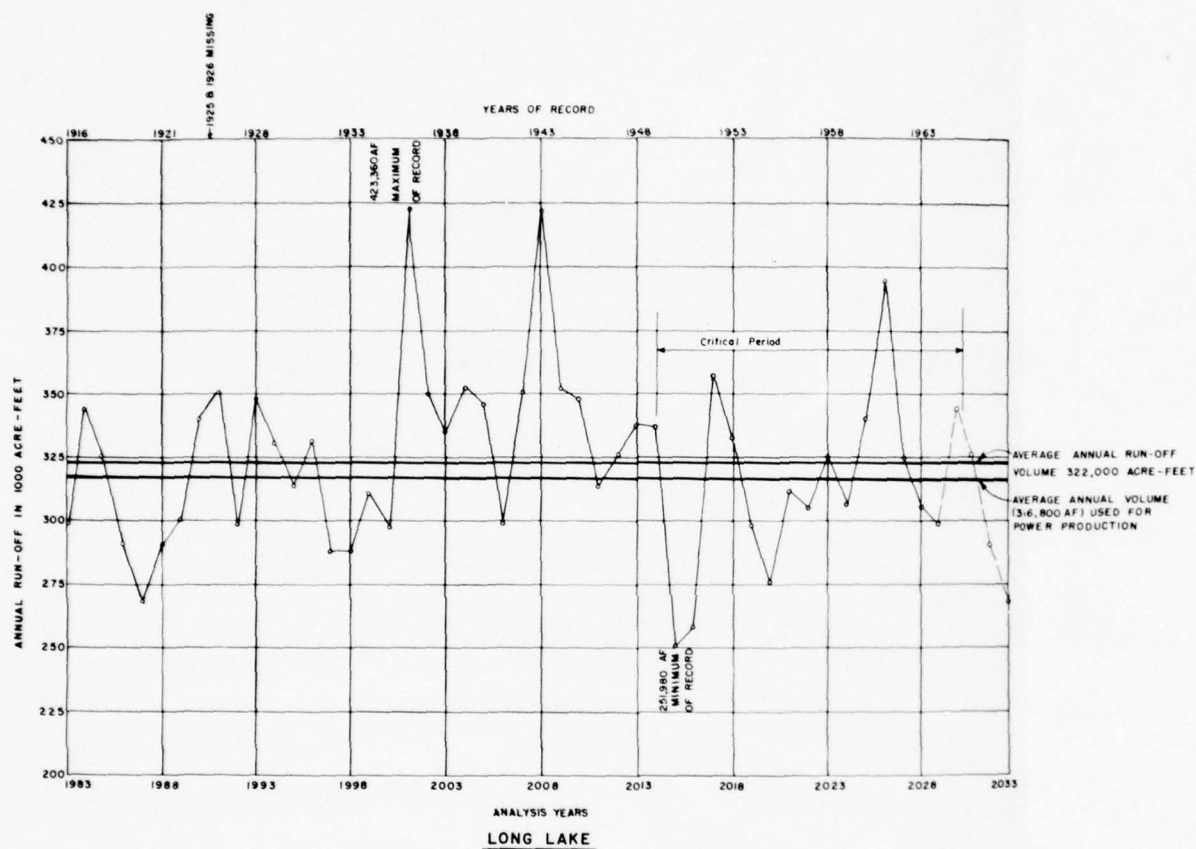
DRAWING NUMBER

Q-4-5-113

SHEET

OF

DESIGN MEMORANDUM NO. 7 APPENDIX A PLATE 17

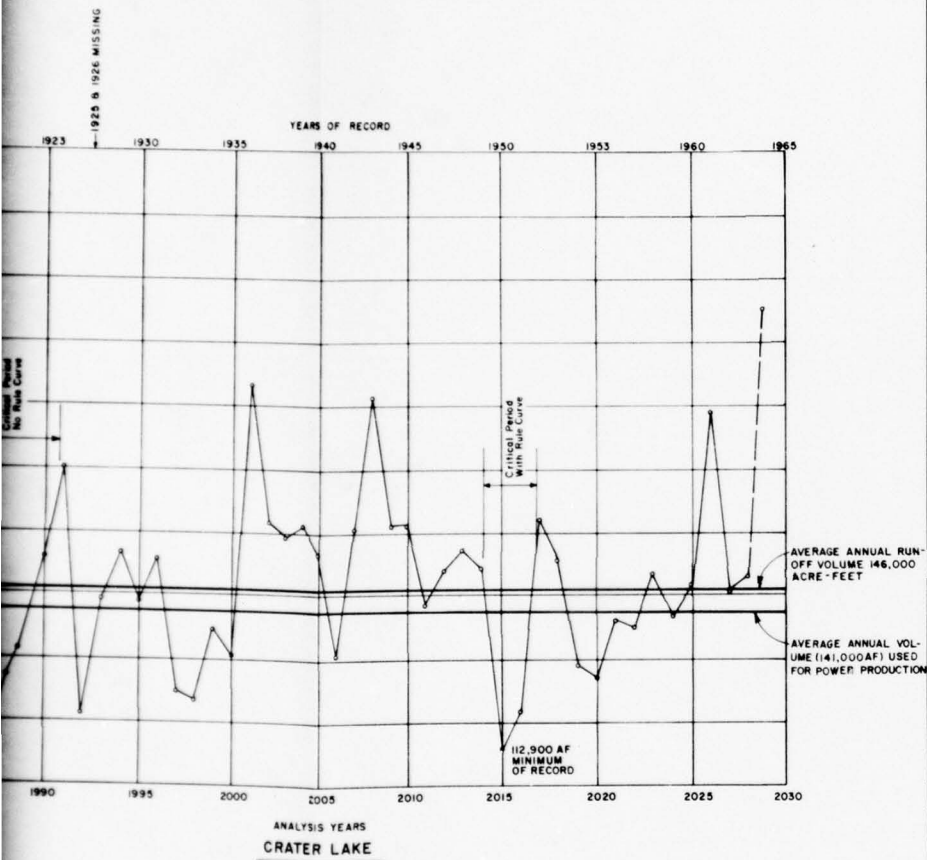


- NOTES
1. Years shown are projected into the assumed time of project operation with 1983 or equivalent to the first year of period of record.
 2. Years connected with dashed lines were reprinted from first of period of record to complete 50 year analysis.
 3. Power Production.

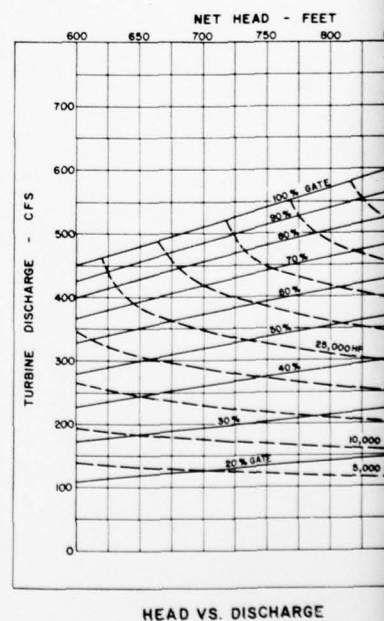
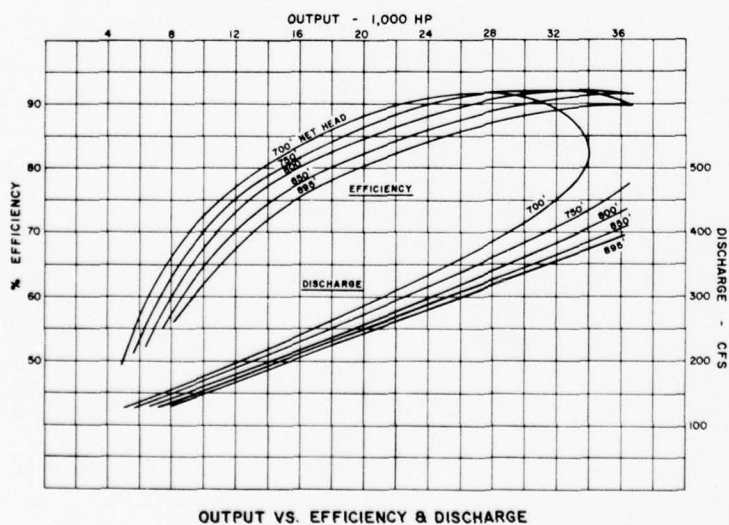
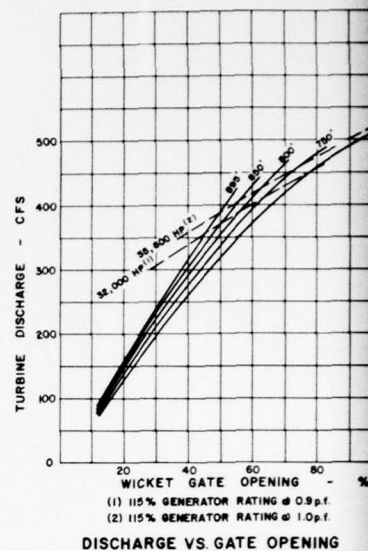
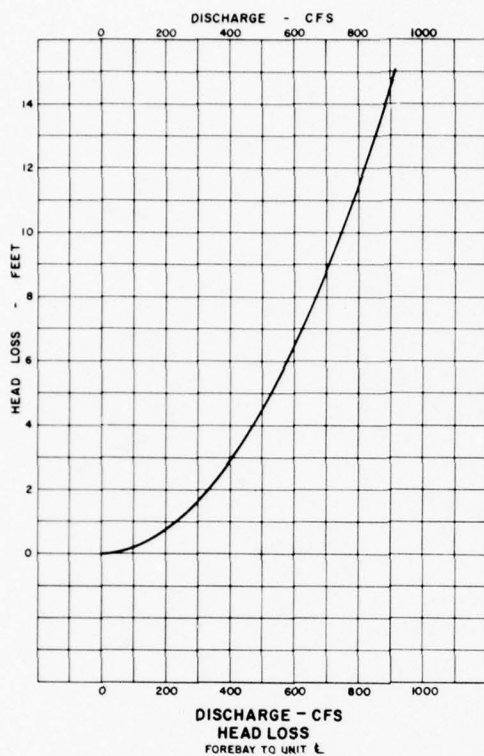
(1) Long Lake produces 225 million KWH/year primary energy and 9.8 million KWH of secondary energy without benefit of rule curve operation. See plates 2 & 3 and 6 & 7.

(2) Crater Lake produces 106 million KWH/year primary energy and 13.9 million KWH of secondary energy with rule curve operation. See plates 4 & 5 and tables 8 & 9.

PERIOD OF RECORD PERIOD OF ANALYSIS
 LONG LAKE 1916 - 1963 1983 - 2032 INCL.
 CRATER LAKE 1913 - 1963 1980 - 2029 INCL.



1 20-6-65 MINOR REV & UPDATING		MDW
U. S. ARMY ENGINEER DISTRICT ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA		
SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT ANNUAL RUN-OFF VOLUME		
DESIGNED E.A.		
DRAWN S. H. ANDERSON		
TRACED		
CHECKED GOS		
PREPARED GOS		
SUPERVISED W.D. Wade Jr.	APPROVED J. J. J. J.	
CHEF POWER INVESTIGATIONS SECTION	SCALE AS SHOWN DATE 31 JULY 1965	
SUBMITTED	FILE NUMBER Q-4-5-113	
CHEF PLANNING AND REPORTS BRANCH	SHEET 1 OF 1	
RECOMMENDED J. J. J. J.		
CHEF ENGINEERING DIVISION		

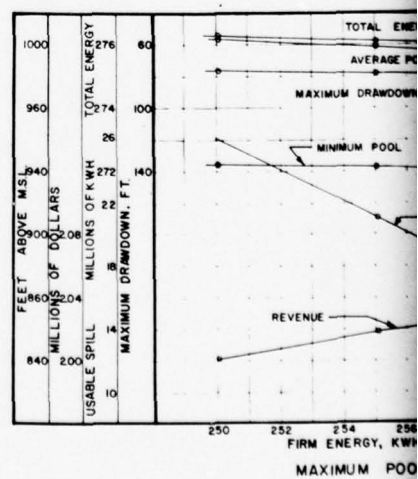
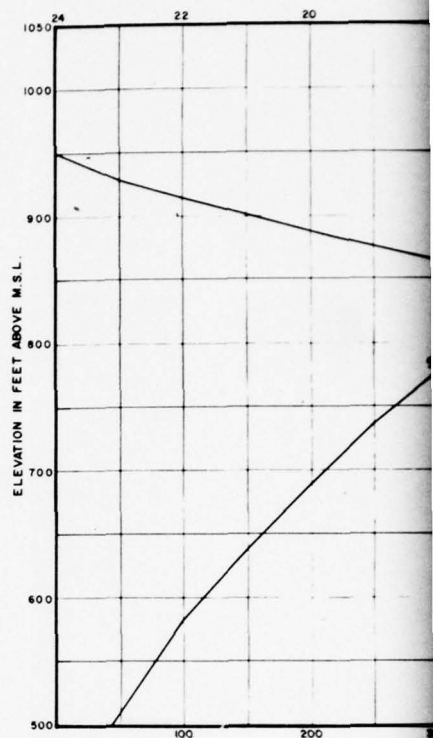
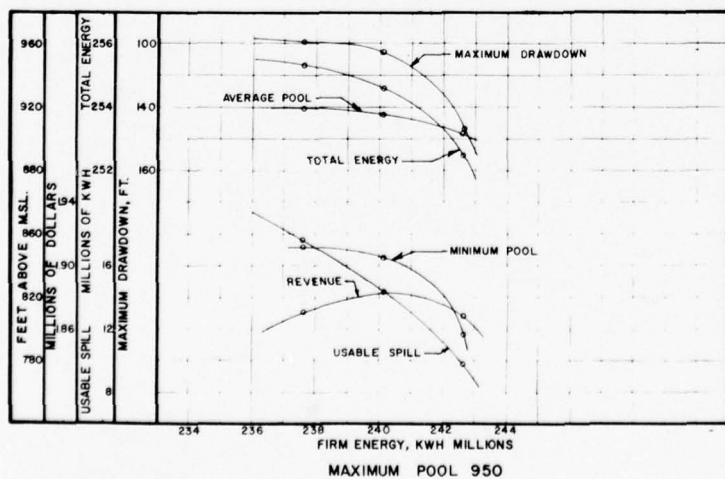
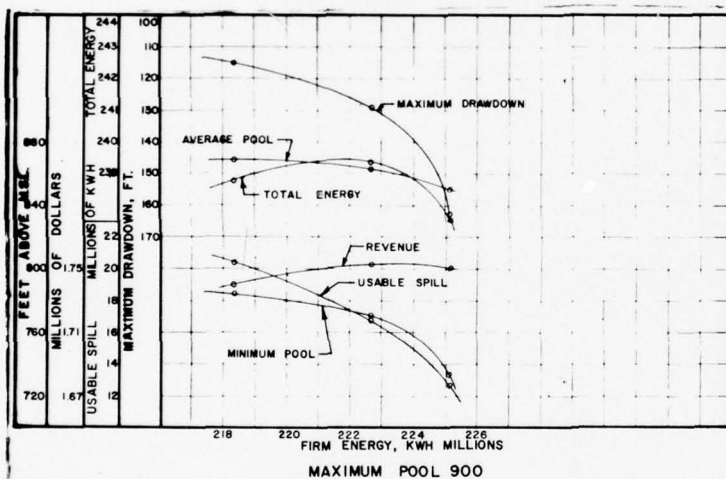
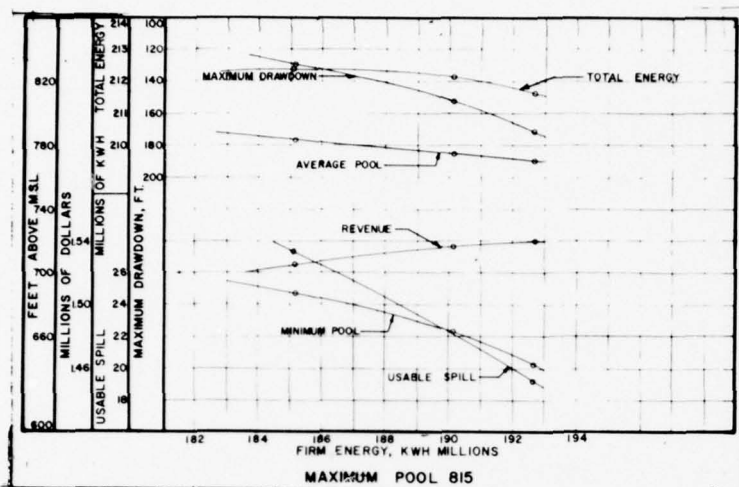


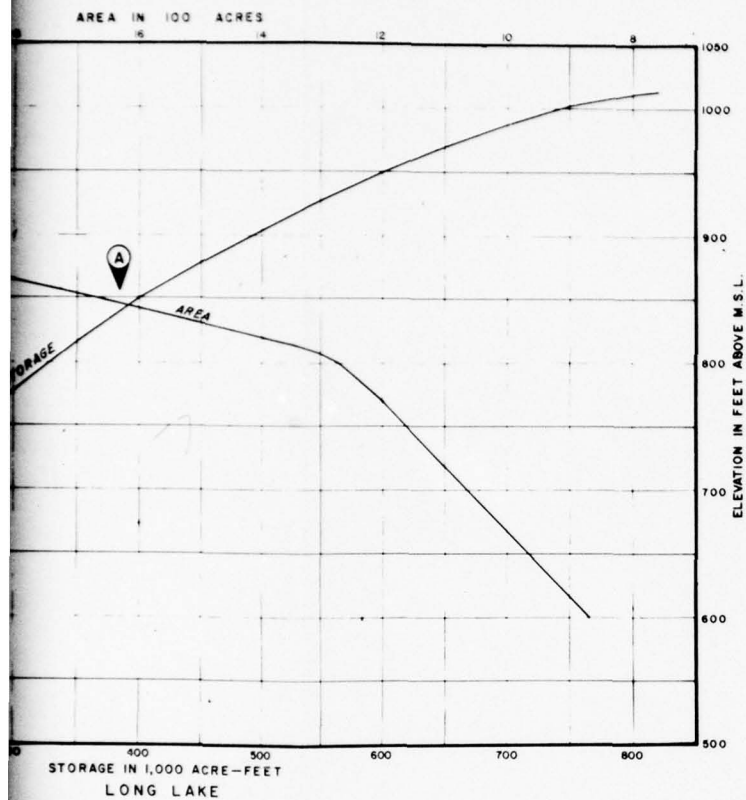
EXPECTED PERFORMANCE
LONG LAKE FRANCIS TURBINES
32,000 HP - 706' NET HEAD - 514 RPM

U. S. ARMY

NOTE:
Curves were furnished by Hydro Electric Design Branch, North Pacific Division.

DESIGNED:		SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT EXPECTED UNIT PERFORMANCE CURVES	
DRAWN:			
TRACED:			
CHECKED:			
PREPARED:			
SUPERVISED:		APPROVED:	
SUBMITTED:		SCALE NONE DATE	
RECOMMENDED:		FILE NUMBER	
		0-4-5-113	
		SHEET 1 OF 1	





NOTES:

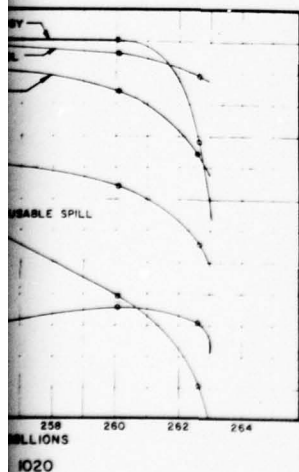
1. The AREA-STORAGE curves are taken from House Document No. 40, 87th Congress, 1st Session. Updated with recent survey data.

The USABLE SPILL (secondary energy) indicates energy that can be produced from water beyond storage capacity but not beyond machine capacity.

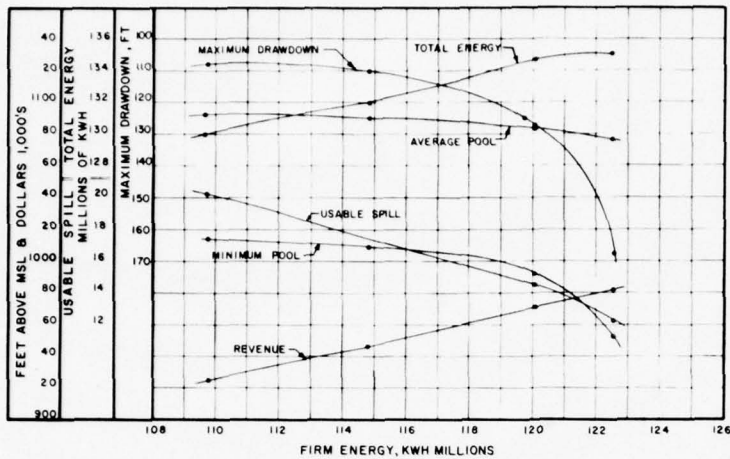
The REVENUE curve is based on a mill rate of 7.5 M/KWH for primary and 5 M/KWH (from project document) for secondary energy and are to be used for comparison purposes only.

The TOTAL ENERGY curve is composite of total energy demand imposed on the project plus secondary energy.

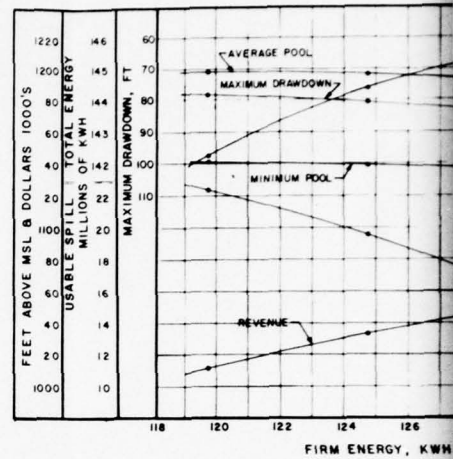
MAXIMUM DRAWDOWN is shown in feet and minimum pool is shown as an elevation in feet.



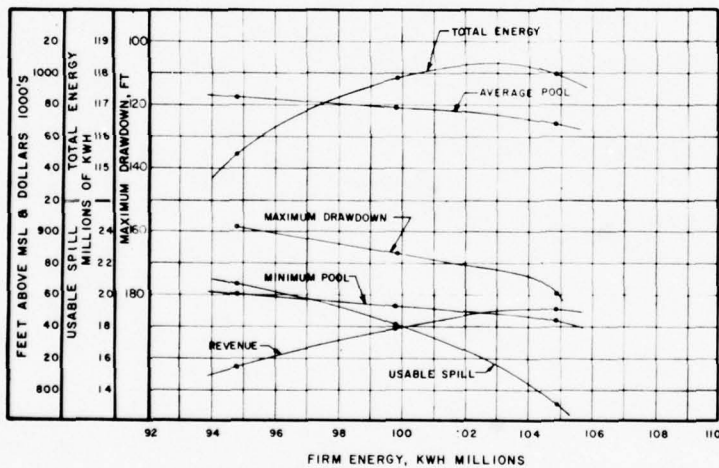
A July 1965 Area-Storage curves updated		DESCRIPTION	
U. S. ARMY ENGINEER DISTRICT ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA			
DESIGNED:	SNETTISHAM PROJECT, ALASKA		
DRAWN:	PRELIMINARY STUDIES		
RECEIVED:	LONG LAKE		
CHECKED:	AREA-STORAGE & SUMMARY CURVES		
PREPARED:			
SUPERVISED:	APPROVED:		
CHIEF, POWER INVESTIGATIONS SECTION	CHIEF, DISTRICT ENGINEER		
SUBMITTED:	SCALE	DATE	31 OCT 64
CHIEF, PLANNING AND REPORTS SECTION	AS SHOWN	FILE	NUMBER
RECOMMENDED:	Q-4-5-113	SHEET	OF



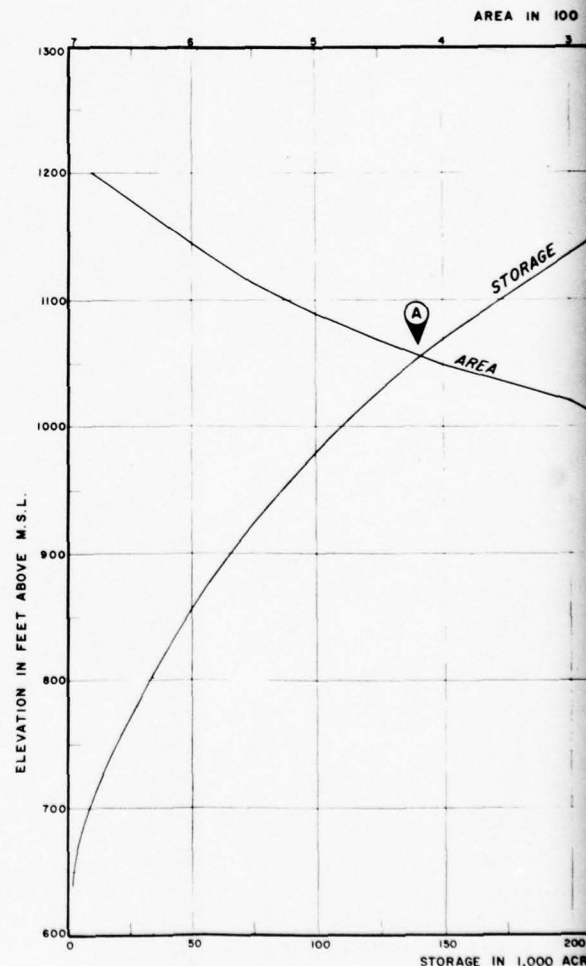
MAXIMUM POOL 1120



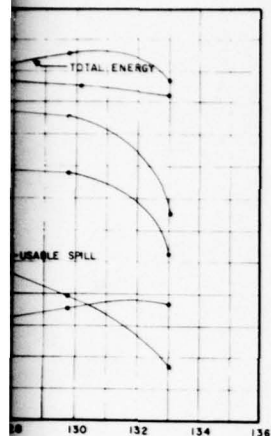
MAXIMUM POOL



MAXIMUM POOL 1020



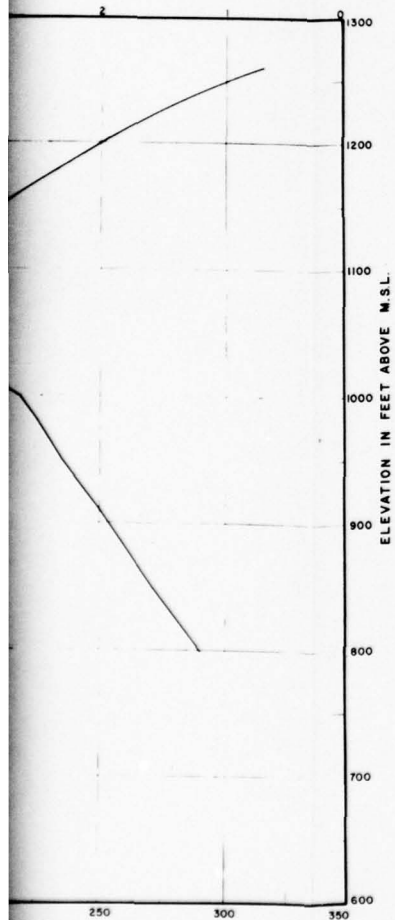
CRATER LAKE
AREA - STORAGE



ALLIONS

220

URES



FEET

VE

NOTES:

1. The AREA-STORAGE curves are taken from House Document No. 40, 87th Congress, 1st Session. Updated with recent survey data.

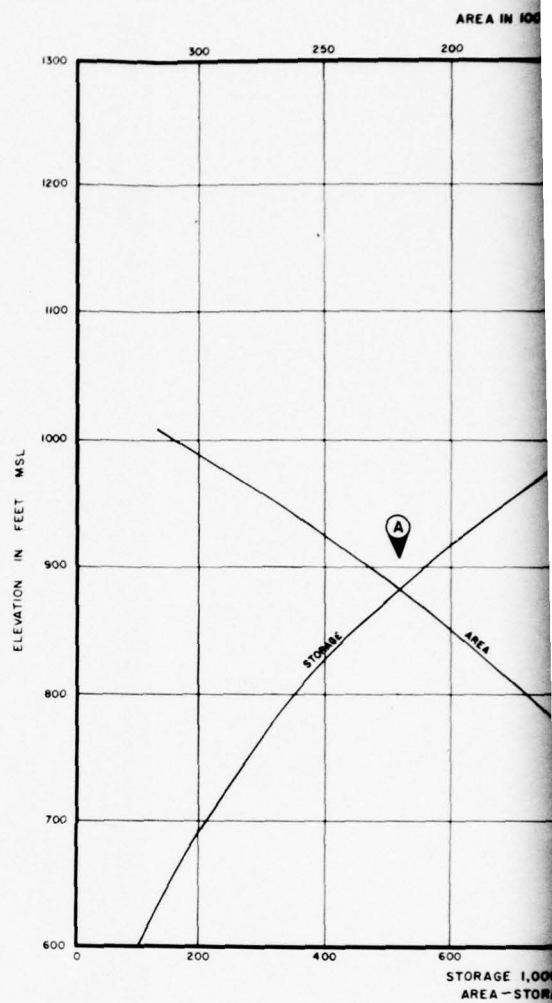
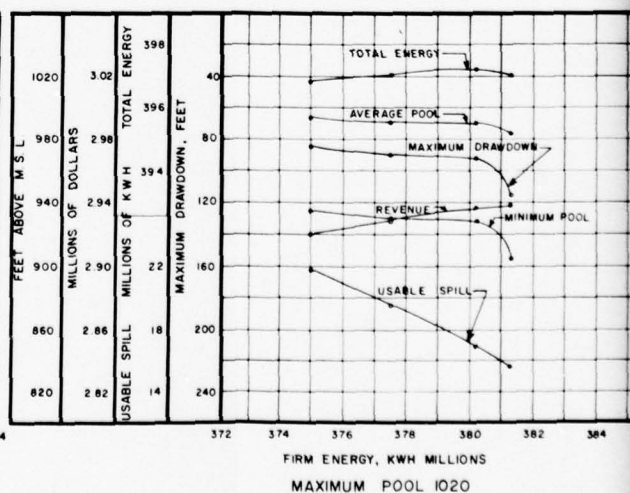
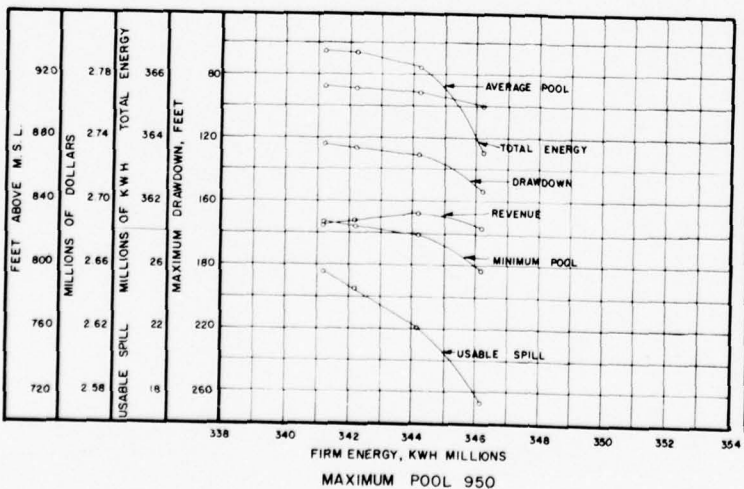
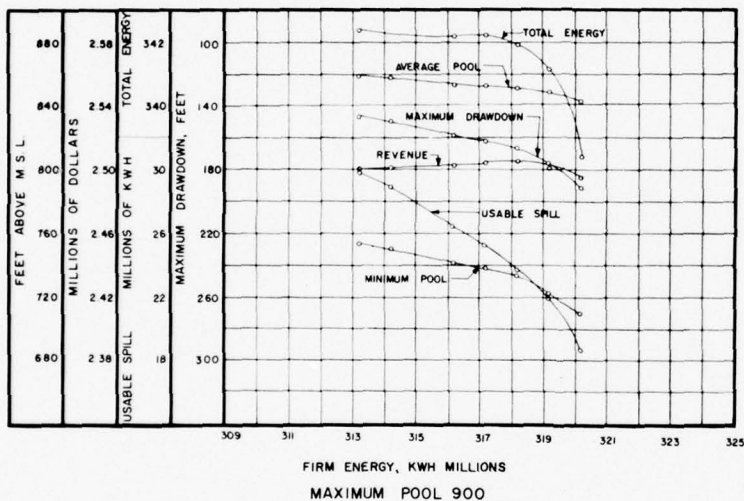
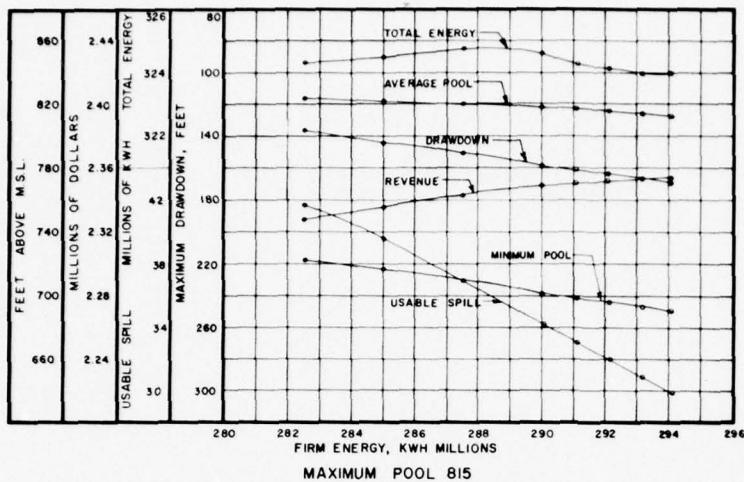
The USABLE SPILL (secondary energy) indicates energy that could be produced from water beyond storage capacity but not beyond machine capacity.

The REVENUE curve is based on a mill rate of 7.5 M/KWH for primary and 5 M/KWH (from project document) for secondary energy and are to be used for comparison purposes only.

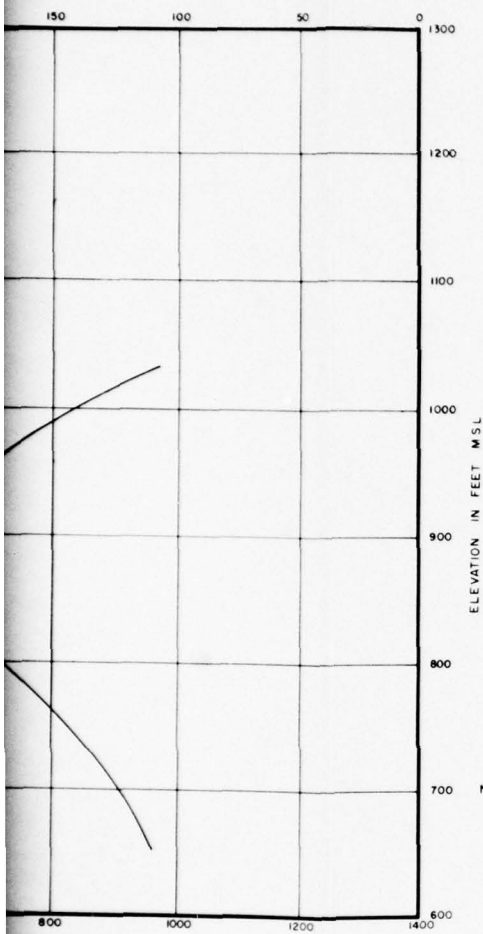
The TOTAL ENERGY curve is composite of total energy demand imposed on the project plus secondary energy.

MAXIMUM DRAWDOWN is shown in feet and minimum pool is shown as an elevation in feet.

A July 1965 Area - Storage curves updated		DESCRIPTION		BY
U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA				
SNETTISHAM PROJECT, ALASKA				
PRELIMINARY STUDIES				
CRATER LAKE				
AREA-STORAGE & SUMMARY CURVES				
DESIGNED	BY	APPROVED		
DRAWN	BY	DATE		
TRACED	BY	SCALE		
CHECKED	BY	AS SHOWN	DATE	
PREPARED	BY	FILE	NUMBER	
SUBMITTED		Q-4-5-113		
RECOMMENDED		SHEET OF		



100 ACRES

1000 ACRE - FEET
DRAINAGE CURVE

NOTES

1. The AREA-STORAGE curves are taken from House Document No. 40, 87th Congress, 1st Session. Updated with recent survey data.

The USABLE SPILL (secondary energy) indicates energy that could be produced from water beyond storage capacity but not beyond machine capacity.

The REVENUE curve is based on a mill rate of 7.5 M/KWH for primary and 5 M/KWH (from project document) for secondary energy and are to be used for comparison purposes only.

The TOTAL ENERGY curve is composite of total energy demand imposed on the project plus secondary energy.

MAXIMUM DRAWDOWN is shown in feet and minimum pool is shown as an elevation in feet.

A July 1965 Area-Storage curves updated.

DESCRIPTION

U. S. ARMY ENGINEER DISTRICT ALASKA
CORPS OF ENGINEERS
ANCHORAGE, ALASKA

DESIGNS

DRAWN

TRACED

CHECKED

PREPARED

SUPERVISED

SUBMITTED

RECOMMENDED

APPROVED

SNETTISHAM PROJECT, ALASKA

PRELIMINARY STUDIES

COMBINED LAKES

AREA-STORAGE & SUMMARY CURVES

APPROVED

SCALE AS SHOWN

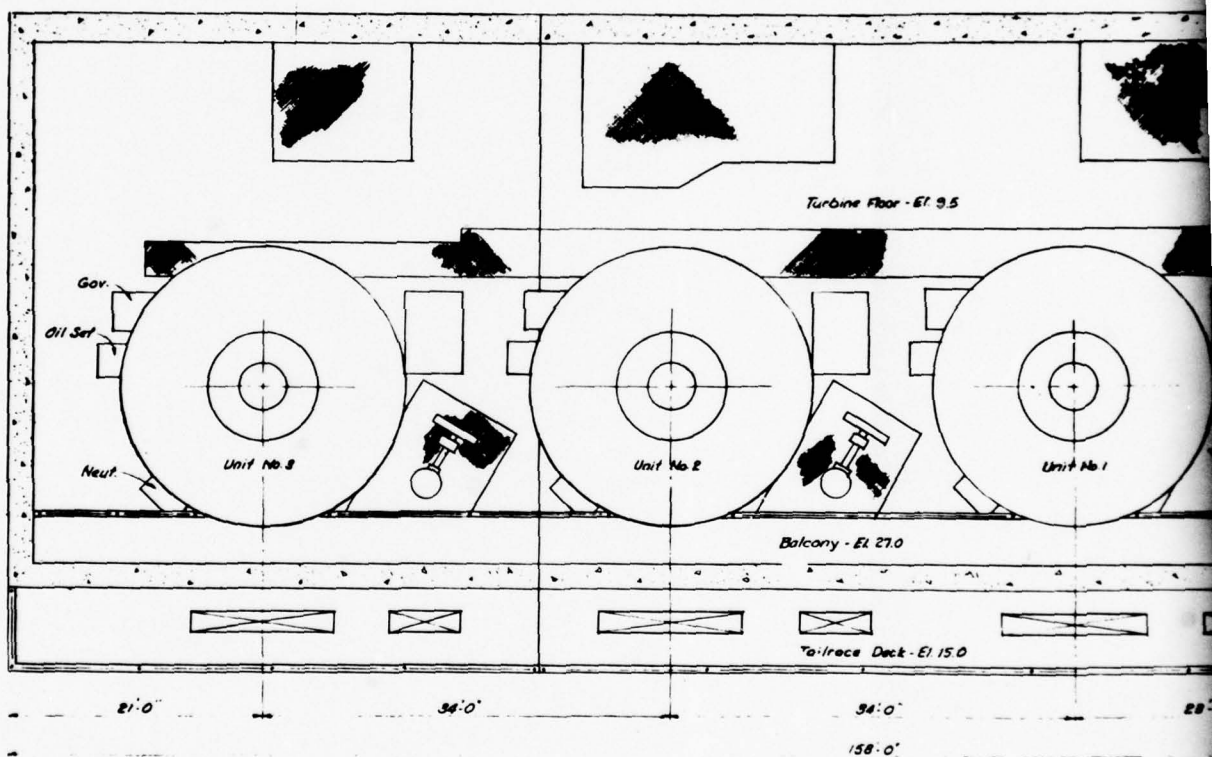
DATE 31 OCT 64

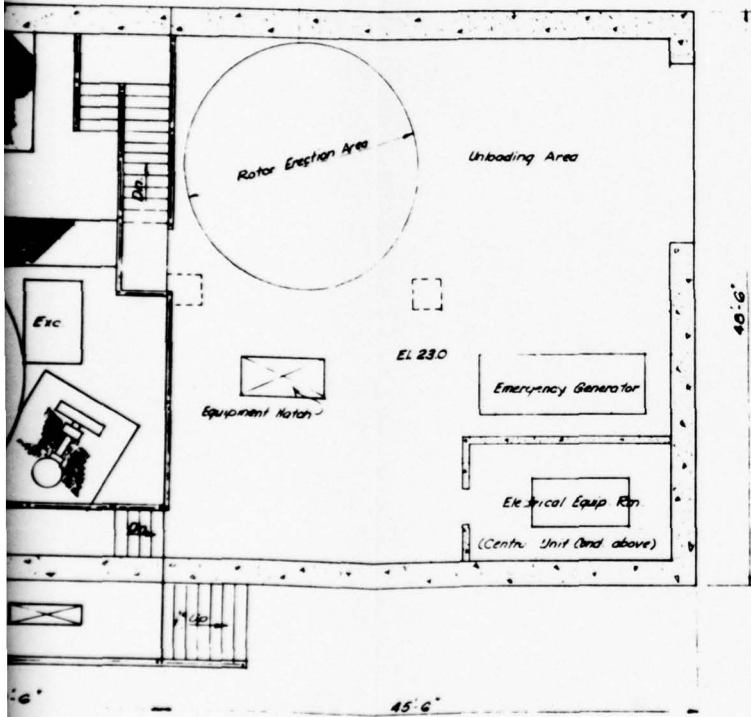
FILE NUMBER

Q-4-5-113

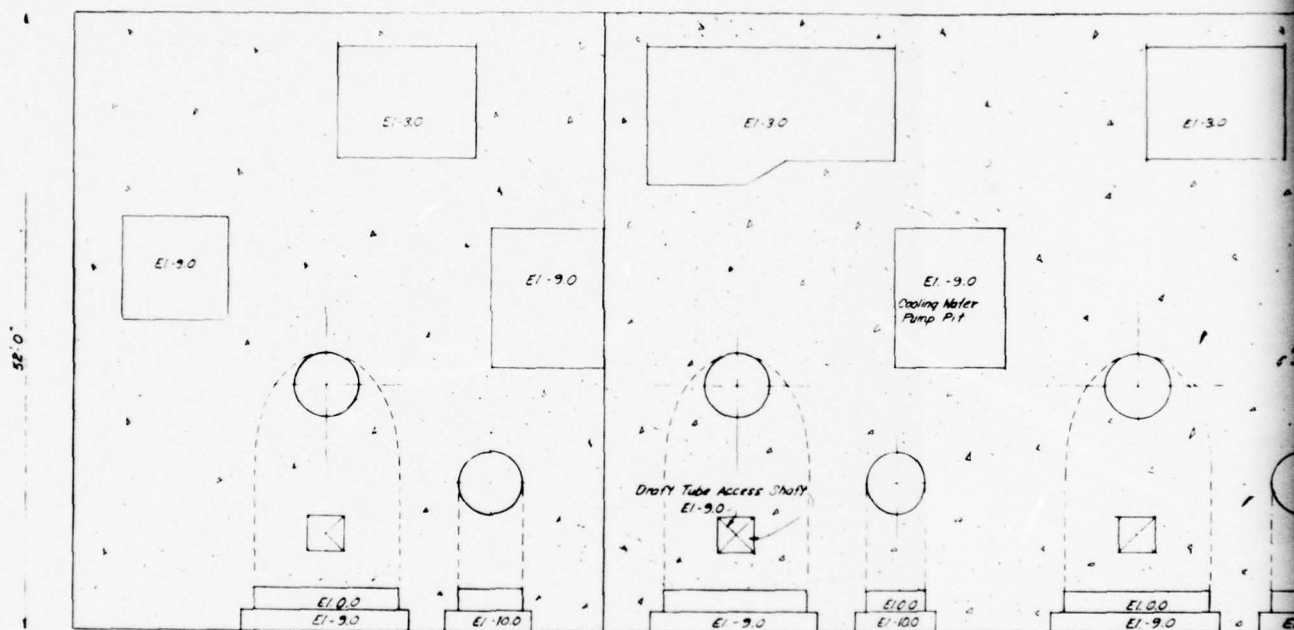
SHEET

OF



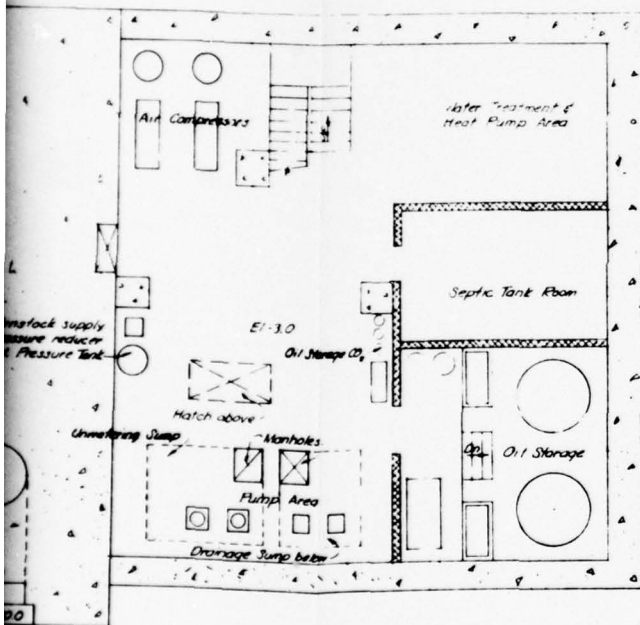


REVISION	DATE	DESCRIPTION	BY
1	10/2/68	Redrawn	
CORPS OF ENGINEERS, U. S. ARMY NORTH PACIFIC DIVISION, PORTLAND, OREGON			
DESIGNED BY <i>Kell</i> DRAWN BY <i>Kell</i> CHECKED BY <i>[Signature]</i>		SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT POWERHOUSE PLAN - EL. 23.0	
SUBMITTED <i>[Signature]</i> DATE <i>[Signature]</i>		APPROVED FOR <i>[Signature]</i> DATE 1 Oct. 1968 SCALE 1"=1'-0"	
		SNP-0-0-0/2	

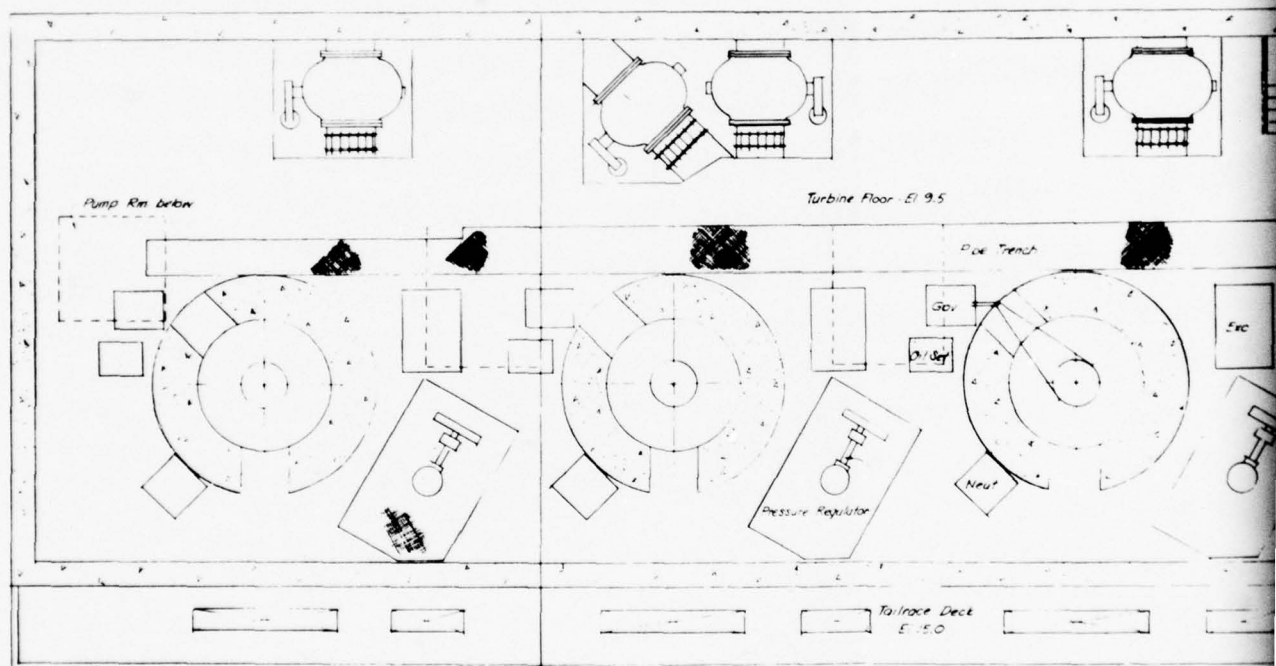


11-2-58

U. S. ARMY



REVISION	DATE	DESCRIPTION	BY
CORPS OF ENGINEERS, U. S. ARMY NORTH PACIFIC DIVISION, PORTLAND, OREGON			
DESIGNED BY <i>Kell</i>	SNETTISHAM PROJECT, ALASKA		
DRAWN BY <i>Kell</i>	FIRST STAGE DEVELOPMENT		
CHECKED BY	POWERHOUSE		
APPROVED BY <i>[Signature]</i>	PLAN - EL-3.0		
DATE <i>[Signature]</i>	APPROVED BY <i>[Signature]</i>	DATE <i>[Signature]</i>	SCALE <i>[Signature]</i>
SNP 0-0-0/N		SNP 0-0-0/N	

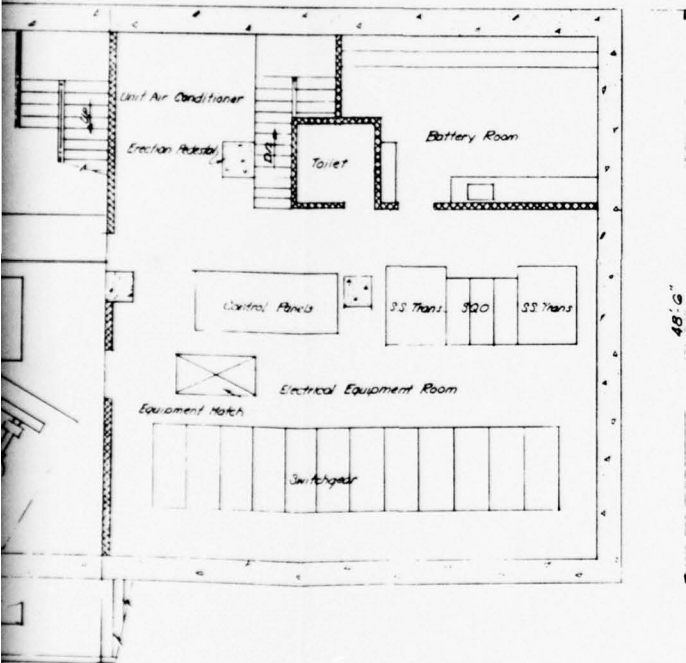


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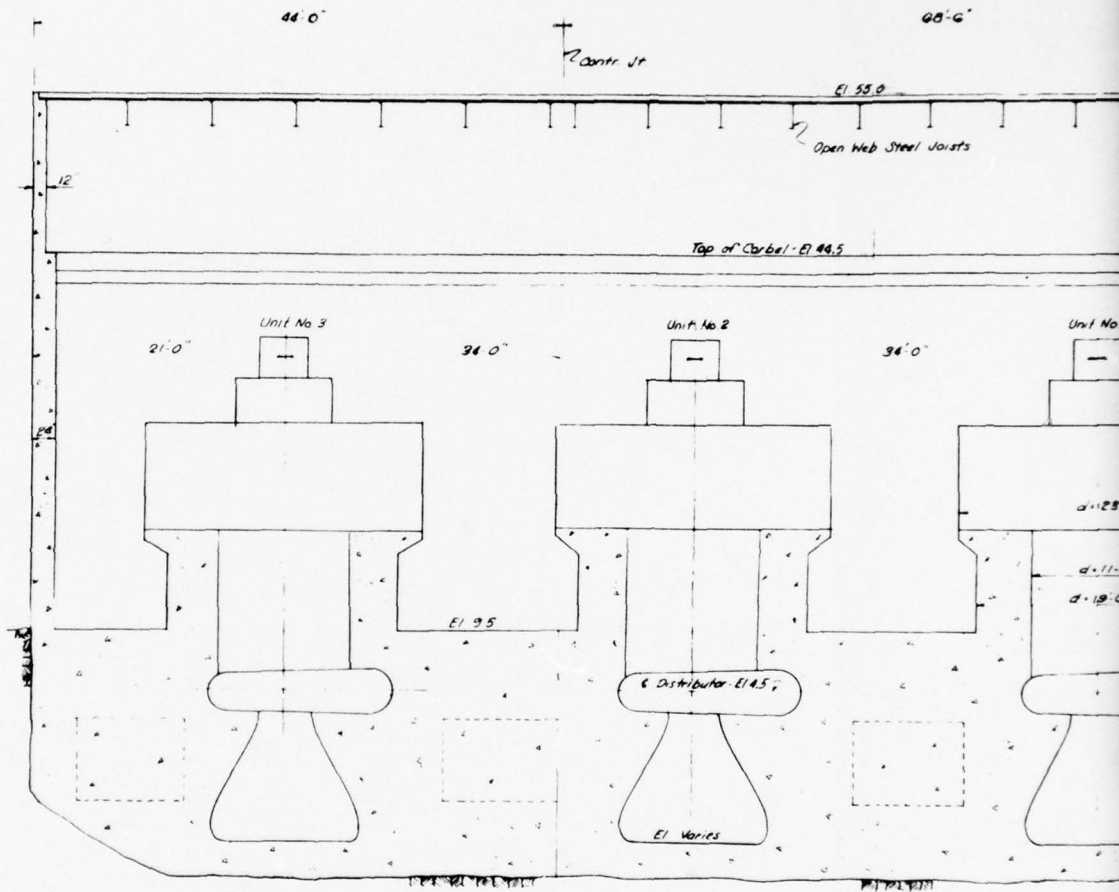
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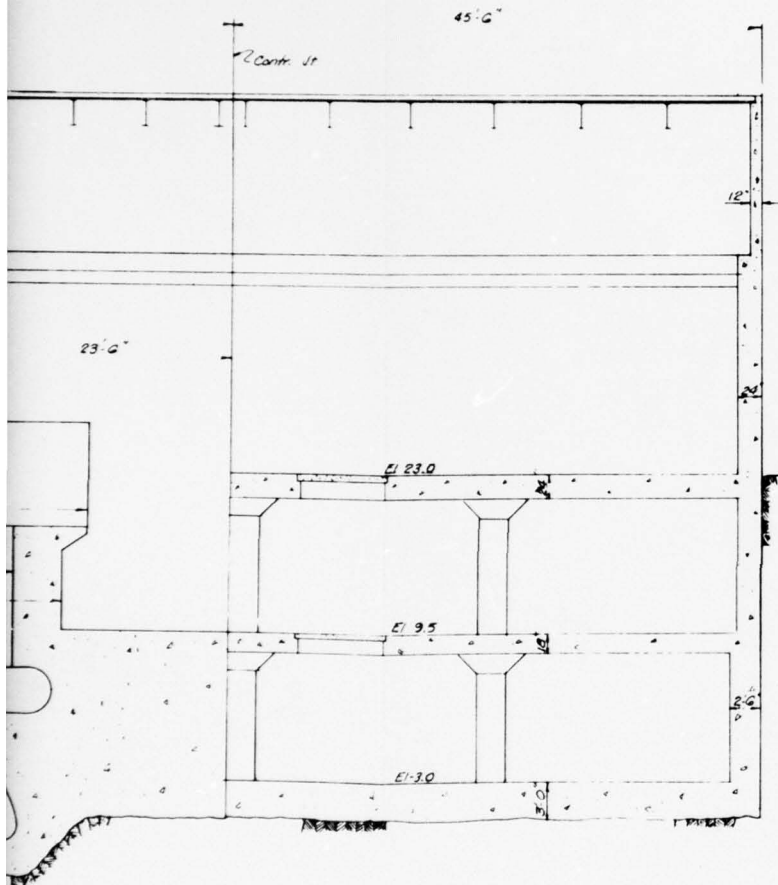
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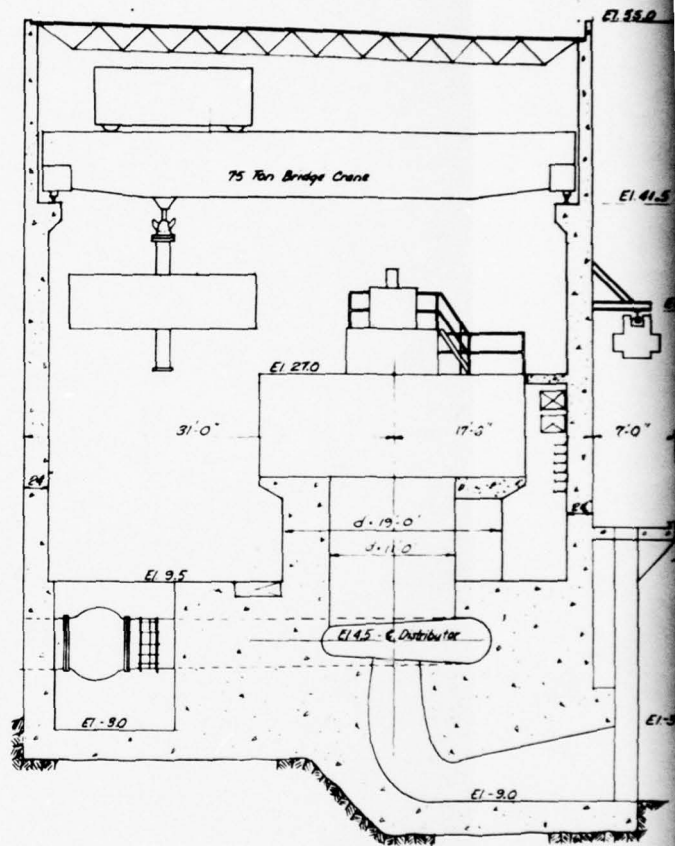


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DRAWN BY <i>K/L</i>	FIRST STAGE DEVELOPMENT	
CHECKED BY	POWERHOUSE	
PREPARED BY <i>E.B. Spear</i>	PLAN - EL. 9.5	
SUBMITTED <i>Robert H. Smith</i>	APPROVED FOR BY <i>Robert H. Smith</i>	DATE 1 Oct. '65
	SCALE 1/8" = 1'-0"	SPEC. NO.
	SHEET SNP 0-0-0/3	

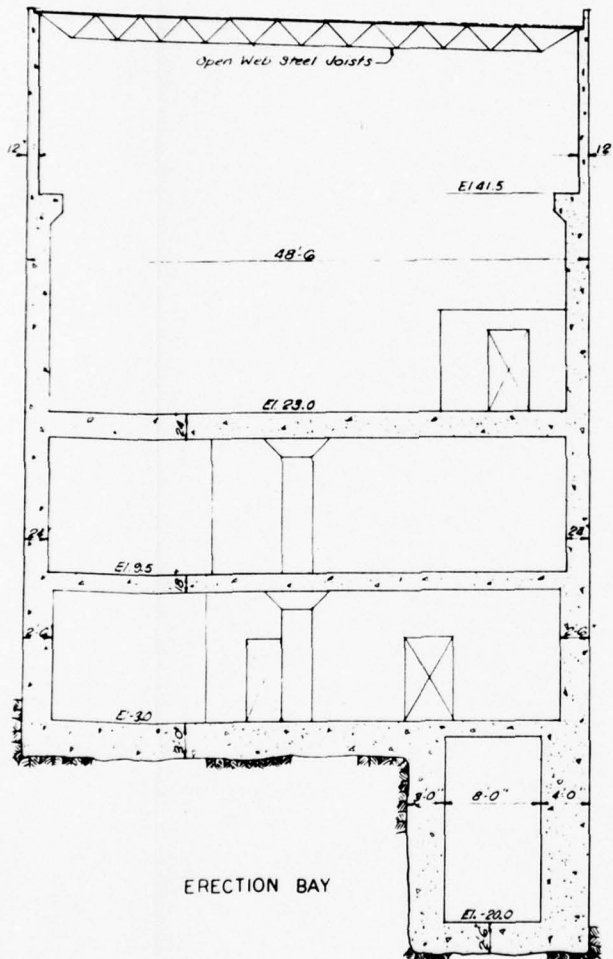




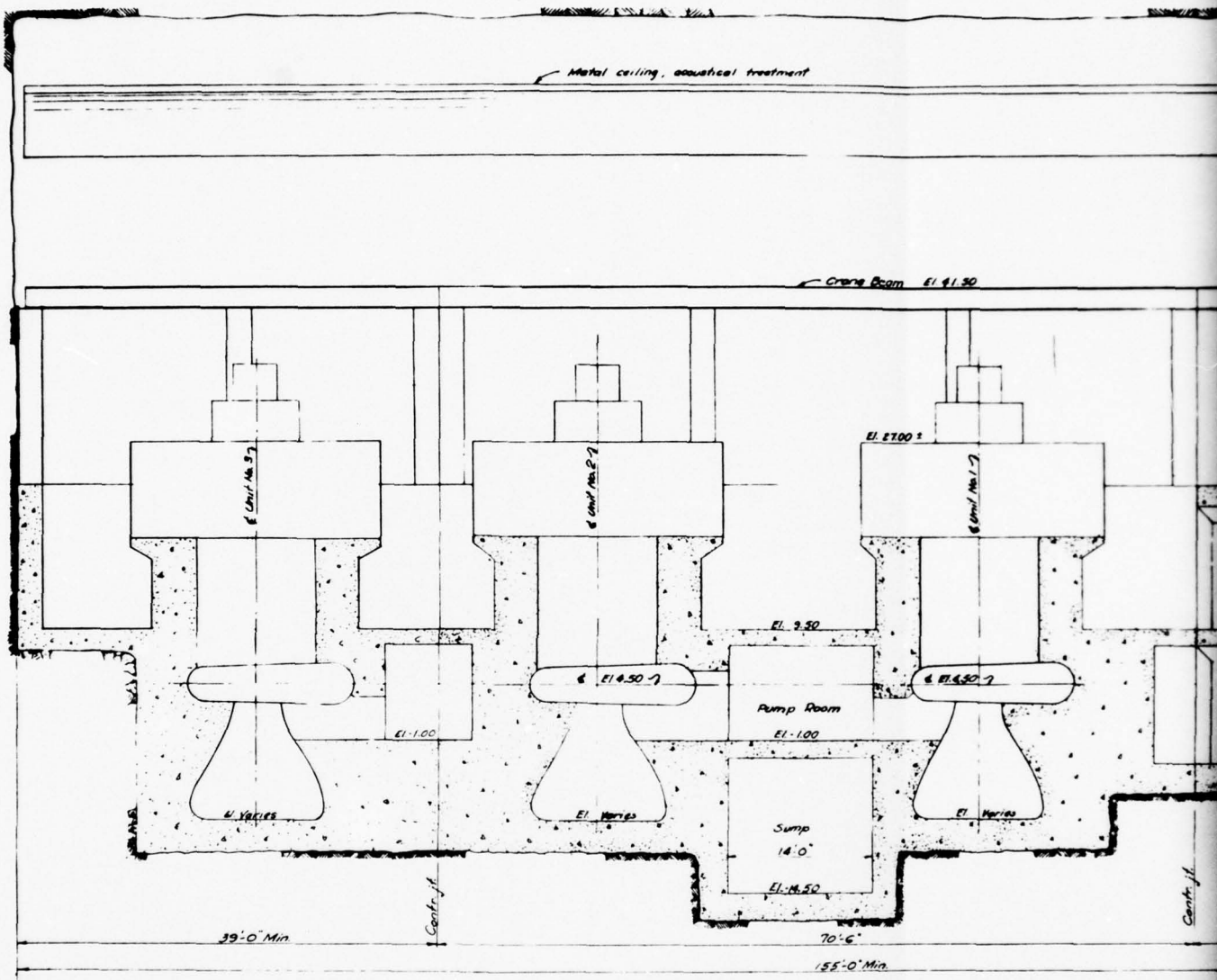
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DRAWN BY	KJL		
CHECKED BY	[Signature] [Title]		
SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT POWERHOUSE LONGIT. SECTION THRU 6 UNITS			
SUBMITTED	[Signature] [Title]		
APPROVED FOR BY	[Signature] [Title]		
DATE	1 Oct 1968		
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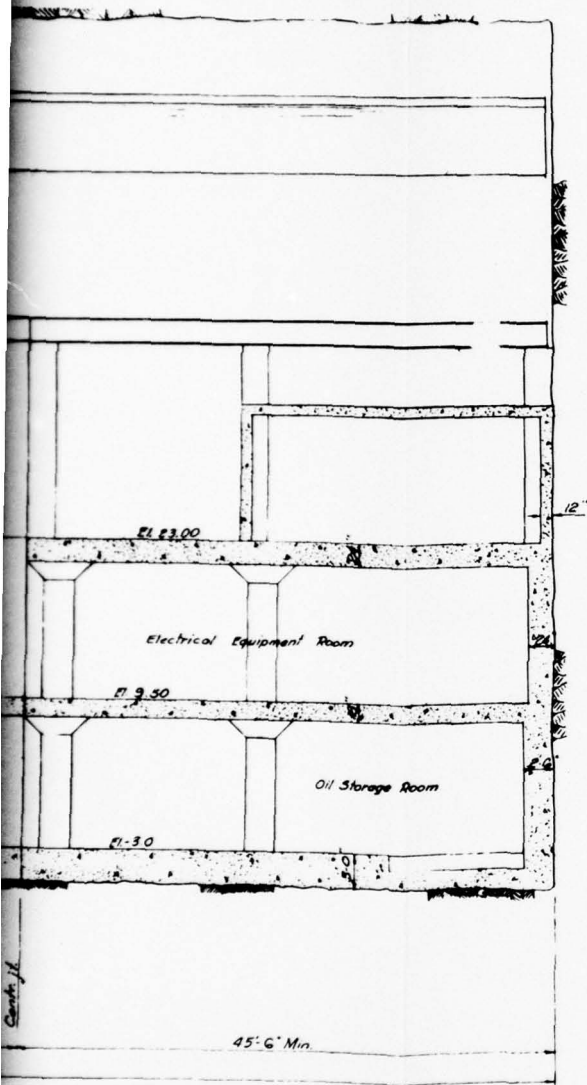


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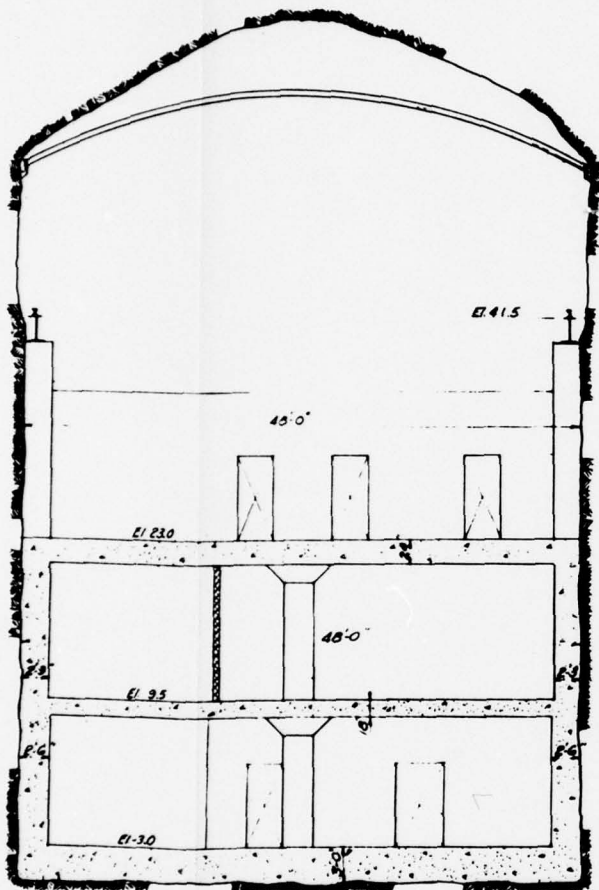


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<p>SUBMITTED <i>[Signature]</i> SNIP OFFICE <i>[Signature]</i></p>		<p>APPROVED FOR BY <i>[Signature]</i> DATE 1 Oct 68 SCALE 1/2" = 1'-0" SPEC. NO.</p>	
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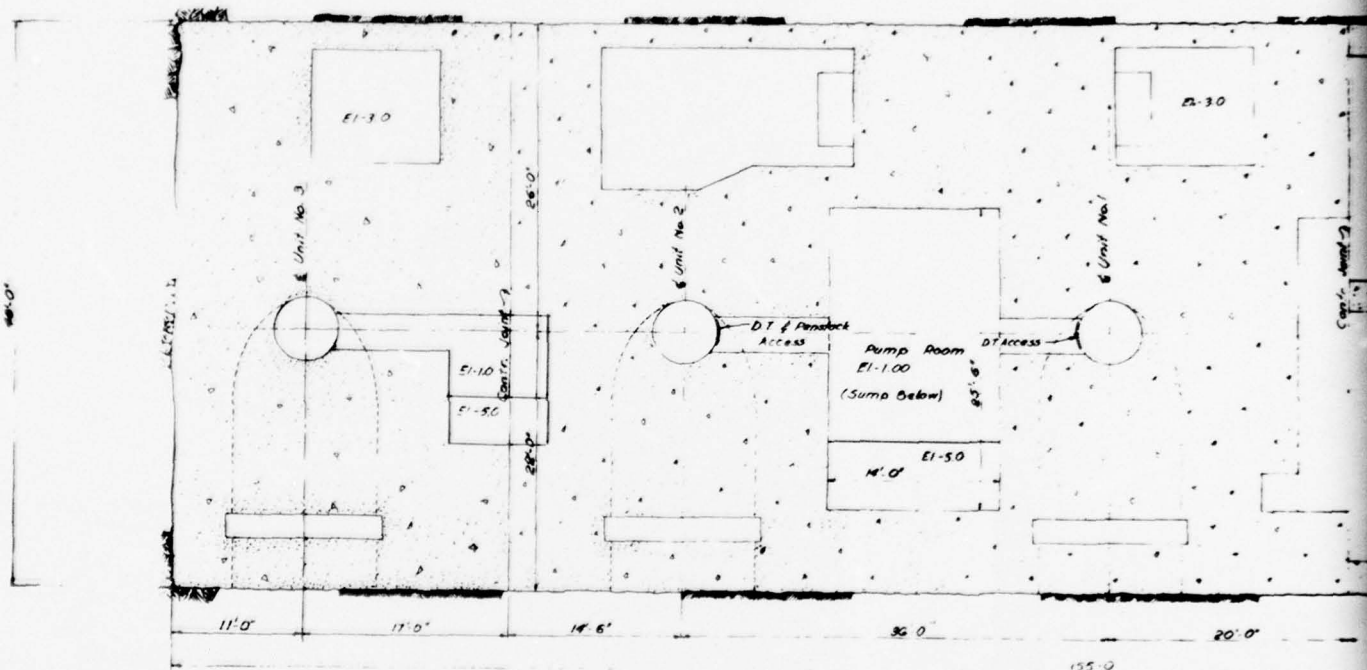


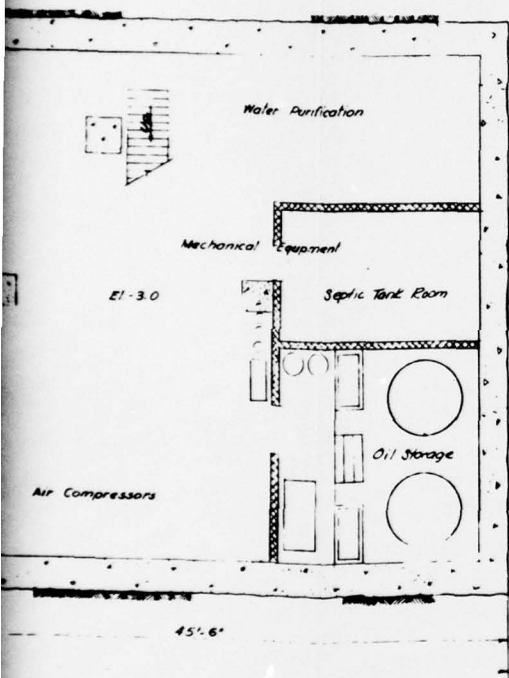
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DRAWN BY <i>V.S.</i>		<p align="center">POWERHOUSE UNDERGROUND LONGIT. SECTION THRU 6 UNITS</p>	
CHECKED BY <i>[Signature]</i>		<p align="center"><i>[Signature]</i> SCALE: 3/8" = 1'-0" (SHEET 28)</p>	
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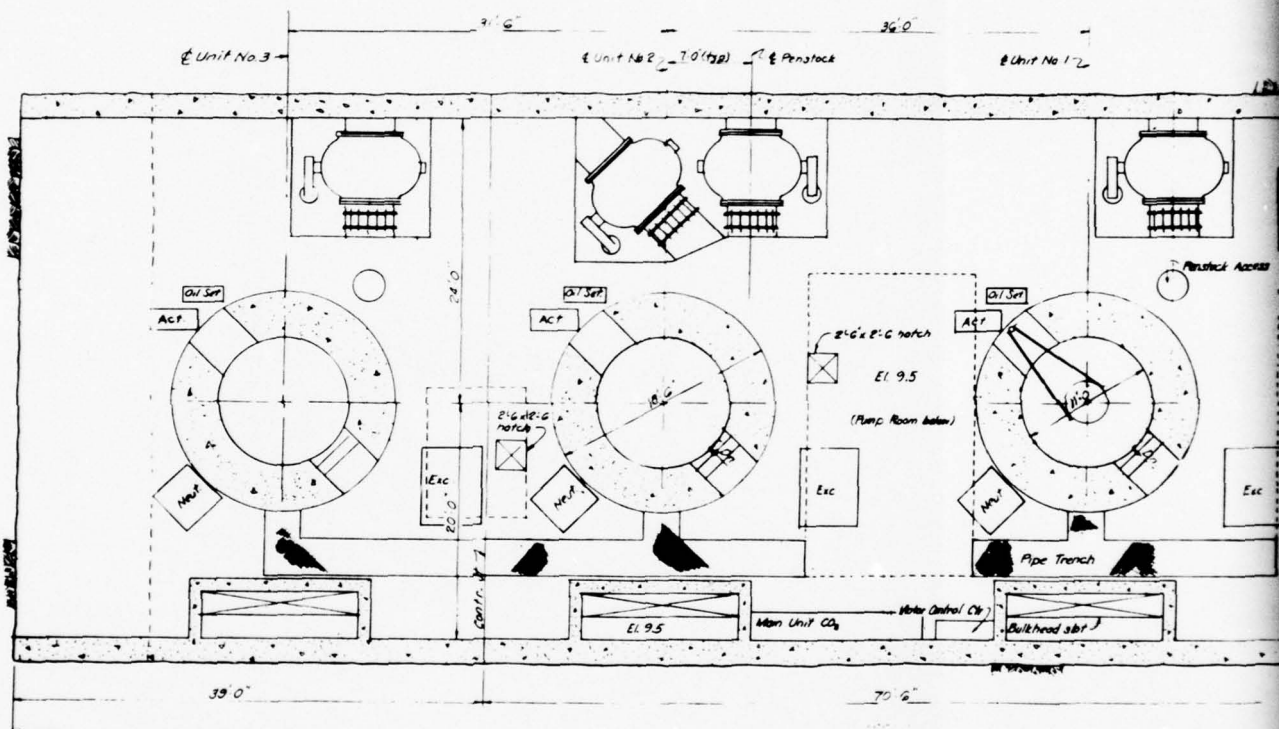
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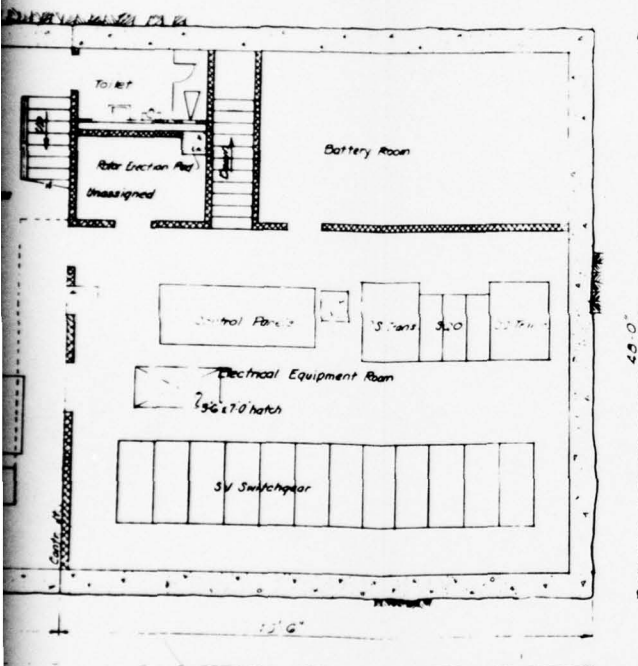
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REVISION	DATE
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CHECKED BY: <i>[Signature]</i>	
APPROVED BY: <i>[Signature]</i>	
DATE: <i>10/2/51</i>	
SNP 0-0-0/9	



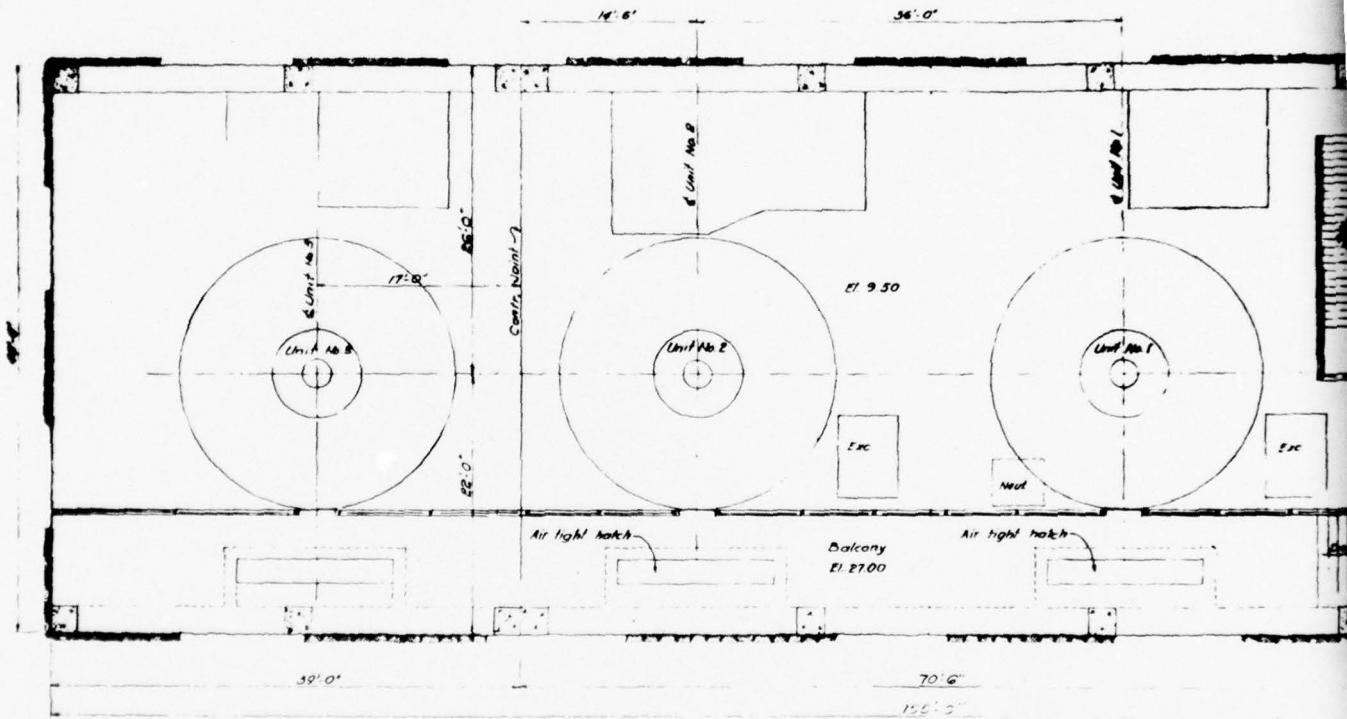


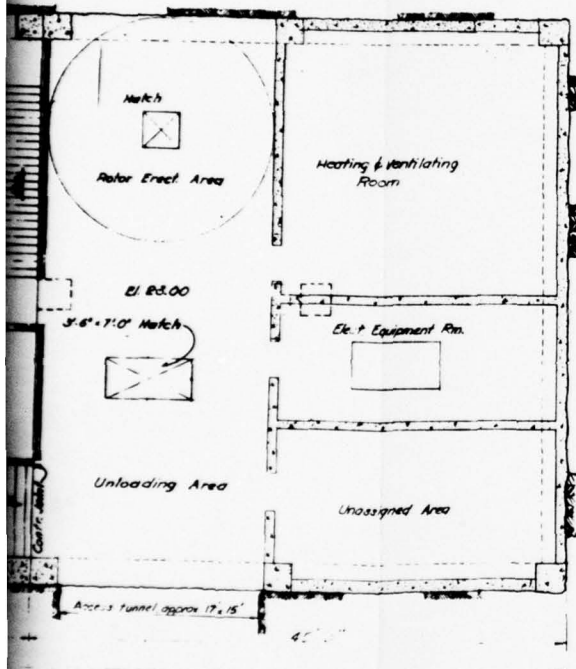
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DESIGNED BY <i>K.L.L.</i>	SNETTISHAM PROJECT ALASKA	
DRAWN BY <i>V.S.</i>	POWERHOUSE UNDERGROUND	
CHECKED BY <i>T.H. Jones</i>	PLAN - EL-3.0	
APPROVED BY <i>[Signature]</i>	APPROVED FOR THE DISTRICT <i>[Signature]</i>	DATE <i>10/2/88</i>
SCALE: AS SHOWN ON DRAWING	DETAILS: 1/8" = 1'-0"	SNP 0-0-0/8





REVISION	DATE	DESCRIPTION	BY
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CORPS OF ENGINEERS, U. S. ARMY NORTH PACIFIC DIVISION, PORTLAND, OREGON			
DESIGNED BY: KUL			
DRAWN BY: KUL			
CHECKED BY:			
PROJECTED BY: <i>[Signature]</i>			
APPROVED FOR BY: CHIEF ENGINEER DATE: <i>[Signature]</i> 10/1/65			
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REVISION	DATE	DESCRIPTION	BY
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<p>CORPS OF ENGINEERS, U. S. ARMY NORTH PACIFIC DIVISION, PORTLAND, OREGON</p>			
<p>DESIGNED BY: <i>K.H.</i> CHECKED BY: <i>K.S.</i> APPROVED BY: <i>[Signature]</i></p>		<p>SNETTISHAM PROJECT ALASKA POWERHOUSE UNDERGROUND PLAN - EL. 23.0</p>	
<p>APPROVED FOR CONSTRUCTION: <i>[Signature]</i> DATE: 25 July 1968</p>		<p>SCALE: 3/8" = 1'-0" (SHEET 1 OF 1) SNP 0-0-0/8</p>	

APPENDIX B

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CORPS OF ENGINEERS ANCHORAGE AK ALASKA DISTRICT
SNETTISHAM PROJECT ALASKA. FIRST STAGE DEVELOPMENT. DESIGN MEMO--ETC(U)
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SNETTISHAM PROJECT, ALASKA

DESIGN MEMORANDUM NO. 7

Appendix B - Transmission

PERTINENT DATA

TRANSMISSION SYSTEM

LOCATION:

Juneau substation, 2 miles southwest of Juneau, along Gastineau Channel, across Taku Inlet, down Stephens Passage, along Speel Arm, to the Snettisham Switchyard, near the mouth of Speel River.

AUTHORIZED:

Flood Control Act 1962, providing for design and construction by the Corps of Engineers and for operation and maintenance by the Bureau of Reclamation.

PLAN:

Construct a switchyard, with transformers, at Snettisham to raise the generator voltage from 13.8kv to 138kv. Transmit a maximum of 77 MVA along 44.6 miles of overhead conductor and through 2.7 miles of submarine cable to Juneau substation. Construct Juneau substation for transforming the power to 69 and 23kv.

PROJECT FEATURES

PERTINENT DATA

POWER:

TRANSMISSION SYSTEM

Voltage	138kv
Energy transmitted	331,000,000KWH/yr
Power, maximum	69MW

LOCATION:

CAPACITY:

Juneau substation capacity	80MVA
Snethisham switchyard capacity	78MVA
Submarine cable capacity	113MVA

AUTHORIZED:

CONDUCTOR:

Size, conductor	795MCM ACSR
Phase spacing, wood structures	14.5 ft
Phase spacing, steel structures	22 ft
Ground clearance	Per NESC

RECLAMATION.

PLAN:

SUBMARINE CABLE:

Size, conductor	250MCM
Insulation	Paper
Type	Oil-filled
Sheath	Lead
Armor	Steel
Length, each cable	14,250 ft
Number, cables	4

STRUCTURES:

Wood	39.3 miles
Steel	5.3 miles

**SNETTISHAM DESIGN MEMORANDUM NO. 7
GENERAL DESIGN MEMORANDUM**

Appendix B - Transmission

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SECTION 1

GENERAL

SNETTISHAM DESIGN MEMORANDUM NO. 7
GENERAL DESIGN MEMORANDUM

APPENDIX B - TRANSMISSION

SECTION 1 - GENERAL

1-01 SCOPE:

In this appendix to Design Memorandum No. 7, major features of the transmission line and terminal facilities are determined. Design Memorandum No. 9, Transmission Facilities (Dec 66) will include line profiles, pole and tower selection and spotting, submarine cable location and terminal design. Procedures will be based upon selection of most desirable design features while clearly depicting governing parameters to facilitate free use of value engineering even into construction stage.

1-02 BASIC DATA AND CRITERIA:

- a. EM 1110-2-1801, Geological Investigations
- b. EM 1110-2-1002, Maps and Drawings
- c. EM 1110-2-1150, Definite Project Studies
- d. ER 1110-2-100, Periodic Inspection and Continuing Evaluation
- e. Bonneville Power Administration Design Standards

Basic data and criteria for transmission line and substation was received from USBR. Additional data on salt water insulator contamination was obtained from BPA. Submarine cable data was obtained from Puget Sound Power and Light and BPA as well as from manufacturers.

1-03 INVESTIGATIONS:

Field investigations leading to House Document No. 40, 87th Congress, 1st Session, included preliminary surveying on transmission line route. In 1964 the area from Snettisham to Juneau was mapped utilizing high level

aerial photography for preliminary route location. In July 1965 the line route was located and marked from Snettisham to Thane. During summer of 1965 loggers cleared turn points on transmission line route from Snettisham to Taku Inlet. The balance of the clearing and a profile survey along the route will be accomplished in 1966.

1-04 LOCAL COOPERATION AND VIEWS:

The project is not dependent upon contributions by local interests; however, citizens and local groups are actively supporting accomplishment of project construction. The City of Juneau has taken steps to make available to the Government the former city garbage dump for Juneau substation.

1-05 CLIMATE:

a. General - The weather in the transmission line area is primarily influenced by maritime conditions that prevail all along southeastern Alaska. The area is in the path of storms crossing the Gulf of Alaska. The weather is varied by mountainous nature of the terrain and different conditions can exist within a few miles of each other along the transmission route.

b. Winds - Wind along the transmission line can vary considerably due to the topographic features. The maximum recorded one minute velocity from 1917 to 1959 in the Juneau area is 68 miles per hour from the north and the southeast. Local residents have reported higher velocities during Taku winds; however, the location of gaging stations is such that there has been no recording of these winds. The Taku wind is a katabatic wind which is caused by the downflow of cold, dense air off of the Juneau ice cap. This wind flows down various natural drainages such as Lynn Canal and Taku Inlet to the warmer lower pressure maritime areas. There are no

recording stations along Taku Inlet Arm or Speel Inlet. Few people have experience in Speel Arm or Taku Inlet during the winter when these winds blow. After extensive checking, the best source of reliable information appeared to be the Captains of the Alaska Marine Highway Ferries which observe regular schedules along Taku Inlet and Lynn Canal. The Captains report that when Taku winds blow, katabatic winds also blow along Lynn Canal with similar force. Therefore, it is assumed that wind velocities at Point Retreat, a lighthouse on the tip of Admiralty Island, would be representative. A fifteen-year record of the one minute winds was obtained for this location and a frequency distribution curve was plotted as shown in Figure 1. The hundred-year wind became 110 mph, the 1000 year wind became 127 mph and the 10,000-year wind became 147 mph. To check the above assumptions the Alaska District expects to install a wind recording station along Taku Inlet in the fall of 1965. It is expected that over much of the line, winds will be considerably below the maximum velocity due to increased frictional resistance of the land and surrounding trees. In certain areas funneling could occur and winds could approach the maximum velocity of 147 miles per hour. Reports have indicated that downslope winds confined in narrow gullies have toppled groups of trees in their path. This indicates the difficulty in predicting the occurrence of these winds. There is topographic evidence and a few verbal reports which lead to the assumption that these winds can be expected along Speel River and Speel Arm. However, the wind is not expected to reach velocities as high as those along Taku Arm where drainage is directly off of glacier fields.

c. Icing - Icing is not expected to be a problem along much of the line. Lines in the Juneau vicinity have had no reported failures due to

icing when they are located in the vicinity of tidewater. The Alaska-Juneau Mining Company has a line which passes over Annex Pass to their Annex Creek hydro-plant. This line reaches a maximum elevation of about 3700 feet. The A-J Company has had few icing problems below 2000 feet. Above this elevation icing has been reported up to barrel size (Photos 1-4). They have gone to a 1/2 inch steel guy wire conductor and to protect the conductor run reactive current to keep losses up. Their reported loss on this section is about 500KW of their 3500KW load. National Electrical Safety Code heavy (1/2 inch ice) loading would be sufficient from Juneau to Mallard Cove. From Mallard Cove up Speel Arm to Snettisham, the conditions and the desire for line reliability should call for an increase in loading to 1 inch of radial ice. The following reasons are given in support of this view.

(1) The line rises to a higher elevation from Bride Point for several miles in the Juneau direction.

(2) Precipitation is far heavier along Speel Arm than in the Juneau area.

(3) The prevailing winds are from the Pacific Ocean. Because of the orientation of Speel Arm and its narrowness these winds probably pass over the Arm and deposit their moisture from higher altitudes in the form of snow and therefore do not exert the warming influence that is felt in the Juneau and Taku Arm areas.

(4) Speel Arm has been reported to have frozen over to Mallard Cove in the past. Because of its shape, it probably does not receive the influx of warm water which has kept Gastineau Channel and Taku Inlet free of ice and also because cold fresh water from the Speel River can stratify in this area; therefore, the sum of these observations indicates that the

line from Mallard Cove to Snettisham may be in conditions approximating a higher altitude. That is, higher precipitation and lower temperatures and should have more allowance for ice load. It is not expected that conductor heating will have a significant effect on ice buildup and has been ignored in this design. (Reference 13)

d. Coincident Ice and Wind Loads - Coincidence of maximum ice loads and wind loads is not expected to occur. The maximum katabatic winds carry little moisture as they come from the cold glacier fields. They are warmed as they pass into the transmission area. Therefore, the NESC heavy loading of 1/2 radial ice (or 1 inch radial ice, as outlined previously) plus 4 pound wind load plus the Code constant at 0°F will be sufficient for conductor sagging.

e. Temperatures - Recorded air temperatures in the Juneau area range from a low of -21°F to a high of +89°F with an average annual temperature of 40.3°F. There are no temperature records along Speel Arm. Maximum winter temperatures will probably be lower and it is possible summer temperatures may be higher along Speel Arm because of less moderation by sea water in the vicinity.

f. Sea Temperatures - Sea water temperatures at the cable crossing were obtained from the Institute of Marine Science, Douglas Marine Station, in Juneau. Bottom temperatures are from 37.4°F to 39.2°F year around. At the surface, temperatures have been recorded from 28.4°F for a winter low to a summer high of 49.7°F. These surface temperatures were obtained with relatively few measurements.

g. Isokeraunic Level - The isokeraunic level in the Juneau area is very low, less than one occurrence per year. Thunderstorms have only been

recorded in Juneau in the months of January, June, July and September. From 1949 to 1964 there were 5 reported thunderstorms for an isokeraunic level of 0.312 per year. The transmission route is located in mountainous country through a great many tall trees. Therefore, actual lightning strokes to the line or substations will be improbable. However, to assure protection for major equipment, overhead ground wires will be extended 1/2 mile from substations and cable terminals.

1-06 GEOLOGIC CONSIDERATION:

a. Overhead Section - Inasmuch as the exact spotting of individual transmission towers has not yet been accomplished in the field, it is not possible to completely evaluate all the wood and steel tower foundations at this time. From a brief reconnaissance of both transmission line routes however, no serious problems are anticipated in securing adequate foundations for these structures. Wherever possible, all steel tower footings will be placed on hard granitic bedrock which is known to outcrop at or close to the ground surface in many areas traversed by the line. The only potentially serious consideration to be carefully noted on each bedrock tower footing will be the possibility of slab-off failures along joints or natural partings in the rock, particularly in exceptionally steep and mountainous terrain. Wherever steel tower footings are by necessity of great depths to bedrock or placed on overburden materials, spread footings will be proportioned in accordance with specific soil types encountered. No special problems are anticipated with the foundations for the wood poles which will be utilized on most of the line. Some difficulties may be encountered however, where the transmission line crosses known muskeg areas.

b. Submarine Cable Section - Special investigations have been conducted and are continuing to be made on the foundation conditions connected with the submarine transmission line crossings of Taku Inlet (see Plate 8). The U. S. Geological Survey has run fathometer surveys across Taku Inlet along the proposed cable routes (see Plate 11), and has taken shallow drive samples of bottom sediments (Exhibit 1). In addition to this work USGS is presently conducting a "sparker" traverse of these same areas in an effort to differentiate between these bottom sediments and possible bedrock outcrops. It can be particularly important to determine the areas of sharp transitions from sediment to bedrock as these zones could subject the submarine cable to more concentrated shearing forces due to differential settlements.

c. Juneau Substation - The presently proposed location for the main receiving station at the Juneau end of the transmission line is on the city dump, located on the north side of Gastineau Channel approximately 2 miles from Juneau. At this time there are some reservations about utilizing this specific area as the site for the substation. The generally unconsolidated nature of such an artificial fill which has been built out from the water's edge with a random assortment of uncompacted materials, strongly detracts from its overall in-place stability. This could be an important consideration in connection with foundation adequacy for the relatively heavy transformers and circuit breakers. It can be an even more serious problem from the standpoint of a mass slide-out type of failure during a severe earthquake occurrence. In order to investigate these important features, the Alaska District plans to dig several backhoe pits to a depth of 20 feet and drill two churn drill holes with accompanying drive samples, to a depth

of at least 100 feet. The backhoe explorations with appropriate sampling will provide required information for the design of a suitable raft type foundation pad of reinforced concrete for the heaviest electrical equipment. The deep churn drill holes with drive samples will determine the depth of the fill material and will disclose the character of underlying natural materials, both of which will help provide required shear strengths and other data for a slip circle or wedge stability analysis of the fill area. The data obtained from these explorations will also be employed to predict settlement.

d. Snow Slide Area - The city dump location is also in the immediate proximity of a major snow slide area (see Plate 10). This snow slide area which borders the city dump on the immediate north has had a history of being active and troublesome during the winter months. An investigation will, therefore, also be made as to the possible effects of this slide on the proposed nearby structures.

e. Future Treatment - In this Design Memorandum, the principal treatment of all geological features connected with the transmission line facilities has been to outline the basic problems involved. Proposed solutions to these problems in the form of definite recommendations will be forthcoming in the specific Design Memorandum for transmission facilities.

SECTION 2
RECOMMENDED PLAN
OF
DEVELOPMENT

SECTION 2 - RECOMMENDED PLAN OF DEVELOPMENT

2-01 VOLTAGE:

a. The voltage recommended for the transmission line is 138kv. An economic analysis of all pertinent factors such as first cost and losses of the submarine cable, overhead line and the cost of the substations indicates that the 138kv voltage would have a present worth value \$201,000 less than the project document 115kv line and would be \$237,000 less than a 161kv line. The primary reason for this change is because the value of losses has increased since the original studies in 1959 by the U. S. Bureau of Reclamation. The energy value has increased from 6.1 mills per kwh as reported in House Document No. 40, to 9 mills. The load factor has been changed from 72 percent with 48MW installed capacity, to a load factor of 55 percent with 60.9MW installed capacity. Also, with the addition of a dam and more years of inflow record, studies show that the energy output will be 331 million kilowatts per year instead of 306 million. These factors increase the annual cost of losses.

b. Plate 12 shows a series of curves which illustrate the changes in cost of the various features with voltage changes. Also plotted is a curve of three percent load growth. This indicates that if any other plants, such as Tease Lake or Speel River (72MW and 15MW) are tied to the line, 138kv will still remain the most economical voltage, although 161kv is very little more costly.

2-02 SUBMARINE CABLE:

a. The following types of cable were considered for the crossing:

- (1) Solid
- (2) Low pressure gas-filled

(3) Low pressure oil-filled

(4) High pressure gas-filled

The Association of Edison Illuminating Company's specifications on the first two types of cables do not extend to the necessary voltages. Although these standards do not cover submarine cable, the manufacturers will be required to follow the applicable AEIC specifications. High pressure gas-filled cable is, to our knowledge, manufactured by only one British company and therefore, was not considered further. Pipe type cables were considered and cost figures were compiled. Pipe type cable could be four single pipes and conductors or two pipes with three cables per pipe. The result of the first alternative is high losses and the second, the purchase of two extra cables and an additional pipe for reliability. For pipe type cables, ACSR would be required to achieve enough strength to pull in the conductor. Material costs for pipe and conductor alone exceeded the cost for the oil-filled cable. When labor costs of laying are also added to material costs, the price became very much higher. Therefore, pipe type cables were no longer considered.

b. Low pressure oil-filled cable has had a long record of successful submarine use and has the advantage of ease of laying and recovery, low cost and the greater number of manufacturers producing it. Problems involved in splicing, internal and external pressure differentials, and laying have all been successfully solved by other power organizations.

c. Steel and aluminum armored oil-filled cables were investigated and steel armor had the lowest overall cost when the sum of the losses and first cost were considered. The 250MCM conductor is the minimum size recommended by the AEIC at 138kv and has a capacity of 113MVA which is 36MVA above the output of Long Lake and Crater Lake combined. Thus, future additions are

possible with this cable. These could include Dorothy Lake and Tease Lake. Therefore, the recommended submarine cable is a 250MCM lead sheathed, paper-insulated, steel armored, oil-filled cable.

2-03 CONSTRUCTION:

Wood pole construction is recommended for the majority of the line. It has the advantage of low cost, ease of maintenance, construction familiarity, and is relatively unobtrusive. In the steep area near the power plant, steel towers are recommended for their adaptability to steep slopes, absence of guys and ability to be airlifted in small sections.

2-04 SUBSTATION AND SWITCHYARD:

a. A switching diagram for Juneau Substation is shown on Figure 2 and its location is shown on Plates 1 and 10 and is essentially as recommended by the Bureau of Reclamation. Triple rated autotransformers with a maximum capacity of 40 MVA are shown. This capacity for each of the two transformers closely matches the capacity of the three 26 MVA transformers at the power-plant. In order to keep costs to a minimum, the second Juneau transformer is shown paralleled with the first with only isolating disconnect switches provided to allow maintenance. This decision is based on the very low probability of transformer failure. The autotransformers supply USBR standard steel construction 69kv and 23kv bays, into which the customer's distribution lines are brought. Space will be provided within the substation fence for one large building for control room, office, warehouse and garage space. Space will also be provided for outdoor storage for such items as spare poles, crossarms, large equipment and other miscellaneous items which are compatible with outside storage.

b. The switchyard at the pwerplant will be located, as shown on Plate 31 in the main report, "Powerhouse Site Plan." Initially, the switchyard will consist of two transformer bays and one line bay. To reduce costs, only disconnect switches are shown for the transformer connections to the high voltage strain bus. The switchyard will follow USBR standard steel construction.

SECTION 3
DEPARTURES
FROM
PROJECT DOCUMENT PLAN

SECTION 3 - DEPARTURES FROM PROJECT DOCUMENT PLAN

3-01 DEPARTURES FROM PROJECT DOCUMENT PLAN:

Certain changes from Project Document Plan, as modified by USBR reappraisal were found desirable, as follows:

- a. Voltage from 115kv to 138kv to reflect increased value of energy and additional generating capability.
- b. Snettisham switchyard capacity from 72 MVA to 78 MVA to match generating capability.
- c. Overhead line was rerouted to avoid higher elevations (special problem areas) with mileage changes as follows:
 - (1) Steel towers from 10.4 to 5.3 miles.
 - (2) Wood H-frame from 25.6 to 39.3 miles.
- d. Juneau substation from 66 MVA to 80 MVA to match generating capability.

SECTION 4

ROUTES

SECTION 4 - ROUTES

4-01 BASIC ROUTE AND ALTERNATE:

a. The transmission route, as outlined in House Document No. 40, 87th Congress, 1st Session, was generally followed with the exception of the route from Prospect Point, near Mallard Cove to Circle Point, near Slocum Inlet (Plates 3, 4, 5, 6 & 7) and Cherokee Flats, near Grindstone Creek to Thane, about 4.5 miles southeast of Juneau (Plates 8 & 9).

b. The recommended route in these areas follows the shoreline. The project document route was extensively investigated. A comparative cost analysis, Exhibit 2, Selected Computations, indicated that the sea level route from Prospect Point to Circle Point portion of the line was cheaper by \$355,000 PW value than the high route even though the sea level route is 19.6 miles in length compared to 11.1 miles for the high route. This was caused by the following factors:

(1) The high line was assumed to have 5.5 miles of steel structures which were designed for heavy ice and wind loadings. This 5.5 miles was mostly that portion of the line above 2000 feet in elevation. The low line was all of the wood pole type construction.

(2) The loss of power revenues is from an estimated four outage days per year on the high line and one outage day per year on the low line (Exhibit 3) USBR letter, dated 6 Nov 64. This outage figure is most likely quite conservative. A single high line outage during the winter when adverse weather conditions occur could cause a power outage for weeks. The Alaska-Juneau Mining Company with extensive experience with a high level line has unqualifiedly recommended against a high route and has results of an engineering study which indicates that an underground cable on the high

pass of their line would pay for itself in approximately two years of operation. Photographs 1 through 4 show some of the snow conditions at Annex Pass and the type of construction which has failed to withstand the snow creep and ice loadings in this area. This type of construction in wood poles is similar to that used by ALCAN (Reference 12) with tubular aluminum structures in their Kitimat to Kemano transmission line over high passes, though it predates ALCAN's construction by many years. Aerial reconnaissance during the spring indicated that deep snow forms on the route above the two thousand foot elevation, large wind cornices have been found on the northeast sides of the ridges, indicating extensive drifting snow and winds. The overall terrain indicates strong possibility of extensive conductor icing and snow creep conditions. Maintenance in this area would be impractical during much of the winter since it could only be accomplished by helicopter. Prevailing low clouds, high winds and icing could make access impossible for weeks at a time. A-J Industries has experienced periods when it has been weeks that they could not maintain their line. The section of the high line from Cherokee Flats to Thane also has similar arguments against it. Although not as high as the route from Prospect Point to Circle Point, it still reaches an elevation of close to 1900 feet above sea level. It also would be exposed to the Taku winds from the north. The only arguments for this routing is that it is about 2200 feet shorter, requires no clearing and is invisible from Gastineau Channel steamer traffic; however, since the low line is set well back from the beach and the pole heights are generally lower than the surrounding timber, it will be quite unobtrusive. A cost comparison indicates that if there were one outage day per year additional on this section of the high line, the cost would outweigh the

low line cost. Even without the additional outage figure, the costs of the lines are very close. Therefore, from a reliability standpoint the low route is considered to be the best alternative.

4-02 SUBMARINE CABLE:

The tentative submarine cable route runs from a point seven hundred feet south of the outlet of Falls Creek on the east side of the entrance to Taku Inlet to a cove about one thousand feet north of Bishop Point on the west side of the entrance to Taku Inlet. The cable will be approximately 14,250 feet in length exclusive of the shore ends. The maximum water depth is approximately 680 feet below high water (Plate 11). The major problem in route location on the cable is to avoid areas where rocks are exposed and bridging from rock to rock could take place. Tidal currents could then cause the cable to sway, abrading the armor and the cable, resulting in a fault. Various methods of exploring the bottom of Taku Inlet to determine optimum cable routing have been considered. To avoid bridging and other deleterious conditions, television cameras, probes, divers and advanced tight beam sonar have been investigated. The depth of much of the route makes all of these methods difficult. Diving can only be used at relatively shallow depths because of impaired visibility and lack of mobility. Probing is expensive and can not delineate an overall area accurately without excessive measurements. Television also is limited by visibility problems, since there is some glacial silt in the water. Tight beam sonar appears to offer greatest promise of success. Even if other methods are employed, information obtained by sonar should be invaluable. A sweep type sonar which is capable of automatically plotting a continuous cross section has been investigated. It has a beam width of two degrees and is able to discriminate

to six inches in range. However, since it has a maximum depth capability of only 480 feet, the separate transducer unit would have to be towed astern at a suitable depth to give the coverage required. Highly reflective targets, such as air filled vessels, would be used to achieve an accurate position control. Results of presently scheduled sparker survey by USGS will probably indicate that broken or exposed rock may only be a problem on the steeper slopes near the shores which areas could lie at considerable depth. Treatment of the shore ends must protect the cable from floating ice, logs, waves, boats, and vandalism. Shore ends must also be designed for maximum heat dissipation to allow the fullest capacity of the cable to be used. The cable terminal facilities need only be situated above high water, and will be designed to reduce salt spray problems on the potheads. Oil pressurization equipment will be required on each end. The most reliable pressurization is by gravity head. Route selected will have a steep rise on the Snettisham side of Taku Inlet and allow fairly close location of the oil reservoirs. On the Juneau side the banks do not rise as steeply and the oil reservoirs must be located at a greater distance from the terminal. An overhead crossing was considered for Taku Inlet but a number of factors mitigated against it, although the 12,000-foot span length is well within technical feasibility limits. The aerial span would require about 10 miles additional transmission line as it would be located up Taku Inlet at the narrowest point. This would cost about \$450,000 additional. The overhead span would be exposed to the full force of Taku winds channeled through the narrow opening and it would be a hazard to extensive light aircraft travel up and down the Inlet and they, in turn, would be a hazard to the line. Its most probable outage period would be during times of

most inclement weather and replacing it might prove infeasible for weeks. Its overall cost would be higher even though short towers would suffice because of steep bank slopes. Furthermore, the steep mountain sides leading to the terminals would expose the connecting line to slide hazards.

SECTION 5
ECONOMIC ANALYSES

SECTION 5 - ECONOMIC ANALYSES

5-01 ASSUMPTIONS:

Future Developments:

a. The load and losses on the transmission line remain constant at plant capacity of 69MW from 14th year to 50th year. This is the assumption currently favored by the U. S. Bureau of Reclamation.

b. The load grows on the transmission line at 3 percent per year from 14th to 50th year.

c. The load grows at 6 percent per year from 14th to 50th year. A second line is built in the 34th year with the Juneau load equally divided between the two lines.

d. The load grows at 6 percent per year from 14th to 50th year. A second line is built in 14th year with the Juneau load equally divided between the two lines.

e. Water will be spilled for first 13 years so losses have no value during this time.

5-02 OVERHEAD LINE SECTION:

All cost comparison studies were based upon constructing 5.3 miles of steel tower line and 39.3 miles of wood pole H-frame line. Voltages from 69kv through 230kv were studied to cover all possible feasible voltages. Costs for construction were obtained from Chugach Electric Association, Anchorage, Alaska, on their 115kv line job completed in 1962 and from the Bonneville Power Administration for steel tower lines. For purposes of estimating, a profile was made from the aerial survey contour maps, Plate 5, for ten miles of the line. Structures were then spotted and 795MCM conductor sagged in with a USBR template for standard 115kv

construction. A takeoff was made to determine the number of each size and type of structure required. These figures were then used as a basis for the entire wood pole section estimate. Fog type insulators were estimated throughout. Comparison costs are tabulated and explained in Table 1.

5-03 SUBMARINE CABLE:

a. Cable - All cost comparisons were made on copper conductor, paper-insulated, oil-filled, lead-sheathed, wire armored cable, as shown in Table 6. Prices were obtained from Phelps-Dodge Copper Products Company, Bonneville Power Administration, Puget Sound Power and Light Company and Alaska Tug and Barge Company. Prices for cables with a minimum capacity of 77 MVA were obtained, both with aluminum wire armor and steel wire armor. Since ionization in the insulation limits the minimum conductor diameter at the higher voltages, the higher voltage cables have a capacity much in excess of 77 MVA as shown in the following tabulation. The MVA capacity of the land portion of the cables as submitted by Phelps-Dodge Copper Products Corporation is as follows:

Conductor Size	115kv		138kv		161kv	
	Steel Armor	Alumin Armor	Steel Armor	Alumin Armor	Steel Armor	Alumin Armor
4/0 AWG	81	91				
250 MCM			98	115	127±	150±
350 MCM				132		
450 MCM		131				
550 MCM			128			
1000 MCM	134					

b. Terminals - Terminal equipment costs were obtained from Phelps-Dodge Copper Products Corporation. These were potheads, oil reservoirs,

valve panels and miscellaneous oil treating and installation equipment. In order to increase the shore end capacity of the cables, it is planned to operate the sheath and armor insulated from the water line to the potheads. The sheath and armor of each cable will be bonded to the other cables at the water line and the potheads insulated from ground. The length of cable from the water line to the pothead will be kept short to limit the voltage developed in the insulated section of sheath and armor. A simple shelter will be built around the potheads and takeoff structure to shelter the insulators, lightning arresters and bushings against salt spray and vandalism. The potheads would be connected to the elevated oil reservoirs by suitable piping to provide gravity pressurization of the cables.

c. Cable Laying - The cable laying cost estimates are based on utilizing one large barge, one large tug and four smaller tugs to keep very close position control. Transit men and radios would be utilized on each shore. The cables would be laid one at a time utilizing a dynamometer to keep accurate tension control, with laying speed not over 1 to 2 miles per hour. Dolphins or other suitable anchoring is contemplated to hold the barge stationary while the cable ends are taken ashore.

d. Splicing - A considerable sum was allowed in the estimate to permit splicing of cable in Seattle, probably on the barge, since the domestic manufacturers may not be able to supply the required lengths in one piece. This sum should cover the rental or fabrication of four large reels and the rereeling of the spliced cable on the drum. If the splices cannot be placed on a reel, the cable may be coiled in the barge compartments for shipment to Juneau and subsequent laying.

5-04 SUBSTATION AND SWITCHYARD:

Comparisons were made at voltages of 69 through 230kv. A one-line switching diagram for line voltages from 115 through 230kv is shown in Figure 2, except that a three-winding transformer would have to be used for 161 and 230kv. Substation costs were obtained for USBR Anchorage Substation, from estimating section of Chief Engineer's office of the Bureau in Denver and from current manufacturer's catalogs. Comparative costs are tabulated and explained in Table 2.

5-05 SUMMARY OF ANALYSES:

The cheapest overall combination of overhead line, submarine cable, Juneau Substation and Snettisham switchyard, including the future additions to the substation and switchyard for each voltage is as follows. These figures are not construction costs but include items such as losses, O & M expenses, interim replacements, etc. Line losses are based on Assumption A: (4-01)

115kv

Overhead conductor, 954 MCM -----	\$3,585,000
Submarine cable, 4/0 AWG, steel armor -----	1,094,000
Terminals -----	<u>1,315,000</u>
TOTAL -----	\$5,994,000
Annual Cost -----	238,500

138kv

Overhead conductor, 795 MCM -----	\$3,447,000
Submarine cable, 250 MCM, steel armor -----	1,012,000
Terminals -----	<u>1,334,000</u>
TOTAL -----	\$5,793,000
Annual Cost -----	230,500

161kv

Overhead conductor, 556.5 MCM -----	\$3,500,000
Submarine cable, 250 MCM, steel armor -----	1,050,000
Terminals -----	<u>1,480,000</u>
TOTAL -----	\$6,030,000
Annual Cost -----	240,000

These figures show 138kv to be cheapest with 115kv costing \$201,000 more and 161kv \$237,000 more on a present worth basis. The annual costs come out \$8,000 and \$9,500 more per year, respectively.

SECTION 6
LINE PERFORMANCE

SECTION 6 - LINE PERFORMANCE

6-01 REGULATION:

Calculations indicate that the voltage rise from Snettisham to Juneau with one Snettisham transformer operating will be about 8 percent. This assumed 1 percent resistance and 10 percent reactance in the transformer. This is an unlikely condition and should cause no trouble if it should occur. Assuming a maximum load of 77 MVA at 0.9 PF, the voltage drop in Juneau due to transmission line regulation is approximately 6 percent. Including the three transformers at Snettisham, this total voltage drop becomes 11.6 percent. This is well within the range of transformer tap adjustments at Juneau and Snettisham and normal range of voltage adjustment on the generating units. After full load conditions are reached, normal load variations are only from half load at night to full load in the daytime. Under this condition, the voltage variation in Juneau would be about 7 to 8 percent. This variation could be handled by adjusting generator voltage on a schedule, similar to the operation at Eklutna Power Plant, near Anchorage.

6-02 STABILITY:

Stability of the system is not a factor at this time, since there are no other comparable powerplants on the system.

6-03 CORONA:

Corona loss on a 795 MCM ACSR conductor at 138kv is no problem, being under 0.1 KW per three phase mile of line.

6-04 CAPACITY:

Due to the high value of energy, the most economical conductor also has a large excess capacity. At the 113 MVA cable capacity, the transmission line losses would be 4.2 percent.

6-05 INSULATION COORDINATION:

a. The protection afforded to equipment windings by 120kv station type lightning arresters is as follows:

Maximum 60 cycle sparkover ----- 220kv

Maximum impulse sparkover ----- 390kv

Maximum volts with 10,000 A discharge --- 350kv

b. The 138kv windings on the transformers will be purchased with a 450kv basic insulation level (BIL) and the submarine cable BIL will be 655kv, based on an impulse strength of 1300 volts per mil for paper insulation. (Reference 3)

c. The flashover voltage levels of the line insulators, line clearances, equipment bushings, etc will be as follows: (Reference 1)

	KV		
	60-Cycle Dry	Flashover Wet	Impulse
Line insulators, 6-1/2 x 10", fog type*			
Suspension string of 8	540	360	865
Tension string of 10	650	440	1070
Clearance of 42" to poles at maximum swing angle	-	-	650
Equipment bushings	-	-	650
Equipment insulators	-	-	650

* Data from Locke Insulator Company catalog

d. With an isokeraunic level of approximately 0.33 in the Juneau area, the number of lightning outages per hundred miles of line per year will be very small. The high values of 60-cycle flashover on the line insulators is considered necessary because of the salt spray contamination problem along

the coast. Additional insulators will be used where contamination is considered to be more severe. Voltage swings under fault conditions will be low with the very low resistance grounds easily obtainable at both Snettisham and Juneau because of the proximity of salt water.

e. Switching surges will be no problem until high voltage breakers are purchased for both ends on the line. Restrike free breakers should be specified at that time as recommended by the Bureau of Reclamation.

SECTION 7
OVERHEAD LINE

SECTION 7 - OVERHEAD LINE

7-01 CONDUCTOR SAG AND STRESS REQUIREMENTS:

a. Conductor sag will be based on National Electrical Safety Code Heavy Loading for the section of the line from Juneau to Mallard Cove. This will be 1/2-inch of radial ice, 4 pounds per square foot of wind and the NESC constant at 0°F. The section of line from Mallard Cove to the powerplant will be designed for 1-inch of radial ice, 4 pounds per square foot of wind and the NESC constant at 0°F. The maximum conductor temperature for sagging purposes will be the usually used 120 degrees even though the ambient temperature does not exceed 84°F since the line may at some time carry additional load. Maximum conductor temperatures were calculated with sun on the line and an emissivity of the conductor of 0.5 (Reference 6). The following results were obtained for a 26/7 795MCM conductor:

TABLE OF CONDUCTOR HEATING

Peak Load MVA	Amps	(1)	(2)	Sum 1&2 °F	(3)	Sum 1&3 °F
		Temp. Rise °F	Temp. Max. °F		Temp. Max. Usual °F	
77	320	9	89	98	84	93
144	640	34	89	123	84	118

b. Wind loading on conductors and insulators was calculated by the equation $P = 0.0025V^2$, where P is pounds per square foot and V is wind velocity in miles per hour. Although a 10,000-year wind velocity of 147 miles per hour is indicated by Figure 1, "Wind Velocity Frequency," tests conducted in Europe show that the maximum wind pressure does not exceed 10 pounds per square foot on the conductor (Reference 11). Because the area is rough and covered with timber, it is expected that wind effectiveness will actually be below 10 pounds per square foot.

c. Reference 11 also states that short term gusts have no apparent effect on conductor swing angles. Reference 8 states that maximum velocities should be used for the structures as short term gusts are capable of enveloping the entire structure. Therefore, the structures will be designed for a 147 mph wind. The safety factor will be reduced because of the very low probability of this 10,000 year wind. Wood pole structure design is based on standard strengths in each class of pole. Since these are minimum pole strengths, variations in strength above this minimum result in a statistically stronger line. By selection of the larger poles for those structures which are apt to be exposed to greater stresses, the probabilities of failure are greatly reduced.

d. The maximum working tension of the conductor will be about 25 percent of ultimate or around 7,800 pounds. This allows low everyday stress and decreases the probability of failure due to aeolian vibration. Generally, vibration is not expected to be a hazard along the line because the topographic conditions required for a steady wind are not available. However, because of the low cost of damping devices, they will be used on the line. Long spans such as those near Snettisham, Mallard Cove and Limestone Inlet will receive special attention in this respect. An added advantage of lower conductor tensions is that the conductor sag will more generally conform to the steep slopes, reducing the requirements for hold-down weights and tension structures. Longer spans, such as Limestone Inlet, (1,800' approximately) may be strung at higher maximum working tensions, if required. However, the slopes are generally steep enough at the crossings to allow stringing with standard tension.

7-02 CONDUCTOR CONSTRUCTION:

The conductor will be 795 MCM, comprised of 26 strands of aluminum

SECTION 7 - OVERHEAD LINE

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TABLE OF CONDUCTOR HEATING

Peak Load MVA	Amps	(1)	(2)	(3)		
		Temp. Rise °F	Temp. Max. °F	Sum 1&2 °F	Temp. Max. Usual °F	Sum 1&3 °F
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c. Reference 11 also states that short term gusts have no apparent effect on conductor swing angles. Reference 8 states that maximum velocities should be used for the structures as short term gusts are capable of enveloping the entire structure. Therefore, the structures will be designed for a 147 mph wind. The safety factor will be reduced because of the very low probability of this 10,000 year wind. Wood pole structure design is based on standard strengths in each class of pole. Since these are minimum pole strengths, variations in strength above this minimum result in a statistically stronger line. By selection of the larger poles for those structures which are apt to be exposed to greater stresses, the probabilities of failure are greatly reduced.

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7-02 CONDUCTOR CONSTRUCTION:

The conductor will be 795 MCM, comprised of 26 strands of aluminum

and 7 strands of steel. It will be 1.108 inches in diameter and have an ultimate strength of 31,200 pounds. The resistivity at 60 cycles will be approximately 0.117 ohms per mile. All of the transmission line can be considered within the area where salt spray in some quantity is possible. For this reason the steel core strands will be aluminum clad to reduce corrosion.

7-03 CONDUCTOR SPACING:

Conductor spacing will be a basic 14.5 feet for the wood pole and approximately 22 feet in the steel section of the line except for long spans. At these points additional separation to allow for one or two node conductor galloping will be used (Reference 14).

7-04 INSULATORS:

The line is all within the area where some salt spray contamination is possible. Bonneville Power Administration states that their lines along the Oregon-Washington coast have become contaminated up to five miles inland. BPA lines are subjected to worse conditions than the Snettisham line in that the beaches are exposed to higher salinity open ocean spray and the Oregon-Washington region has longer periods of dry weather where rainfall does not wash the accumulated salt off the insulators. BPA has found that 6 fog type insulators give good protection to their 115kv lines where they have had flashovers with as many as seven standard insulators. The Alaska District discussed with BPA the feasibility of putting in test insulator strings to get data on contamination (BPA has insulation test installations on the Oregon-Washington coast). It was pointed out that the period of record would not be sufficient and that conditions were probably too variable to permit any significant results. Fog type insulators as listed below will be used on the line.

TABLE INSULATOR REQUIREMENTS

Structure Types (Reference 9)	No. Suspension Insulator/Phase	No. Tension or Angle Insulators/Phase
HS, HSB	8	
3A, 3AB, 3AT, 3AC		10
3T, 3TX	8	10
Steel	10	11

Where strings are inclined from the horizontal, the insulators will be placed so the bowls are facing downward. The insulators will be 10-1/2 by 6-1/2 inch with ball and socket fittings. Insulators with 5-3/4 inch spacing were considered but other agencies' experience has shown that hot line work is difficult with the closer spacings. The insulators will be of the 25,000 pound M&E type. Where additional strength on dead ends or angle structures is required, yokes and double strings will be used.

7-05 HARDWARE:

Wood pole hardware will conform to the general types outlined in the USBR's "Standard Specifications and Drawings for Construction" of 138kv and 161kv wood-pole, H-frame Transmission Lines, October 1960 (Reference 9). Crossarm hardware will be grounded by copper wire which will run down one pole and be wrapped around the butt of the pole as shown on Drawing 40-D-4978 in Reference 9. The hardware grounding is to prevent leakage currents from causing crossarm and pole burning under contaminated insulator conditions.

7-06 STRUCTURES:

a. Suitability to Sites - Wood pole structures will be used throughout the line except in the area from Snettisham 5.3 miles toward Juneau, where steel towers will be used (Plates 2 & 3). The selection of the individual structures will depend on conductor tension, span lengths, wind and ice

loadings, clearances, topography, reliability desired and geologic conditions. In general, structures will be located on high points where feasible. When possible, they will also avoid swamps, lakes, bays and slide areas. The line was located so as to be as accessible as possible, achieve minimum length and cost and be far enough from the salt water shoreline to minimize salt contamination. The section from the plant to mile 5.3 had no easy solution because of the steepness of the terrain and the scarcity of suitable sites to place the structures. Steel towers were selected for this portion of the line for the following reasons:

(1) Guying problems are avoided.

(2) Installation is easier. The steel towers can utilize a grouted steel bar for each leg. The bar hole may be either 3-1/2 or 4-inch and may be drilled with normal equipment without blasting which might disturb the surrounding rock. Steel towers also have greater adaptability to very steep slopes. The steel tower may have a 5-foot uphill leg and a 35-foot downhill leg with a 65-foot tower body; this would require a 70-foot uphill pole and a 95-foot downhill pole to achieve the same maximum 60° slope with 12-foot pole spacing. To blast a pole hole in rock, however, would require benching into the hillside to prevent breakout of the hole which would result in additional length added to the poles.

(3) Steel towers, if damaged, can be replaced in segments which lie within the lifting capability of small helicopters. Further, if the steel tower is damaged new legs can be welded to the reinforcing bar type footing using light portable welders. In the case of wood poles the stub would have to be removed and a new pole placed. This would require time-consuming digging in frozen earth or rock if the pole were broken in the winter or

spring period when a slide is most probable. If powder were used to remove the pole, replacing the pole would require aerial transport of backfill.

b. Ease of Erection - All structures will be designed for ease of erection. Where feasible, structures have been kept off steep hillsides. Wherever possible, similar materials will be used throughout so as to reduce to a minimum the stocking of material and requirements for transporting different types of materials during outages. Crossarms will all be of the sawn type. Consideration was given to spar types for simplicity of helicopter erection; however, linemen have difficulty standing on spars, hardware problems, reduced strengths, eccentric loading and checking causing rotting were factors which weighed against their use. Wood pole structures will be designed wherever possible, to be within the lifting capability of a 4,000 pound lift helicopter. Much of the line will most likely be constructed by helicopter since a good portion of terrain is too rocky, steep and timber covered to allow economic construction of roads and cat trails. If helicopters will be required on a major portion of the line, the contractor will undoubtedly utilize the machines throughout so as to reduce equipment and scheduling operations to a minimum. Further, with advent of heavy use of helicopters for military operations, greater volume production of the larger machines will reduce their costs and increase their availability. Helicopters have proven economical for line construction in many areas less rugged than the Juneau area. Some information on helicopters in construction is contained in References 15 to 34 and Section 9 following.

c. Availability of Materials - The materials for the transmission line wood structures are readily obtainable in the Pacific Northwest. The hardware will be standard type obtainable from a number of manufacturers

in the United States. Wherever possible hardware types will conform to those utilized by the Bureau of Reclamation to reduce maintenance supply problems. Steel towers are available from manufacturers in the United States; however, other government agencies are able to obtain very low prices from European companies. Towers will be standard with types used by large transmission line builders wherever possible to take advantage of reduced manufacturer set up costs, availability of replacement material and testing and design experience of other agencies. Suitable fog type insulators can be obtained in the United States from a number of manufacturers. Again, foreign manufacturers have supplied insulators at very low cost to other agencies. Generally foreign manufacturers will probably not bid on Snettisham equipment unless the balance of payments becomes favorable for the United States. The Corps of Engineers' policy, as outlined in ASPR 6-104.4(b) adds 50 percent to their bid prices for bid evaluation. Since United States manufacturers are aware of this, it is most probable that the Corps of Engineers will not be able to achieve the same costs on supplied equipment as are possible by other agencies. Ready mix concrete will be available in the Juneau area for the Juneau substation. The Taku Arm cable terminals will require the shipment of prescreened and washed gravel for the construction of the concrete portion of these facilities. Cable ducts and cable covers could be precast and shipped by barge to the terminals to reduce the volume of concrete to be mixed at these locations. Along the line, concrete requirements will be a minimum, generally limited to grouting at various tower footings. This requirement will be minimal for drilled hole type footings. For concrete pad type footings, of which there may be only a few, requirements for concrete will have to be met by helicopter supply. Probably mixing on a barge

and lifting concrete directly to the tower footings by helicopter will be the most economical. Near Snettisham, concrete and aggregate will be available from the contractor building the dam, powerplant and other works. Gravel in quantity is in short supply along the transmission route. The only sources are the outfalls of the creeks and a small amount along the beaches. A good supply of gravel can be found at Falls Creek, Taku Harbor, Limestone Inlet, Slocum Inlet and Crater Cove near the powerplant. Any large requirement for gravel or rock for the construction of roads may have to be met by crushing and/or blasting of rock. In certain areas, pole holes in rock, which will be excavated by blasting, may be deficient in rock backfill. In these cases backfill may have to be supplied from other sources. Rock is a desirable material for backfill where x-braced poles are expected to develop uplift forces.

7-07 SAFETY FACTORS:

The safety factors, as outlined by Reference 4, are as follows:

	<u>@ full load</u>	<u>@ 60°F, no wind</u>
Poles	2.0	5.5
Crossarms	4.0	5.5
Guys (angle)	2.0	-
Guys (line)	1.5	-
Insulator Strings	2.5	-
Conductor	2.0	4.0

Above safety factors will be checked against an ultimate load to achieve a balanced overall design (Reference 8). This will reduce the possibility that one portion of the transmission system will be much weaker. It will also allow reducing the cost on those items that are considerably stronger

than necessary. The values calculated for ultimate loads to failure may also be useful during operation of the system. If operations personnel are aware that conditions along the line are approaching ultimate loads they may be in a position to take remedial measures. The structures and line clearances will conform to the condition outlined in Reference 10, except where analysis shows other values to be more applicable. The overwater crossings, such as Limestone Inlet, must have sufficient clearance over high water to allow small craft to operate in safety. These crossings will be checked with Federal Aviation Agency for aircraft interference and marking and will also require navigation permits from the U. S. Department of the Army Permit Section.

SECTION 8
SUBMARINE CABLE

SECTION 8 - SUBMARINE CABLE

8-01 SUBMARINE CABLE:

a. The submarine cable must be designed for a number of stresses. These include laying stresses, repair stresses, oil pressurization stresses and stress generated due to temperature changes.

b. Cable laying stresses of necessity must be quite high since the cable weighs about 11.9 pounds per foot in air and about 8.9 pounds per foot in salt water and in the deeper section will be 680 feet below the laying barge. Therefore, the minimum possible tension for laying must be greater than 6000 pounds. Puget Sound Power and Light Company laid a similar cable at comparable depths with 10,000 pounds tension. The stresses in picking up the cable may be higher than those in laying.

c. The armor must be designed to protect the cable from abrasion and supply sufficient strength for laying and recovery. Galvanized steel armor has been widely used in this country for this purpose and has a record of good performance as far as strength and corrosion resistance are concerned. Aluminum armor was used on the Puget Sound Power and Light cable and has to date proved satisfactory.

d. The cable must also withstand oil pressures. It is desirable to keep a positive pressure at all times on the cable. Under load changes there is a change in oil temperature causing oil flow either from or to the reservoirs on either end. Under conditions of decreasing load after the cable has been carrying high current the oil flows toward the center of the cable. Positive oil pressure will be required to overcome the frictional resistance of the oil flow. Since the oil is about 0.9 the specific gravity of sea water, the land end of the cable will be subjected to

about 30 pounds per square inch static pressure at all times in order to maintain positive oil pressure at the 600-foot depth. Depending on the temperature changes and cable core design, the pressure required for overcoming frictional resistance will be around 20 pounds per square inch. This gives a total pressure of 50 pounds per square inch at the shore ends of the cable. This pressure, if contained by the lead sheath alone, would cause about 370 pounds per square inch stress in the lead. To reduce stresses and creep in the lead, some of the stress will have to be carried by materials outside of the lead. Certain manufacturers of cable have prestressed the lead sheath with glass fibers bedded in PVC to overcome the lead creep which occurs at 150 to 200 pounds per square inch. Also, metallic tapes have been used.

SECTION 9
COMMUNICATIONS

SECTION 9 - COMMUNICATIONS

9-01 COMMUNICATIONS:

a. It is contemplated that the communications to be installed for the construction period will consist of four voice channels and one teletype channel on a VHF, UHF or microwave radio link. This system may be adequate for operations of the plant until it is placed under supervisory control from Juneau. At that time, additional channels can be added, up to a maximum of 96 under one scheme now being investigated.

b. There is no carrier contemplated for relaying initially so there will be no carrier communication at this time.

c. The existing low frequency radio system can be retained as backup after construction is completed.

d. The communications are covered in greater detail in a "Communication" supplement to Design Memorandum No. 5, "Access and Construction Facilities."

SECTION 10
OPERATION
AND
MAINTENANCE

SECTION 10 - OPERATION AND MAINTENANCE

10-01 TRANSMISSION LINE:

a. Inaccessibility of a large portion of the line and ruggedness of terrain will cause operation and maintenance expenses to be much higher than costs for similar voltage lines in the contiguous states. Much of the equipment required is for emergency repairs and will be used very rarely in normal operation. The line can be patrolled most economically by helicopter, and on approximately 20 miles of line, would be the only practical way. If helicopters or airplanes are used for routine transportation of operators and maintenance personnel, the route of the flights could be along the right-of-way so that relatively frequent observations would be accomplished. Helicopter landing pads will be provided at or relatively near each structure, as required, to facilitate inspection and repair. It is also possible to construct roughly 26 miles of access road along or near the line but this would still leave the above 20 miles to be done by helicopter. The design of the structures will be such that they can be constructed by a 4,000 pound capacity helicopter and would, of course, be repairable by the same means. The helicopter would also be used for the yearly brush spraying program. Danger trees and evergreens growing in the right-of-way would be cut down by the linemen on their routine line patrols.

b. Operation and maintenance costs and assumptions are discussed further in Exhibit 2 and Table 1.

c. Space is available at Snettisham and Juneau for the storage of spare poles, crossarms, hardware, insulators, conductor, guy wire and spare steel tower sections for routine and emergency repairs.

10-02 JUNEAU SUBSTATION:

Maintenance and operation should follow routine USBR standard practices with the exception that it may be necessary to wash down insulator strings and bushings on a routine basis. All washdowns in the substation can be done with the equipment de-energized and without service interruption except for transformer bushings. Fog type nozzles should allow cleaning of energized transformer bushings in safety. Of course, insulator washdown will have to be conducted when the temperatures are above freezing. Some steel structure base lifting and regrouting may be necessary due to differential settlement of the fill material.

10-03 SNETTISHAM SWITCHYARD:

Operation and maintenance should follow routine USBR practices. There is no provision for taking the transformers into the powerhouse for repair. If major work is required, a temporary shelter will have to be built over the transformer and mobile cranes brought in from Juneau, if required. This decision is based upon the very low probability of transformer trouble during the 50-year life of the project. Insulator and bushing cleaning should not be necessary because of the very low salinity of the water and the very high rainfall level in the area.

10-04 SPARES:

It is anticipated that reasonable quantities of spare items will be included in construction procurement to permit initiation of operation with a normal stock of spare parts.

SECTION 11

REAL ESTATE REQUIREMENTS

SECTION 11 - REAL ESTATE REQUIREMENTS

11-01 DESCRIPTION:

The transmission line right-of-way will be approximately 44.6 miles long and 200 feet wide containing some 1,087 acres. From its point of origin at the project site to Bishop Point, the right-of-way is located entirely upon Government-owned land with a possible exception at Taku Harbor where it may cross a small portion of private land. From Bishop Point to the Juneau townsite boundary, a distance of approximately 4-1/2 miles, the right-of-way is located sufficiently inland to avoid all but some 180 feet of private land. The distance from the townsite boundary to the termination of the transmission line near Juneau is approximately 5 miles, the ownership of which is held by private interests and the State of Alaska. The right-of-way extends approximately 900 feet across several privately owned small tracts, 18,600 feet across numerous mineral surveys and mining claims, most of which are patented and all owned by Alaska-Juneau Industries, Inc., and 14,900 feet across State land. This State land has been selected and granted tentative approval which is tantamount to ownership. The mineral surveys and claims are lying idle and have not been in production for several years. As near as can be determined there are no other such interests in either the main project area or the remainder of the transmission line right-of-way. The Juneau substation will be situated on a 400' x 400' site approximately two miles southeast of the city center. Two tentative locations are under study; however, one of them, the A-J Industries' rock dump, will not be considered further due to the high cost of acquisition. The other location is the abandoned city garbage dump which the city has indicated it would consider making available for use as a substation site.

11-02 TYPE OF ACQUISITION:

The Corps of Engineers is to acquire all the land and rights necessary for the Snettisham project and is to construct said project after which it will be turned over to the Department of Interior, Bureau of Reclamation, for operation and maintenance. The transmission line right-of-way on Federal land will be acquired by use of Forest Service Construction and Use Permits, once the location of the line is finalized. Acquisition of the transmission line right-of-way across State and private lands will be accomplished by acquiring permanent easements. Since structures of a permanent nature are to be constructed on the substation site, this land will be acquired in fee.

11-03 RELOCATIONS:

There will be no relocations of roads, railroads, pipelines, utilities or cemeteries, etc. There will be some access roads to the transmission line.

11-04 REAL ESTATE COSTS:

a. Fee - No land is to be acquired in fee other than the Juneau substation site. Present consideration is being given the abandoned city garbage dump, an area of around 7 acres located approximately 3,000 feet southeast of the A-J Industries' rock dump. The suitability of this site will depend on foundation characteristics which are unknown at this time. The city has tentatively indicated it would make the land available at no cost; however, this has not been formalized. Its value is estimated at around \$25,000.

b. Easement - Of the total 1,087 acres in the transmission line right-of-way it will be necessary to acquire permanent easements covering

approximately 160 acres. This includes 69 acres of State land and 91 acres of private land. All but 5 acres of the private land consists of mineral surveys and claims owned by A-J Industries, Inc. The 5 acres involves the crossing of around four small tract ownerships of 4 to 5 acres each and a privately owned U. S. Survey tract. The actual right-of-way location has not been finalized and is subject to change at a later date. It is the policy of local power companies in both Anchorage and Juneau to acquire this type of easement for no consideration unless damages are incurred by the property owner. Even though the transmission line should be sufficiently above the road to be out of sight, its presence and the clearing of timber may still result in a loss of value to the private land amounting to as much as full value for the land involved. No sales of this land were found, however, they are valued at around \$500 per acre by the Borough Assessor which appears reasonable. The mining claims have been the property of A-J Industries for many years and have not been offered for sale even though they are not being operated. The Assessor values them at \$400 for a standard claim of around 20 acres or \$20 per acre. The State land has not been valued by the Assessor and in all probability easements can be acquired across it for little or no consideration. For purposes of this report, however, it will be valued at \$25 per acre taking into account the small amount of merchantable timber involved.

c. Administrative Costs - These costs are based upon an estimate of the actual work to be performed in acquiring the necessary real estate rights. Included are mapping and surveying, appraising, title evidence, negotiating and closing. They are listed as follows:

Mapping and Surveying -----	\$4,000
Appraising -----	500
Title Evidence -----	350
Negotiating and Closing -----	<u>650</u>
TOTAL	\$5,500

d. Summary - The following is a summary of the estimated real estate costs as of this time. Since the availability of the substation site at no cost to the Government is not presently known, its value will be included. A contingency factor of 25% is also included to cover the possibility of having to acquire easement rights over additional private land and to consider a possible increase in value from the date of this report to final project acquisition.

LAND RIGHTS

Government owned land - 923 acres	NO COST
State Land - Easement rights only	
69 acres @ \$25/acre	\$1,725.00
Private Land - Easement rights only	
86 acres mineral surveys @ \$20/acre	\$1,720.00
5 acres Small Tract Ownerships @ \$500/acre	\$2,500.00
Juneau Substation Site - Fee acquisition	<u>\$25,000.00</u>
Total Cost of Land Rights	\$30,945.00
Contingencies - (25% of \$30,945)	\$7,736.00
Administrative Costs -	<u>5,500.00</u>
Estimated Total of Real Estate Costs	\$44,181.00
Called	\$44,200.00

11-05 ACQUISITION SCHEDULE:

Application has been made for the withdrawal of Federal land in the main project area and approval is now pending. A Memorandum of Understanding between the Alaska District Engineer and the Regional Forester involving all the Federal land needed for planning and construction of the project is currently under study. It is presently the plan to have the transmission line right-of-way and substation site acquired by June 1967.

11-06 FUTURE CONSIDERATIONS:

Consideration should be given to acquiring right-of-way for a future additional line. It is anticipated that there will be little problem on the east side of Taku Inlet. On the Juneau side, the city will probably expand. Where possible, if there is no additional cost, an increased right-of-way width to 300 feet should be used. The additional width of 100 feet could be taken on the uphill side of the present projected 200-foot right-of-way. The additional width will probably fall outside of private land but will call for the additional acquisition of State of Alaska lands and mining claim lands owned by Alaska-Juneau Industries, Inc. The addition of a pulp mill could so radically alter power requirements in the Juneau area that second line planning may have to be instituted shortly after completion of the Crater-Long Lakes Project.

SECTION 12
COST ESTIMATES

SECTION 12 - COST ESTIMATES

12-01 GENERAL:

a. Cost Estimates - The cost estimates for Appendix B were computed from data from numerous sources. These included manufacturers' quotations and catalogs, data from Bureau of Reclamation, Bonneville Power Administration, U. S. Forest Service, contractors and various bid abstracts. Material costs were escalated to present values, as necessary, with appropriate allowances for freight. Labor costs were largely based upon the following:

- (1) Chugach Electric Association 90-mile, 115kv line built in 1962 from Cooper Lake to Anchorage.
- (2) U. S. Bureau of Reclamation Anchorage substation 1953 bids.
- (3) Puget Sound Power and Light Company experience.
- (4) U. S. Forest Service experience.
- (5) Bonneville Power Administration experience.
- (6) Contractors and abstracts of other bids.

Labor costs were then adjusted for time, roughness of terrain, weather conditions and increased transportation difficulties.

b. Contingencies - Contingencies used are 25 percent. More accurate estimates will be made upon completion of field work now scheduled for the summer of 1966. At that time the specific number of structures, their heights, accurate line and cable lengths, substation foundation conditions, tower weights and substation steel will be determined.

c. Summary - Summary by item and feature is included in this section. Also included are more detailed breakdowns of items used in the cost estimate. These estimates do not include contractor's profit and overhead which are included in the aforementioned summary.

The detailed estimate on "Land Rights and Rights-of-Way" can be found in Section 11 of this Appendix, titled "Real Estate Requirements." The detailed estimate for "Structures and Improvements" is shown under the breakdown for "Station Equipment" and consists of a \$68,000 building for Juneau substation. The detailed estimates for "Poles and Fixtures," as well as a portion of "Overhead Conductors and Devices", are tabulated in Tables 3, 4 and 5.

SECTION 12 - TRANSMISSION PLANT
COST ESTIMATE SUMMARY

Acc't No.	Account Description	Cost (\$1,000)					Totals by Accounts
		Juneau Substation	Snettisham Switchyard	Overhead Line	Submarine Cable	Government Cost 25%	
07.80	Land and Land Rights	\$ 25.0	\$ 0	\$ 8.6	\$ 0	\$ 8.4	\$ 42.0
07.81	Clearing and Rights of Way	0	0	353.1	0	87.9	441.0
07.82	Structures and Improvements	74.8	55.0	0	0	32.2	162.0
07.83	Station Equipment	670.6	248.2	0	0	230.2	1,149.0
07.84	Towers and Fixtures	0	0	293.2	0	73.8	367.0
07.85	Poles and Fixtures	0	0	823.5	0	205.5	1,029.0
07.86	Overhead Conductors and Devices	0	0	972.6	0	243.4	1,216.0
07.88	Underground Conductors and Devices	0	0	0	710.6	177.4	888.0
07.89	Addtl Cost f/all weather roads	0	0	1,430.0	0	358.0	1,788.0
		\$ 770.4	\$ 303.2	\$ 3,881.0	\$ 710.6	\$ 1,416.8	\$ 7,082.0

* Based on constructing 26 miles in the areas considered feasible at \$53,100 per mile plus 25% contingencies.

This figure per mile is from the Forest Service and from an Anchorage contracting firm.

Above costs include contractor's profit and overhead.

12-02 DETAILED ESTIMATE:

a. Clearing Lands & Rights-of-Way - Complete Disposal

Substation to Bishop Point

15.4 miles x 100' width

12.1 acres x 15.4 x \$850/acre * = \$158,500

North of Taku Harbor

12.1 acres x 0.25 x \$850/acre * = 2,500

Taku Harbor

12.1 acres x 1.25 x \$850/acre * = 12,800

Limestone Inlet

12.1 acres x 0.9 x \$850/acre * = 9,200

Subtotal \$183,000

b. Merchantable Timber to be Removed -

12.1 acres x 8.3 x \$600/acre ** = \$ 60,000

c. Timber to fell and lop -

12.1 acres x 16 x \$400/acre = \$ 78,000

TOTAL \$321,000

NOTE: Chugach Electric Association 1962 cost for clear, cut, grub and disposal in Chugach National Forest was \$425 per acre.

* Increased cost over CEA because of bigger timber and more difficult burning conditions.

** Increased cost because timber will be lower in value than costs to remove it.

12-03 DETAILED ESTIMATE:

a. <u>Snettisham Switchyard -</u>	<u>Mat'l & Frt</u>	<u>Labor</u>
Land and Land Rights	0	0
Structures and Improvements		
Rockfill, 20,666 yds x \$2.25		46,500
Gravel Surfacing 1,147 yds x \$3.00		3,440
		<hr/>
Subtotal		\$49,940
Station Equipment:		
Bus Compartments (none)		
Conduit, buried		
for transformers - 400' of 1' = 400'		
switchyard lights- 400' of 1' = 400'		
miscellaneous - 500' of 1' = <u>500'</u>		
Total footage 1,300'	600	1,200
cable trench, concrete 48" x 48" x 4"		
walls, 250' 40 cu yds	1,600	4,000
reinforcing steel @ 100 lbs/cu yd		
= 4,000 lbs	600	400
steel plate trench cover,		
1/4" x 5' x 250' = 12,500 lbs	3,000	500
		<hr/>
Subtotal	\$5,800	\$6,100
Control Equipment (none)		
Conversion Equipment		
two transformers, 26MVA, 13.8-138kv	106,000	18,000
Fence, 510', 7' high, 3-strand barb		
wire top	2,800	2,500

	<u>Mat'l & Frt</u>	<u>Labor</u>
Synchronous Condensers (none)		
Foundations		
8, concrete, for switchyard steel		
= 10 cu yds	400	800
2, concrete, for transformers,		
10' x 14' x 18" = 16 cu yds	640	300
reinforcing steel for above @ 75 lbs/		
cu yd = 1950 lbs	500	200
excavation and backfill for above		
100 cu yds	-	500
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Subtotal	\$1,540	\$1,800
General Station Equipment		
15 outdoor lights	900	900
Platforms, gratings, etc. (incl w/		
switchyard steel)	-	-
Primary and secondary voltage connections		
high voltage leads, 800 lbs	1,000	1,500
control cable (charge to P.P. as it		
ties in with control board)	-	-
power cables, low voltage only (assume		
3000' of 1/c #8 ave)	600	1,800
insulators, etc.	4,000	4,000
AC power distribution panel	400	400
substation steel structures, three		
138kv bays, single bus, no OCB's,		
@ 20,000 lbs/bay = 60,000 lbs	18,000	18,000

	<u>Mat'l & Frt</u>	<u>Labor</u>
gravel fill, 24" deep 1500 cu yds	-	3,000
ground mat, 4,000 lbs	2,000	10,000
Subtotal	\$26,000	\$38,700
Switchboards (none)		
Switching equipment		
2 - 138kv disconnect switches	10,500	3,000
Tools and appliances		
miscellaneous tools	500	-
TOTAL	\$154,040	\$120,940

b. Juneau Substation -

Cost of ground on city dump, 400' x 400' 25,000

Structures and improvements

concrete building for Juneau substa-
tion, 26' x 119' with 3,096 sq ft
@ \$22/sq ft (as per Plate 39 in main
report. Further foundation investi-
gation may indicate the desirability
of using prefabricated metal building.
A 4,000 sq ft building was quoted at
\$63,000 with foundation, insulation,
lighting, heating, plumbing and erec-
tion, complete by an Anchorage firm)

68,000

TOTAL

\$68,000

	<u>Mat'l & Frt</u>	<u>Labor</u>
Station Equipment		
Bus compartments (none)		
Conduit, buried		
69kv bays (4) @ 250' ea of 1" = 1000'		
23kv bays (4) @ 150' ea of 1" = 600'		
Transformer (1) A 200' of 1" = 200'		
Miscellaneous, 1000' of 1" = 1000'		
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Subtotal 2800'	1,100	2,000
Cable Trench, concrete, 48" x 48" x 4",		
570' = 85 cu yds	2,600	8,400
Reinforcing steel @ 100 lbs/cu yd,		
8500 lbs	1,100	900
Steel plate trench cover, 1/4" thick		
x 5' x 570' = 28,800 lbs	5,700	540
	<hr/>	
Subtotal	\$10,400	\$9,840
Control Equipment		
Battery, est. 120 A.H. 125v	5,000	2,000
Two chargers, est 20 amps	4,000	1,000
Control board, 8 panels	50,000	6,000
A.C. Power Board	2,500	1,000
D.C. Power Board	2,500	1,000
Six 69kv CTS	9,000	6,000
Six 69kv PTS, 2 bushings	14,000	6,000
Six 23kv CTS	5,000	4,000

	<u>Mat'l & Frt</u>	<u>Labor</u>
Six 23kv PTS, 2 bushings	7,000	4,000
	<hr/>	<hr/>
Subtotal	\$99,000	\$31,000
Conversion Equipment		
Auto transformer, 24/32/40MVA, 138-69-23kv	78,000	8,000
Station service transformer, 110KVA, 24,000-120/240v	1,000	500
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Subtotal	\$79,000	\$8,500
Fences, 1600', 7' high w/3 strand barb wire	8,000	6,500
Synchronous condensers (none)		
Foundations		
10 concrete, for 69kv bays, 3/4 cu yd ea = 7.5 cu yds	250	500
10 concrete for 23kv bays, .61 cu yd ea = 6.1 cu yds	200	500
1 transformer pad, 12' x 20' x 16" = 13 cu yd	400	100
1 station service transformer pad 4' x 6' x 12" = 1 cu yd	50	50
4 pads for intermediate bussing struc- tures, .61 cu yd = 2.5 cu yd	80	200
Reinforcing for above @ 75 lbs/cu yd = 3300 lbs	430	300

	<u>Mat'l & Frt</u>	<u>Labor</u>
Excavation and backfill for above		
100 cu yds	-	500
	<hr/>	<hr/>
Subtotal	\$1,410	\$2,050
General Station Equipment		
Air Compressor (rent, if required)	-	-
Test Equipment	5,000	-
Fault location equipment, 20 lights, outdoor type	1,000	1,000
	<hr/>	<hr/>
Subtotal	\$6,000	\$1,000
Platforms, gratings, etc. (included with switchyard steel)	-	-
Primary and Secondary Voltage Connections		
High voltage leads, 1500 lbs	1,500	2,500
Control cable, 3 to 12/c, #10 & #12, (assume 12,000 ft of 4/c average)	7,000	21,000
Power cables, assume 500' of 500MCM, 600 volt, 1/c, insulated	900	1,000
Small power wiring, assorted, assume 4000' of 1/c #8AWG (ave.) @ 13¢/ft	500	1,500
23kv copper tube bus, 1", 600' insul- ators, etc.	1,600	1,600
A.C. Power distribution panel	400	400
Substation steel structures		
Four 69kv bays @ 30,000 lbs ea	30,000	30,000

	<u>Mat'l & Frt</u>	<u>Labor</u>
Four 23kv bays @ 8,000 lbs ea	8,000	8,000
One 115kv disconnect switch structure		
@ 2,000 lbs	500	500
Gravel fill, 12" = 6000 cu yds	15,000	6,000
Ground mat, 4,000 lbs	2,000	7,000
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Subtotal	\$66,400	\$79,500
Switchboards with control equipment (none)		
Switching Equipment		
Three 69kv OCB's, 1200 A, 250MVA	95,000	3,600
Ten 69kv disconnect switches	28,000	3,000
Three 23kv OCB's, 600 A, 500MVA	45,000	2,700
Thirty 23kv disconnect switches, 1-pole,		
hook stick operated	6,000	1,000
One 138kv disconnect switch w/grounding		
blade	6,000	1,300
One 138kv high speed grounding switch	4,000	1,000
One 2-pole 23kv fused disconnect switch	400	400
	<hr/>	<hr/>
Subtotal	\$184,400	\$12,400
Tools and Appliances		
Switch Hooks, one 12' and one 16'	100	-
Oil filter and centrifuge	3,000	100
Miscellaneous tools	500	-
	<hr/>	<hr/>
Subtotal	\$3,600	\$ 100
TOTAL	\$458,810	\$150,890

	<u>Mat'l & Frt</u>	<u>Labor</u>
c. <u>Detailed Estimate, Steel Towers -</u>		
Steel - material, \$0.25 x 12,000 x 37	111,000	
labor, \$0.25 x 12,000 x 37		111,000
Insulators, suspension 870 @ \$7.32	16,700	
Insulators, dead end 1400 @ \$7.32		in hdw
Hardware, suspension, material \$51 x 29	1,500	
Hardware, suspension, labor \$199 x 29		5,800
Hardware, dead end, material \$347 x 8	2,800	
Hardware, dead end, labor \$303 x 8		2,400
Footings, grouted steel bar:		
material \$220 x 37	8,200	
labor \$980 x 37		36,300
TOTAL		\$290,000

d. Detailed Estimate, Overhead Conductor & Devices -

795MCM 26/7 ACSR

material 706,000' x 1.094lbs/ft x \$0.38	294,000	
labor 706,000' x 1.094lbs/ft x \$0.44		340,000

Overhead Ground Wire *

material, 2 miles x \$1,000	2,000	
labor, 2 miles x \$1,000		2,000

Other Devices

Stockbridge Dampers **

material, 900 x \$13.00	11,700	
labor, 900 x \$32.00		28,700

TOTAL		\$372,000
-------	--	-----------

* Only 1/2 mile from switchyard, cable terminals and substation

** For estimating purposes 16 lb dampers assumed on long spans and exposed deadends.

	<u>Mat'l & Frt</u>	<u>Labor</u>
e. <u>Detailed Estimate, Submarine Cable -</u>		
Armored Conductors, buried (none)	-	-
Armored Conductors, submarine cable		
250 MCM, 138kv, steel armoured,		
57,000 ft, mfrs quote @ \$5.73/ft	327,000	-
Add \$0.25/ft for plastic bedding	14,300	-
Freight on 684,000 lbs to Seattle	28,000	-
8 Potheads, 138kv, frt & hndlg	19,000	10,000
8 Oil Reservoirs, fndtn & fence	20,000	4,000
Valve Panel	300	500
3600' of 1-1/2" oil pipe to reservoir	3,200	4,000
2 Cable Terminal Buildings		
150 cu yds concrete, each bldg	9,000	18,000
14,000 lbs reinforced steel, each	3,640	2,800
Insulators, lightning arresters,		
steel stands, pipe	40,000	4,800
138kv leads & jumpers, 120'	120	300
Bonding sheath & armor at low water line	200	500
Insulation under potheads	100	100
Precast Cable Trench, 400'		
Concrete, 80 cu yds	2,400	2,000
Reinforced steel for trench		
@ 150 lbs/cu yd	1,800	1,200
Excavating ditch & placing		
concrete trench	-	1,000

	<u>Mat'l & Frt</u>	<u>Labor</u>
Laying 57,000' of Cable		
Rental of barge from Seattle, 1-1/2 mos.		6,000
Reinforcing barge deck for large reels		5,000
Renting or fabricating 4 large reels		
(it may be possible to coil cable in		
hold of barge and eliminate these reels)	4,000	4,000
8 splices, \$1,000 each	8,500	10,000
Reeling cable on large drums	-	2,000
Large tug rental, one month	-	20,000
4 small tug rental from Juneau, 2 wks	-	25,000
Driving 4 dolphins, 3 piles each,		
500' total @ \$8.00/ft	1,000	3,000
Contractor's men from Seattle, assume 10		
men for 10 days @ \$100/man-day	-	10,000
Local labor, assume 20 men for 10 days		
@ \$75/man-day	-	15,000
Treating oil, filling reservoirs, etc.	7,000	2,000
High potential testing	-	5,000
	<hr/>	<hr/>
TOTAL	\$489,560	\$156,200

SECTION 13
SCHEDULE
FOR
DESIGN
AND
CONSTRUCTION

SECTION 13 - SCHEDULE FOR DESIGN AND CONSTRUCTION

13-01 SCHEDULE FOR DESIGN AND CONSTRUCTION

The weather is the most important controlling factor in scheduling construction of the Snettisham Transmission Line. In the vicinity of Speel Arm the snow remains late; to mid-May or later in spots and the weather becomes quite adverse by October (winds, rain, fog and snow). The best months to accomplish field work are June through September. Around Juneau, the season is longer. The schedule, as shown in Figure 4, calls for the following:

- a. Plan, profile, cable terminal surveying and substation terminal surveying all completed by October 1966.
- b. Foundations and Materials investigations for the transmission structures, cable terminals, Juneau substation and underwater cable to be completed by October 1966.
- c. Design Memorandum No. 9 will be utilizing raw data as it becomes available and is currently scheduled to be finished by December 1966.
- d. Plans and specifications will be completed after approval of Design Memorandum No. 9.
- e. Right-of-way clearing will start as soon as possible after Design Memorandum No. 9 has been approved. If the right-of-way is cleared at an early date, bidders on the transmission line will have a better opportunity to evaluate construction problems leading to more competitive bids.
- f. The submarine cable will be advertised for several months to allow foreign bidding. If the Corps of Engineers' 50 percent policy has not relaxed by that date, it is probable that only domestic manufacturers will bid. The cable takes in excess of 12 months to manufacture and deliver.

The cable may be laid by the manufacturer on a turnkey contract or may be put out on bid to a laying contractor.

g. The construction schedule for the overhead section of the transmission line provides two seasons for completion. If the contractor elects helicopter construction, it may be completed in one season.

h. The construction schedule for the substation provides two seasons for construction. This is more than will probably be required. (Figure 4)

SECTION 14

REFERENCES

SECTION 14 - REFERENCES

14-01 REFERENCE LITERATURE:

Following is a partial list of references used for this study:

- (1) Westinghouse Transmission and Distribution Reference Handbook-1950
- (2) The Calculation of the Temperature Rise and Load Capability of Cable Systems, J. H. Neher and M. H. McGrath. AIEE Transactions, Vol. 76, Part III, Oct. 1957, p.752-64.
- (3) Specifications for Impregnated Paper, Insulated Lead-Covered Cable, Oil-Filled Type, 5th Edition, Jan. 1951, by Association of Edison Illuminating Companies.
- (4) Design Standards No. 4, Power Systems, Chapter 4, Transmission Lines, U.S.B.R., May 1961.
- (5) Graphic method for sag tension calculations for ACSR and other conductors. ALCOA Aluminum Overhead Engineering Data, Section 8, Aluminum Company of America.
- (6) Current-Temperature Characteristics of Aluminum Conductors, ALCOA Conductor Engineering Handbook, Section 6, Aluminum Company of America.
- (7) Sag-Tension Computations and Field Measurements of Bonneville Power Administration, Paul F. Winkleman. Power Apparatus and Systems, Feb. 1960.
- (8) A Guide to Transmission Structure Design Loadings, F. W. Farr, et al. Power Apparatus and Systems, November 1964.
- (9) Standard Specifications and Drawings for Construction of 138 and 161-kilovolt wood-pole, H-frame Transmission Lines. U.S.B.R., October 1960.

(10) Safety Rules for the Installation and Maintenance of Electric Supply and Communication Lines. National Bureau of Standards Handbook 81, U.S.D.C., November 1961.

(11) Wind Loads on Overhead Lines (for U.S.S.R.) A. P. Birjulin, V. V. Burgsdorf, B. J. Makhlin, CIGRE, Paris, France, Report 225, 1960.

(12) ALCAN Nechaki-Kemano-Kitimat Development. Reprint from The Engineering Journal, November 1954 and April 1953.

(13) The Mechanical Behavior of Bundled and Single Conductors. New Measurements of Hornisgrinde Testing Station. W. Lerbfried, H. Mors. CIGRE, Paris, France, Report 209, 1960.

(14) Overhead Conductor Vibration, ALCOA Aluminum Overhead Conductor Engineering Data, Section 4. Aluminum Company of America.

(15) Electrical Engineering, August 1964, page 534.

(16) Electric Light and Power, June 1962, page 50.

(17) Electrical West, February 1963, page 40-47.

(18) Electrical West, May 1965, page 23.

(19) Electrical World, 17 August 1959, page 58.

(20) Electrical World, 18 June 1962, page 8.

(21) Electrical World, 25 June 1962, pages 57, 58, 86 and 100.

(22) Electrical World, 30 June 1962, page 44.

(23) Electrical World, 20 August 1962, page 66.

(24) Electrical World, 27 August 1962, page 70.

(25) Electrical World, 3 December 1962, pages 30, 36 and 38.

(26) Electrical World, 7 June 1963, page 25.

(27) Electrical World, 14 June 1963, page 55.

(28) Electrical World, 15 July 1963, page 79.

- (29) Electrical World, 12 August 1963, page 112.
- (30) Electrical World, 16 October 1963, page 94.
- (31) Electrical World, 2 March 1964, page 24.
- (32) Electrical World, 18 July 1964, page 72.
- (33) Electrical World, 7 September 1964, page 62.

EXHIBITS

EXHIBIT 1

TAKU INLET SEDIMENT SAMPLE ANALYSIS

25 March 1965

<u>Sample No.</u>	<u>Density Lbs/Cu. Ft-Dry</u>	<u>Percent Moisture</u>	<u>Classification</u>
NW-1	83.4	80.4	ML
NW-2	70.0	50.7	ML
NW-3	62.7	48.3	ML
NC-1	41.0	105.6	ML
NC-2	54.8	53.5	ML
NC-3	22.6	22.8	ML
NX-ES #1	98.5	34.0	ML
NX-ES #2	81.8	35.0	ML
NX-ES #3	82.0	35.2	ML
CC-1	48.9	98.6	MH
CC-2	43.6	101.1	MH
CC-3	43.9	97.1	MH
SC-1	44.2	98.8	MH
SC-2	44.6	93.9	MH
SC-3	45.0	90.4	MH
SE-1	83.6	36.3	ML
SE-2	67.4	51.7	ML
SE-3	70.2	44.6	ML
EX ES #3	76.2	38.9	SM
EX ES #2	43.6	124.4	SM
EX ES #1	91.5	31.6	SM
SW-1	69.6	81.6	ML
SW-2	82.9	25.8	SC
SW-3	71.6	40.6	MH
WC-1	52.2	86.8	SM
WC-2	84.2	32.2	ML
WC-3	11.8	286.4	OL

EXHIBIT 1

EXHIBIT 2

SELECTED COMPUTATIONS

SNETTISHAM TRANSMISSION LINE

Operation and Maintenance - Roads vs Helicopters, Comparative Analysis

ROAD

Miles of cat trail	26	
Cost/mile	\$20,000	
Total Cost	\$520,000	
Maintenance @ \$100/mi-yr		2,600
Annual cost of capital investment	\$520,000 (0.0397)	<u>\$20,700</u>
	Annual Cost	\$23,300

HELICOPTER

Bell 204B Type - capacity 2800 lbs cabin

or 4000 lbs sling

First Cost	\$350,000
Life	10 yrs
Salvage	\$50,000
Fuel Consumption	70 gal/hr kerosene
Maintenance	\$100/op. hr.

$$\begin{aligned}\text{Capital Recovery} &= 350,000 - 50,000 \text{ (Crf-3-1/8-10)} + 0.0313 (50,000) \\ &= \$36,760/\text{yr}\end{aligned}$$

$$\text{Hourly cost } 70 \times \$0.25/\text{gal} + \$100 = \$120/\text{flight hr.}$$

EXHIBIT 2 (cont'd)

Snettisham Transmission Line - Roads vs Helicopters

Surplus Helicopter w/\$50,000 modifications and reconditioning

0 Salvage

Life 10 yrs

Fuel, maintenance same as above

CR = 50,000 (Crf - 3-1/8 - 10) = \$5,900/yr

Assumption of use

Summer, 4 mos. @ 4 hrs/day @ 2 das/mo = 32 hrs

Winter 8 hrs

40 hrs/yr

Annual Cost

New Machine

First Cost \$36,760

Maintenance & fuel
(120 x 40) 4,800

Pilot (part time) 1,000

\$42,560/yr

Surplus Machine

First Cost \$5,900

Maintenance & fuel
(120 x 40) 4,800

Pilot 1,000

\$11,700/yr

EXHIBIT 2 (cont'd)

Snettisham Transmission Line - Roads vs Helicopters

Road Equipment Costs for Pole Work/yr

Equipment	Life	First Cost	Salvage	Use/yr
D-6 w/winch	10 yrs	25,000	5,000	180 hrs
Line Truck	5 yrs	9,000	1,000	25 hrs
Barge	15 yrs	50,000	5,000	200 hrs

Cat Cost/yr

$$\begin{aligned} CR &= (\$25,000 - 5,000)(Cr_f - 3-1/8 - 10) + 5,000 (0.0313) \\ &= 2,517/\text{yr} \end{aligned}$$

Truck Cost/yr

$$\begin{aligned} CR &= (\$9,000 - 1,000)(Cr_f - 3-1/8 - 5) + 1,000 (0.0313) \\ &= \$2,000/\text{yr} \end{aligned}$$

Cat Fuel Cost and Maintenance

$$\$5/\text{yr} \times 180 = \$900/\text{yr}$$

Truck Fuel Cost and Maintenance

$$\$4/\text{hr} \times 25 = \$100/\text{yr}$$

Landing Barge Costs

Assume barge not required for other purposes - charter boats available

$$\begin{aligned} CR &= (\$50,000 - 5,000)(Cr_f - 3-1/8 - 15) + 5,000 (0.0313) \\ &= \$3,956/\text{yr} \end{aligned}$$

Fuel Cost \$4/hr + \$1 oil

Maintenance \$3/hr

$$\text{Cost/yr } 200 \times \$8/\text{hr} = \$1,600/\text{yr}$$

EXHIBIT 2 (cont'd)

Snettisham Transmission Line - Maintenance of Roads vs Helicopters

Outage Calculations

Maintenance of line break near Taku Inlet

Description		Roads min.	Helicopter min.
(1) {	Scout line and report	40	0
	Load	30	10
	Run to site	180	15
	Clear snow move equipment 1 mi	60	0
	(Repair)	same	same
	Move equipment back	20	0
	Load	10	10
	Run	180	15
	Unload	30	10
	Total	550	60
		9 hrs - 10 min	1 hr

Duration of outage (1) = 310 min

T = 310 - 25 = 285 min

= 285/60 = 4 hrs 45 min

Value of T @ \$8,610/day

\$8,610 x 4.75/24 = 1,700/outage

Additional Labor and equipment time 8.16x\$40/hr 330

2,030

Use 2,100

EXHIBIT 2 (cont'd)

Snettisham Transmission Line - Maintenance of Roads vs Helicopters

Comparative Costs

Description	Helicopter \$/yr	Road \$/yr
Road and maintenance		22,300
Helicopter, large	42,560	--
Helicopter Hiller 12E (\$100/hr for areas where there are no roads)	3,000	5,000
Cat		3,400
Truck		2,100
Barge		4,600
Additional transport time (3 men 1 hr/day @ \$10/hr for 80 hrs/yr)		2,400
Additional labor time to move poles to roadless spots by winch (beyond Hiller 12E capacity)		3,000*
6 poles/yr @ \$500/pole		
Subtotal	\$45,560	\$42,800
Additional cost for 1 outage per year of road		2,100
	45,560/yr	44,900/yr
Savings road over helicopter	660/yr loss	
If surplus helicopter used	14,700	44,900/yr
Savings of helicopter over roads	\$29,200/yr	

AD-A067 895

CORPS OF ENGINEERS ANCHORAGE AK ALASKA DISTRICT
SNETTISHAM PROJECT ALASKA. FIRST STAGE DEVELOPMENT. DESIGN MEMO--ETC(U)
OCT 65

F/G 13/2

UNCLASSIFIED

4 OF 6

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EXHIBIT 2 (cont'd)

Snettisham High and Low Line Comparison

PROSPECT POINT TO CIRCLE POINT

Data: Voltage 138kv

Conductor 26/7 795MCM ACSR

<u>Line Data:</u>	<u>High Route</u>	<u>Low Route</u>
Length, feet	58,600	103,650
Length, miles	11.08	19.60
Wood pole length, miles	5.63	19.00
Steel Tower length, miles *	5.47	-
Angle Structures	37	32

Assumptions:

Tower Weights, 12,100 lb avg

Footings, grouted bars

Steel Cost, \$0.60/lb erected

O & M Cost/mi, \$20,200 PW

* Steel towers used in those areas above 2000 foot elevation.

EXHIBIT 2 (cont'd)

Snettisham High and Low Line Comparison

<u>Description</u>	<u>High Line</u>	<u>Low Line</u>
	\$	\$
Towers, 58 x \$8,460	491,000	
Insulators, steel section	17,320	
Wood Poles *, \$21,800 x 5.63	123,000	
\$21,800 x 19.60		428,000
Conductor, wood section, \$14,200 x 5.63	80,000	
\$14,200 x 19.60		278,000
Conductor, steel section, \$16,200 x 5.47	88,500	
Clearing, steel section	0	
Clearing, wood \$7,750 x 5.47	43,600	
\$7,750 x 19.60		151,700
Losses, Present Worth, \$12,300 x 11.08	136,500	
\$12,300 x 19.60		242,000
Subtotal	979,920	1,099,700
O & M, Present Worth **		
\$20,200 x 11.08	224,000	
\$20,200 x 19.60		396,000
Interim Replacement ** $0.2\% \times 25.13 = 5.02$		
0.0502 x 676,820	34,000	
0.0502 x 706,000		36,000
Subtotal	1,237,920	1,531,700

* Includes guys, insulators, etc.

** Same values of steel as wood due to adverse conditions

EXHIBIT 2 (cont'd)

Snettisham High & Low Line Comparison

<u>Description</u>	<u>High Line</u>	<u>Low Line</u>
Outage Value		
(4)(8,610)(25.13)	865,000	
(1)(8,610)(25.13)		216,000
GRAND TOTAL	\$2,103,000	\$1,748,000
Difference	\$355,000	

CHEROKEE FLATS TO THANE

Data: Voltage 138kv
 Conductor 26/7 795MCM ACSR
 Miles HL = 6.36 LL = 6.77

<u>Description:</u>	<u>High Line</u>	<u>Low Line</u>
* Wood Poles, \$21,800 x 1.5 x 6.36	207,000	
\$21,800 x 6.77		147,000
Conductor, \$16,200 x 6.36	103,000	
\$14,200 x 6.77		96,000
Clearing, \$ 7,750 x 6.77	0	52,300
Losses, Present Worth, \$12,300 x 6.36	78,200	
\$12,300 x 6.77		83,200
O & M Present Worth, \$20,200 x 6.36	128,000	
\$20,200 x 6.77		137,000

* Includes guys, insulators, etc.

EXHIBIT 2 (cont'd)

Snettisham High and Low Comparison

Interim Replacement	0.0502 x \$310,000	15,000	
	0.0502 x \$243,000		12,000
Subtotal		531,000	528,000
Outage Value **		216,000	0
GRAND TOTAL		\$747,000	\$528,000
Difference	\$219,000		

**VALUE OF OUTAGE IN POWER SALES

Most Probable Period:

<u>Month</u>	<u>%KWH/mo</u>	<u>days/mo.</u>	<u>%/day</u>
Dec	9.70	31	0.313
Jan	8.96	31	0.289
Feb	8.13	28	0.291
Mar	8.38	31	0.270
			<hr/>
			1.163

$$4 \frac{0.291/\text{day}}{1.163}$$

KWH/day

$$0.291/100 \times 328.3 \times 10^6 \text{ KWH} \quad 957,000 \text{ KWH}$$

$$@ 9 \text{ mills} = \$0.009 \times 957,000 = \$8,610/\text{day}$$

EXHIBIT 2
SELECTED COMPUTATIONS

Formula for Computing Losses

Computation of Formula for I^2R losses on conductor and cable:

- B = average power
- X = some increment the load curve varies from B
- K = some constant composed of resistance and voltage
which can be multiplied by kilowatts squared to
give losses
- n = number of records used
- K' = X/B

$$\text{Average loss} = KB^2 \left[1 + \frac{2}{n} \sum_{J=1}^{J=n} K_J^1 + \frac{1}{n} \sum_{J=1}^{J=n} K_J'^2 \right]$$

This formula allows the use of any load curve shape with any base power amount for any voltage. It was computed for the Juneau load curve and used on the Snettisham transmission computations and gave a figure of approximately 9 percent increase (value inclosed in brackets) for the base I^2R which is the result of KB^2 .



IN REPLY
REFER TO: 640

EXHIBIT 3
UNITED STATES
DEPARTMENT OF THE INTERIOR

BUREAU OF RECLAMATION

ALASKA DISTRICT HEADQUARTERS
P. O. BOX 2567, JUNEAU, ALASKA

November 6, 1964

District Engineer
U.S. Army Engineer District, Alaska
P.O. Box 7002
Anchorage, Alaska 99501

Re: NPAEN-PR-R

Dear Sir:

We are agreeable to your proceeding with procurement of the following described work boats for use at the Snettisham Project, as discussed with representatives of your office:

1 - Landing Craft capable of ferrying a line truck, D-8 tractor, materials and associated personnel.

1 - Materials and Supplies Boat of the 40 to 50 foot, 20-knot class, twin diesel powered engines, ice resistant hull, and a seaworthy design to enable use in high rough seas.

As mutually agreed, craft of the above description shall be procured by the U.S. Army Engineer District, Alaska. They will be used by Corps of Engineer personnel during the design and construction phases of the project and later turned over to the Bureau of Reclamation for operation and maintenance use. During the transition period from construction to operation of the project, interagency coordination for joint use of the boats will be required.

The following evaluation of O & M costs for the transmission line over the high mountain pass route compared to the sea level route are submitted for cost comparison purposes only:

High Pass Route (Slocum Inlet-Mallard Cove)

	Miles	Construction Cost	Replacement	O & M
Steel Towers & Fixtures	5.0	1,260,000	\$ 649	\$2,150
Wood Pole & Fixtures	5.5	103,000	471	1,210
			<u>\$1,120</u>	<u>\$3,360</u>
Total OM&R			\$4,480	

EXHIBIT 3

Sea Level Route (Slocum Inlet-Mallard Cove)

	<u>Miles</u>	<u>Construction Cost</u>	<u>Replacement</u>	<u>O & M</u>
Steel Towers & Fixtures	0	0	0	0
Wood Pole & Fixtures	18.5	346,000	\$1,580	\$4,070

Total OM&R \$5,650

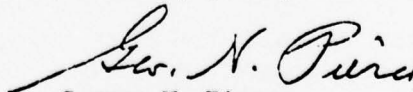
Average loss of revenue due to one 24 hour outage =
730,000 kwh/day x \$.008/kwh = \$5,840/day

Line damages which would require possibly one day to repair on the sea level routing could require 2 or more days to repair on the high-pass routed line. In addition, we estimate that outage frequency will be at least double on the high pass route or an outage ratio of 4 to 1 for the two routes. The loss in revenue annually, therefore, would be 4 days x \$5,840 per day = \$23,360 on the high pass route. This compares to the related one day revenue outage loss of \$5,840 and the additional annual OM&R cost of \$1,170 for a total of \$7,010 for one severe line outage per year on the sea level route. The net difference in OM&R and potential revenue loss between the two routes is \$16,350 annually, which has a present worth value of \$460,320 if paid annually, for fifty years at 2.5% interest.

We trust that the above evaluation, together with the information you have received from the Forest Service, will enable you to complete your study of the relative economics of the alternative routes.

Unfortunately, it is not possible to place a monetary value on the personal hardships and business losses which will be incurred in the Greater Juneau-Douglas area if extended outages are experienced on the Snettisham transmission line. These are very serious intangibles, however, and certainly favor the Sea Level Route.

Sincerely yours,



George N. Pierce
District Manager

cc: Chief Engineer, Attn: D240/D-600
Commissioner, Attn: 600

TABLES

EXPLANATIONS OF TABLE 1

ECONOMIC ANALYSIS - OVERHEAD LINE SECTION

- (1) Voltages were chosen from 69kv to 230kv to absolutely cover the entire range of feasible voltages.
- (2) The current is shown for each voltage at 77MVA load. This load is equal to the 69MW plant overload capacity divided by a 0.9 power factor.
- (3) Conductors are aluminum cable, steel reinforced. Sizes were chosen for study which were estimated to bracket the most economical size. On the 138kv, 161kv and 230kv tabulations, the minimum size shown is the minimum size which can be used due to corona loss limitations.
- (4) The current capacity of each conductor is from Ref 1, page 50.
- (5) Conductor weights are for 134 miles of single phase conductor and are taken from Ref 1, page 50.
- (6) Conductor material costs - a price of .38¢/lb was used for this study. This is approximate catalog price plus freight.
- (7) Labor Cost - CEA prices for 556.5MCM (1.1 million lbs) was .69¢/lb total in 1959 and 1962. At .38¢ material cost, labor was .31¢/lb. At Snetisham, due to the rougher terrain, it was assumed that stringing would be a greater percentage of conductor material cost and stringing costs were increased by approximately 40 percent over CEA prices. An estimating graph, Figure 5, was made to take into account the variation in stringing cost per pound with changes in conductor size.
- (8) Cost of Structures - It was assumed that a steel tower section runs 5.3 miles from the powerplant and wood pole H-frame construction continues from this point into Juneau, except for the 2.7 mile submarine cable crossing, making 39.3 miles of wood pole line. The weights of standard steel towers used on the coast were used to arrive at the following tons per mile. The installed cost of towers on the west coast has been around 25 to 30 cents per pound and one job by Chugach Electric Association in Anchorage, in 1962 with 9 special towers, cost 81 cents per pound. Since we will have about 37 towers, more or less standard, an installed price of 50 cents per pound has been used.

	<u>Tons/Mile</u>	<u>Cost</u>
69kv	26	\$138,000
115kv	28	148,000
138kv	29	154,000
161kv	30	159,000
230kv	32	170,000

TABLE 1 EXPLANATIONS

In addition to the steel erection costs, a figure of \$1,200 per structure for tower footings has been added. For 37 structures, this adds a cost of \$45,000. It is expected that a large percentage of the footings can be reinforcing bars grouted into drilled holes in the rock. Where rock is not near the surface, a dug hole with a steel plate type footing will be used. A profile along 10 miles of the route of the wood pole line was roughed out using the contours as shown on Plate 5. Structures were spotted and conductor was drawn in using a sag template for 795MCM ACSR and ground clearances specified by the U. S. Bureau of Reclamation. A takeoff was then made of the heights and types of structures required for all voltages, taking into account different ground clearances, lengths of insulator strings and depths of pole settings. The cost for each type and height of structures was estimated by escalating by 9 percent the prices paid by Chugach Electric Association in 1962 on their 115kv line. The costs of the other voltage lines were adjusted by the installed cost differences for crossarms, X-braces, and insulator strings. U. S. Bureau of Reclamation standard HS suspension structures on tangent sections and three-pole dead-end structures at all turn points were assumed.

- (9) Clearing Costs - Clearing cost data was taken from the 115kv line of Chugach Electric Association built in 1962. Their clearing cost was \$425.00 per acre in the most densely wooded areas. Snettisham clearing costs are estimated at 50 percent more than this because of the less accessible terrain. The clearing widths assumed are 80 ft for 69kv, 100 ft for 115kv and 138kv, 110 ft for 161kv and 125 ft for 230kv. Also, since there are numerous rocky areas, canyons and water crossings where clearing is not required, the total length is assumed as 40 miles for calculating acreage.
- (10) Total Construction Costs - The total construction costs are the sum of Columns 6, 7, 8 and 9. These costs are estimated for relative comparison of different schemes and should not be considered as detailed cost estimates.
- (11) - (14) Present Worth of Losses - Losses during first 13 years when water is spilled have no value. After the 13th year with 331 million kilowatt-hour per year delivery, average kva is 42,000 at 0.9 P.F. and I^2R losses are based on this kva with the effect of charging current. These losses have been calculated for each of loading assumptions A, B, C and D. Under assumption A, no load growth after 14th year, the sum of the value of the losses for 37 years is 21.75 times the yearly losses. Under assumptions B, C and D, the present worth is the sum of the products of two diverging series. The losses are increasing as the square of the current and the present worth value of the losses is declining, as they occur further and further in the future. Under any of these assumptions only a multiplying factor is necessary to convert the losses from assumption A. The formula for calculating present worth loss factor K_s is as follows:

$$K_s \left[\frac{(1+r)^2}{1+i} - 1 \right] = \frac{(1+r)^{2n} - 1}{(1+i)^n}$$

where r = rate of load growth
i = interest rate, in this case 3-1/8%
n = number of years in series

The loss factors for the different assumptions are as follows:

	K_s	Ratio of PW's
A	21.75	1.0
B	64.60	2.97
C	-	4.95
D	-	3.86

- (15) Present Worth of Interim Replacements - The Federal Power Commission Technical Memorandum No. 1, "Instructions for Estimating Electric Power Costs and Values," dated April 1, 1960 recommends interim replacement factors of 0.20% for wood pole transmission lines and 0.10% for steel tower lines. A .20% factor has been assumed on this line to give an additional allowance for more severe condition at Snettisham. Clearing costs are not included in the line costs.
- (16) Present Worth of O & M Costs - These costs can vary considerably depending upon the method of transportation utilized for moving men and equipment. If a new, large helicopter is purchased for \$350,000, the yearly cost is estimated at \$21,280 for helicopter debt service and operation, \$3,000 for renting a small copter, \$1,000 for spraying, \$1,000 for materials and \$10,000 for labor, making a yearly cost of \$36,280, say \$36,000. This figure assumes that half the cost of the large copter is charged against investigations. If a used large helicopter is procured and reconditioned for \$50,000, the total yearly cost falls to approximately \$23,000. If roads are built where economically feasible, a small copter used for wintertime patrolling and a large copter flown in for any heavy lifting for emergencies in inaccessible areas, the yearly cost becomes \$22,300 for road debt service and maintenance, \$5,000 for small helicopter rental, \$2,000 for spraying, \$1,000 for materials, \$10,000 for ground equipment debt service and operation, and \$12,000 for labor, making a total of \$52,300, say \$52,000. Debt service and maintenance for roads is included in this calculation since it appears quite probable that this line could be built by helicopter without any appreciable roads. Since the roads would then be built solely for O & M, their capital recovery and maintenance costs can properly be charged to this account. For the purpose of this study, a small variation in cost was assumed for different voltages to reflect differences in material, labor and spraying costs. The yearly costs assumed are for operation with a new large copter as follows.

Voltage (kv)	Cost/year
69	\$34,000
115	35,000
138	36,000
161	37,000
230	38,000

(17) - (20) Total Present Worth - These columns represent the comparative 50-year costs of the different voltages and conductors under each of the four loading assumptions.

TABLE 1

ECONOMIC ANALYSIS - OVERHEAD LINE SECTION

KV (1)	Current @ 77 MVA (Amp) (2)	CONDUCTOR					Struct. Costs (M/\$) (8)	Clear'g Costs (M/\$) (9)	Total Const Costs (M/\$) (10)	PRESENT WORTH OF LOSS UNDER ASSUMPTION		
		Size	Cap'y	Weight for 134 mi.	Mat'l Cost	Labor Cost				A	B	C
		(MCM) (3)	(Amp) (4)	(M/lbs) (5)	(M/\$) (6)	(M/\$) (7)				(M/\$) (11)	(M/\$) (12)	(M/\$) (13)
69	654	795	900	772	294	340	797	248	1679	2300	6830	11400
69	654	954	1010	867	330	373	817	248	1768	1950	5800	9650
69	654	1192.5	1160	1080	410	464	837	248	1959	1540	4570	7620
115	387	266.8	460	242	91	193	868	310	1462	2500	7430	12350
115	387	636	780	619	235	303	878	310	1726	1050	3120	5200
115	387	795	900	772	294	340	888	310	1832	840	2500	4150
115	387	954	1010	867	330	373	908	310	1921	710	2110	3510
115	387	1192.5	1160	1080	410	464	928	310	2112	580	1720	2870
138	322	266.8	460	242	91	193	942	310	1536	1650	4900	8170
138	322	477	670	464	176	265	952	310	1705	930	2760	4600
138	322	636	780	619	235	303	962	310	1810	690	2030	3420
138	322	795	900	772	294	340	972	310	1916	550	1630	2720
138	322	954	1010	867	330	373	992	310	2005	462	1370	2290
138	322	1192.5	1160	1080	410	464	1012	310	2196	371	1100	1840
161	276	397.5	590	386	147	255	1084	341	1827	800	2370	3960
161	276	556.5	730	540	205	286	1094	341	1926	570	1690	2820
161	276	954	1010	867	330	373	1125	341	2169	335	995	1660
161	276	1192.5	1160	1080	410	464	1155	341	2365	269	798	1330
230	194	666.6	800	606	230	291	1182	388	2091	240	713	1190
230	194	954	1010	867	330	373	1215	388	2306	167	496	826

ES	PRESENT WORTH		TOTAL PRESENT WORTH OF LINE UNDER ASSUMPTION			
	Interim Replact	O&M	A	B	C	D
(M/\$) (14)	(M/\$) (15)	(M/\$) (16)	(M/\$) (17)	(M/\$) (18)	(M/\$) (19)	(M/\$) (20)
8850	68	853	4900	9430	14000	11480
7530	71	853	4642	8492	12342	10222
5940	80	853	4432	7462	10512	8832
9650	56	878	4886	9816	14736	12036
4050	68	878	3722	5792	7872	6722
3240	72	878	3622	5282	6932	6022
2740	76	878	3585	4935	6385	5615
2240	82	878	3652	4792	5942	5312
6370	60	904	4150	7400	10670	8870
3590	67	904	3604	5434	7274	6264
2660	72	904	3476	4836	6206	5446
2120	77	904	3447	4527	5617	5017
1790	80	904	3451	4359	5279	4779
1430	89	904	3560	4289	5029	4619
3090	72	928	3627	5197	6787	5917
2200	76	928	3500	4620	5750	5130
1290	88	928	3520	4180	4845	4475
1030	96	928	3658	4187	4719	4419
926	82	953	3366	3839	4316	4052
645	92	953	3318	3847	4177	3996

TABLE 1

23

EXPLANATION OF TABLE 2

ECONOMIC ANALYSIS -- SUBSTATIONS

Prices on substation equipment were obtained from manufacturers catalogs and manufacturers representatives. Current prices on complete switchyard bays at different voltages were obtained from the Estimating Section of the Denver office of the Bureau of Reclamation. This tabulation is not a detailed cost estimate but primarily a comparison study with items being included which change in cost with changes in voltage. The following paragraph numbers correspond to the line numbers on the table:

JUNEAU SUBSTATION:

- (1) Four 69kv Bays with OCB's = Four bays are required only for the case where incoming line is 69kv since a bay will be required for the incoming line. The Bureau's figure of \$64,000 per bay has been escalated 25 percent for Alaskan conditions, 10 percent for overhead, 5 percent for freight and 10 percent for contingencies, for a total of 50 percent.
- (2) Three 69kv Bays with OCB's = At 115kv and above, the incoming line would initially be run directly to the step down transformer as shown on Figure 2, "Switching Diagram." This, then requires only three OCB bays. These figures are Bureau estimates escalated by 50 percent.
- (3) One 69kv Bay with one disconnect = This transfer bay is required in all schemes to allow switching of load from main bus to transfer bus for maintenance purposes. Cost of OCB and two disconnect switches was deducted from above full bay prices and 50 percent added to arrive at these figures.
- (4) One disconnect switch with grounding blade = This switch is required only on schemes above 69kv. The figures include escalated cost of the switch plus \$10,000 to cover cost of switch supporting structure and foundation.
- (5) Three 23kv Bays with OCB's = The Bureau's figure of \$39,000 per bay has been escalated by 50 percent.
- (6) One 23kv Bay with one disconnect = Catalog cost of one OCB and two disconnect switches has been deducted from the Bureau's figure of \$39,000 and remainder escalated by 50 percent.
- (7) One 69-23kv Transformer = This 9/12/15MVA transformer would be required only in the case where transmission line is 69kv. Catalog price of \$46,000 includes 200kv low side BIL, western zone delivery, bushing current transformers, lightning arresters and extra creep bushings. Transformer price is escalated 30 percent for freight, overhead and contingencies and \$10,000 added for installation.

TABLE 2 EXPLANATIONS

- (8) Transformer, 24/32/40MVA - These prices were taken from manufacturer's catalogs including the features in 7 above. The transformer price is escalated 30 percent also and \$10,000 added for installation. The 115 and 138kv schemes are priced with autotransformers.
- (9) Building, Control Equipment, etc. - A 40 by 100 foot building is assumed to allow space for a control room, office, warehouse and garage areas. At \$15 per square foot this is \$60,000. An eight panel control board is \$40,000 in purchase price. Accessory equipment, such as station transformers, battery, AC and DC boards, etc. are estimated at \$30,000. Labor on installation of control boards, battery and power boards, etc. is estimated at \$25,000.
- (10) Utilities and Improvement - The cost of fencing, grading, utilities, etc. is estimated at \$40,000.
- (11) Cables, Conduits and Miscellaneous - This item covers cost of minor conduit and cable work and miscellaneous items not covered in 9 above.
- (12) Juneau Substation Total - This is the substation cost with the use of autotransformers on 115 and 138kv.

SNETTISHAM SWITCHYARD:

- (13) Two Transformers-26MVA - These transformer prices are from manufacturer's catalogs and include bushing CT's, high side lightning arresters, western zone delivery and extra creep high side bushings. The figure in this line includes 30 percent escalation plus \$5,000 each for handling to site.
- (14) Two Disconnect Switches - These figures include the catalog prices of the switches escalated by 100 percent.
- (15) Switch and Takeoff Structure - These figures cover cost of steel switch support and line takeoff structures.
- (16) Labor, Overhead, Contingencies, Profit - These figures cover costs of above items, including \$10,000 for installation of each transformer. No items on low voltage side of the transformers are included.
- (17) Cable, Conduit, Miscellaneous - These figures include high side bussing, connections to CT's and all control, indication, protection and annunciation required for the transformers.
- (18) Snettisham Switchyard Total - These are total comparative costs of the switchyard.
- (19) Second Juneau Transformer - It is assumed that the second transformer will be required on the 69kv scheme. On the 115 to 230kv plans, the

second transformer is assumed to be paralleled with the first transformer and disconnect switches only provided for isolating purposes to keep down costs. In addition to transformer and disconnect switch costs, \$10,000 has been added for the control, relaying and metering installation.

- (20) Present Worth of 2nd Transformer - This is the above figure present worth to 1969.
- (21) Crater Lake Switchyard Addition - It is assumed that the third unit will be added in 1980, the two year delay being possible by using existing capacity in Juneau, and the fact that camp and station loads do not go through the transformers, the power factor will be near unity and peak loads occur when the ambient temperature is low. The figures are 50 percent of the cost of the initial two unit installation.
- (22) Present Worth of Crater Lake Addition - This figure is the above figure present worth to 1969.
- (23) Present Worth Total - This is the sum of the Juneau Substation, Snettisham Switchyard and present worth costs of the second Juneau Transformer and the Crater Lake Switchyard addition.

TABLE 2

ECONOMIC ANALYSIS - SUBSTATIONS

Item	VOLTAGE (KV)				
	69	115	138	161	230
JUNEAU SUBSTATION					
(1) Four 69kv Bays with OCB's	\$384,000	-	\$288,000	\$288,000	\$288,000
(2) Three 69kv Bays with OCB's	-	\$288,000	\$288,000	\$288,000	\$288,000
(3) One 69kv Bay with disconnect switch only	43,000	43,000	43,000	43,000	43,000
(4) One disconnect switch with ground blade	-	17,000	18,000	19,000	27,000
(5) Three 23kv Bays with OCB's	175,000	175,000	175,000	175,000	175,000
(6) One 23kv Bay with disconnect switch	34,000	34,000	34,000	34,000	34,000
(7) One 69-23kv Transformer	70,000	-	-	-	-
(8) Transformer, 24/52/40 MVA	-	101,000	104,000	168,000	204,000
(9) Building, control, etc.	155,000	155,000	155,000	155,000	155,000
(10) Utilities & Improvement	40,000	40,000	40,000	40,000	40,000
(11) Cables, conduit, miscellaneous	10,000	10,000	10,000	10,000	10,000
(12) JUNEAU SUBTOTAL	\$911,000	\$863,000	\$867,000	\$932,000	\$976,000
SNETTISHAM SWITCHYARD					
(13) Two Transformers (26 MVA)	\$165,200	\$183,000	\$184,000	\$201,000	\$262,000
(14) Two Disconnect Switches	11,600	19,200	22,600	22,600	30,000
(15) Switch and Takeoff Structure	5,000	6,000	7,000	8,000	10,000
(16) Labor, Overhead, Contingencies, profit	30,000	32,000	34,000	36,000	40,000
(17) Cable, Conduit, miscellaneous	15,000	16,000	17,000	18,000	20,000
(18) SNETTISHAM TOTAL	\$226,800	\$256,200	\$264,600	\$285,600	\$362,000
(19) 2nd Juneau Transformer - 1976	\$138,000	\$128,000	\$132,000	\$197,000	\$241,000
(20) Present Worth - 2nd Transformer	111,000	103,000	106,000	159,000	194,000
(21) Crater Lake Switchyard Addition - 1980	114,000	130,000	134,000	144,000	181,000
(22) Present Worth - Switchyard Addition	81,000	93,000	96,000	103,000	129,000
(23) PRESENT WORTH TOTAL	\$1,329,800	\$1,315,200	\$1,333,600	\$1,479,600	\$1,611,000

TABLE 2

TABLE 3

3A or 3AB STRUCTURE COSTS

3AB or 3A Structures - Single Insulator String

All Costs - \$1,000

Length Class	No. Poles	Cost/ Strct \$	Pole Hdw + Labor \$	Pole Hdw Total \$	Excav + Bckfl \$	Excav Total \$	Poles \$	Poles Total \$	Setting \$	Setting Total \$	Guys \$	Guys Total \$
55-1	16	4.303	.200	3.2	.483	7.73	.447	7.16	.180	2.88	1.200	19.20
60-1	8	4.533	.210	1.68	.594	4.74	.516	4.12	.180	1.44	1.230	9.83
65-1	8	4.743	.210	1.68	.687	5.49	.603	4.82	.210	1.68	1.230	9.83
85-1	4	6.343	.230	0.92	1.569	6.27	1.101	4.43	.330	1.32	1.290	5.16

3AB or 3A Structures - Double Insulator Strings

70-1	4	5.466	.220	0.88	.855	3.42	.705	2.82	.240	0.96	1.260	5.03
75-1	4	5.827	.220	0.88	1.058	4.23	.833	3.33	.270	1.08	1.260	5.03
TOTALS	44			\$1.76		\$7.65		\$6.15		\$2.04		\$10.06

anchors \$	Anchors Total \$	INS \$	INS Total \$	INS Hdw + Labor \$	INS Hdw + Labor Total \$
1.400	22.40	.218	3.49	.175	2.80
1.400	11.20	.218	1.74	.185	1.48
1.400	11.20	.218	1.74	.185	1.48
1.400	5.6	.218	0.87	.205	0.82
1.400	5.6	.436	1.74	.350	1.4
1.400	5.6	.436	1.74	.350	1.4
	\$11.20		\$3.48		\$2.80

GRAND TOTAL = \$213,540

TABLE 3

TABLE 4

3T STRUCTURE COSTS

3T Structure - Single Insulator String

All Costs - \$1,000

Length Class Poles	No. Strct	Cost/ Strct \$	Pole Hdw + Labor \$	Pole Hdw Total \$	XArms Labor + Mat'l \$	XArms Total \$	Excav + Bckfl \$	Excav Total \$	Poles \$	Poles Total \$	Settings \$	Sttn Total \$
50-1	4	5.123	.216	.86	.370	1.48	.410	1.64	.371	1.48	.150	.60
55-1	4	5.302	.216	.86	.370	1.48	.483	1.93	.447	1.79	.180	.72
70-1	12	6.612	.226	2.72	.380	4.56	.795	9.55	.705	8.46	.240	2.88
70-1	8	7.071	.336	2.68	.380	3.04	.855	6.83	.705	5.63	.240	1.92
<u>3T Structure - Double Insulator String</u>												
90-1	4	9.101	.346	1.38	.400	1.6	1.935	7.83	1.155	4.61	.360	1.44
TOTALS	32			\$8.50		\$12.16		\$27.78		\$21.97		\$7.56

XBrace \$	XBrce Total \$	SGuys \$	SGuys Total \$	Anch \$	Anch Total \$	INS \$	INS Total \$	INS Hdw + Lab \$	INS Hdw Total \$
.290	1.16	.880	3.52	1.400	5.6	.610	2.44	.426	1.7
.290	1.16	.880	3.52	1.400	5.6	.610	2.44	.426	1.7
.290	3.48	.920	11.05	1.400	16.8	.610	7.32	.446	5.35

			Db1 Guys	DGuy Total					
.290	2.32	1.260	10.01	1.400	11.2	.953	7.62	.652	5.21
.580	2.32	1.320	5.28	1.400	5.6	.953	3.81	.652	2.61

\$10.44	\$33.38	\$44.80	\$23.63	\$16.57
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GRANT TOTAL \$206,790

TABLE 4

TABLE 5

HS STRUCTURE COSTS

ALL COSTS \$1,000

Length Class	No. Strct	Cost/ Strct \$	Poles Hdw & Labor \$	Xarms Labor + Mat'l \$	Xarms Total \$	Excav Bckfl \$	Excav Total \$	Poles \$	Poles Total \$	Sett \$
45-1	35	1.520	5.95	.217	7.6	.259	9.08	.205	7.05	.1
50-1	20	1.580	3.4	.217	4.34	.273	5.46	.245	4.90	.1
55-1	47	1.700	8.00	.217	10.20	.321	15.1	.298	14.0	.1
60-1	75	1.850	12.75	.227	17.00	.396	29.6	.344	25.8	.1
65-1	35	1.990	5.95	.227	7.93	.458	16.0	.402	14.1	.1
70-1	24	2.220	4.08	.237	5.69	.530	12.7	.470	11.3	.1
75-1	16	2.600	2.72	.237	3.79	.705	11.25	.555	8.88	.1
85-1	8	3.190	1.36	.247	1.97	1.046	8.37	.734	5.87	.2
90-1	4	3.520	0.68	.257	1.03	1.290	5.16	.770	3.08	.2
TOTALS	264		\$44.89		\$59.55		\$112.72		\$94.98	

s	Settings Total \$	XBraces \$	XBraces Total \$	INS \$	INS Total \$
	3.50	.145	5.07	.426	14.9
	2.0	.145	2.9	.426	8.52
	5.63	.145	6.81	.426	20.00
	9.0	.145	10.85	.436	32.70
	4.9	.145	5.07	.436	15.30
	3.84	.145	3.48	.446	10.70
	2.88	.290	4.64	.446	7.14
	1.76	.290	2.32	.456	3.65
	0.96	.290	1.16	.466	1.78
	\$34.47		\$42.30		\$114.69

GRAND TOTAL = \$503,600

TABLE 5

EXPLANATION OF TABLE 6

ECONOMIC ANALYSIS - SUBMARINE CABLE

The following numbered paragraphs correspond with the column numbers in the table.

- (1) Voltage - Only 115, 138 and 161kv cables were chosen for study. Since the present worth cost of the cheapest 69kv overhead conductor was approximately one million dollars more than the higher voltages under loading assumption A, no further consideration was given to this voltage. The 230kv cable was also excluded because the excessive charging current would require investment in reactive control equipment and the 230kv cable material cost would be at least \$250,000 more than the lower voltage cables.
- (2) Armor - Cable armor types selected for this study were No. 4 AWG aluminum alloy and No. 4 and No. 6 BWG steel wire.
- (3) Conductor Size - Minimum conductor sizes were selected to carry 77MVA or more.
- (4) Material Cost - Cable and accessory terminal equipment costs were received from Phelps-Dodge Copper Products Corp. for 115 and 138kv. Prices on the cable only were also received from them for 161 and 230kv cable. The figures in this column include the cable and terminal equipment costs, \$30,000 for freight to Juneau, \$0.25 per foot for substituting plastic bedding in place of jute, and construction material costs for each terminal as follows:

115kv	-----	\$20,000
138kv	-----	\$22,000
161kv	-----	\$25,000

The terminal price estimates include precast concrete cable trenches, an enclosing building for the potheads to protect them from salt water spray, piping and a transmission line takeoff structure, either integral with the building or separate.

- (5) Labor Cost - The labor cost estimate for constructing the terminals, splicing the cable, laying and testing is as follows:

a. Splicing cable and rereeling	-----	\$ 40,000
b. Reinforcing barge deck, etc.	-----	5,000
c. Barge rental, 1 month	-----	4,000
d. Tug rental from Seattle, 500 H.P., 1 month	-----	20,000
e. Rental for 4 tugs, from Juneau, 2 weeks	-----	25,000
f. Contractor's labor, assume 10 men from outside for 25 days @ \$100/man-day	-----	25,000

TABLE 6 EXPLANATIONS

g. Local labor, assume 20 men for 10 days	
@ \$75/man-day -----	\$ 15,000
h. Additional labor on two terminals -----	<u>26,000</u>
TOTAL	\$ 160,000

Above tug and barge costs are based on estimates received from the Alaska Tug and Barge Company on 24 September 1965.

- (6) Initial Cost - This is the sum of columns 4 and 5.
- (7) Yearly Cost - 30-Year Life - The cable is assumed to have a life of 30 years and the figures in this column represent the yearly capital recovery cost.
- (8) Present Worth on 50-Year Life - In order to compare all costs in this study on a uniform basis, the yearly costs in Column 7 have been converted to a 50-year present worth figure.
- (9) Losses per year-KWH - The cable losses per foot are shown in Table 7, "Submarine Cable Loss Tabulation." The figures in this column include all losses on the three cables which are in active use plus the insulation loss only on the spare cable, which is assumed to be continuously energized. These losses are based on assumption A.
- (10) Losses per year (\$1,000) - These dollar figures are the value of the above KWH losses at 9 mills per KWH.
- (11) Present Worth of Losses (\$1,000) - These figures are the present worth of losses for 50 years with the first 13 years being of no value.
- (12) Total Present Worth of Installation (\$1,000) - These figures are the sums of columns 8 and 11.

TABLE 6 EXPLANATIONS

TABLE 6
ECONOMIC ANALYSIS - SUBMARINE CABLE

Voltage (kv)	Armor	Cond. Size (MCM or AWG)	Mat'l Cost \$1,000	Labor Cost \$1,000	C O S T		
					Initial \$1,000	Yearly 30-yr Life \$1,000	P.W. on 50-yr Life \$1,000
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)
115	ST	4/0	418	160	578	30	754
115	AL	4/0	531.5	160	691.5	35.8	990
115	AL	450	616.5	160	776.5	40.2	1010
115	ST	1000	647.5	160	807.5	41.8	1050
138	ST	250	458	160	618	32	804
138	AL	250	580.5	160	740.5	38.4	965
138	AL	350	617	160	777	40.2	1010
138	ST	550	558	160	718	37.2	935
161	ST	250	509	160	669	34.7	871
161	AL	250	687	160	847	43.9	1110
161	AL	350	730	160	890	46.1	1158
161	AL	450	761	160	921	47.7	1198

Losses per Year (MMII)	Losses per Year \$1,000	P.W. of Losses \$1,000	Total P.W. of Installtn \$1,000
(9)	(10)	(11)	(12)
2,587	23.3	340	1094
1,887	17.0	248	1148
1,355	12.2	178	1188
1,605	14.4	210	1260
1,587	14.3	208	1012
1,324	11.9	173	1138
1,242	11.2	163	1173
1,481	13.3	194	1129
1,360	12.3	179	1050
1,204	10.8	157	1267
1,122	10.1	147	1305
1,098	9.9	144	1342

TABLE 6

EXPLANATION OF TABLE 7

SUBMARINE CABLE LOSS TABULATION

Following constants and formulas were used in calculating submarine cable losses. All losses were calculated based on loading assumption A. Losses on loading assumptions B, C and D can be quickly approximated by using the multiplying factors developed in the overhead section analysis:

Resistances (ohms per circular mil foot)

Copper (100 percent IACS)	- - - - -	10.371
Aluminum (61 percent IACS)	- - - - -	17.0
Aluminum Alloy #5052 (est. 35 percent IACS)	- - -	29.6
Lead (7.84 percent IACS)	- - - - -	132.3
Steel (11.04 percent IACS)	- - - - -	93.8

#4 AWG = 41,740 CM., O.D. = 0.204"

#4/O AWG = 211,600 CM.

#6 BWG = 36,864 CM., O.D. = 0.192"

#4BWG = 53,824 CM., O.D. = 0.232"

Loss formulas, watts per foot and nomenclature from Reference 2

Conductor Loss = W_c

$$W_c = \text{Ave. } I^2 \times 1.09 \times \frac{R_c}{10^6}$$

where R_c = Conductor AC resistance, ohms/cm.ft.
and I = current in amperes

Insulation Loss = W_d

$$W_d = \frac{0.00276 E^2 r \cos}{\log_{10} 2T + \frac{D_c}{D_c}}$$

where E = voltage, line to ground, kv
 r = insulation specific capacitance = 3.5
 \cos = insulation P.F., = 0.006
 T = insulation thickness, inches
 D_c = conductor O.D., inches

Sheath and Armor Loss W_{sa}

$$W_{sa} = \text{ave } I^2 \times 1.09 \times R_{sa}$$

where R_{sa} is sheath and armor parallel resistance, ohms/cm.ft.
losses divide inversely proportional to resistance.

Armor A.C. resistance assumed 50 percent more than R_{dc} including 4 percent cable lay addition.


TABLE 7 EXPLANATIONS

TABLE 7
SUBMARINE CABLE LOSS TABULATION

Volts (kv)	Avg. Curr (amps)	CONDUCTOR				INSULATION			SHEATH					
		Size (MCM)	O.D. (in.)	Res. M Ω /ft.	Losses W _c (w/ft)	Th. (in.)	O.D. (in.)	Losses W _d (w/ft)	Th. (in.)	I.D. (in.)	O.D. (in.)	Area (MCM)	Res. M Ω /ft.	Le W (w/ft)
115	206	4/0	.800	49.0	2.54	.435	1.67	.86	.120	1.68	1.92	870	153.2	1.
115	206	4/0	.800	49.0	2.54	.435	1.67	.86	.120	1.68	1.92	870	153.2	.
115	206	450	.98	23.5	1.09	.435	1.85	1.00	.125	1.86	2.11	1000	132.5	.
115	206	1000	1.503	10.4	.43	.435	2.18	1.24	.130	2.19	2.45	1200	111.1	.
138	168	250	.833	42.3	1.30	.505	1.845	1.07	.125	1.85	2.10	1000	132.5	.
138	168	250	.833	42.3	1.30	.505	1.845	1.07	.125	1.85	2.10	1000	132.5	.
138	168	350	.906	30.3	.933	.505	1.916	1.22	.125	1.92	2.17	1020	130.0	.
138	168	550	1.048	18.9	.581	.505	2.058	1.25	.130	2.07	2.35	1140	116.0	.
161	143	250	.833	42.3	.945	.590	2.013	1.31	.125	2.02	2.27	1000	121.0	.
161	143	250	.833	42.3	.945	.590	2.013	1.31	.125	2.02	2.27	1000	121.0	.
161	143	350	.906	30.3	.676	.590	2.086	1.33	.130	2.09	2.35	1150	115.0	.
161	143	450	.98	23.5	.524	.590	2.160	1.46	.130	2.17	2.43	1190	111.0	.

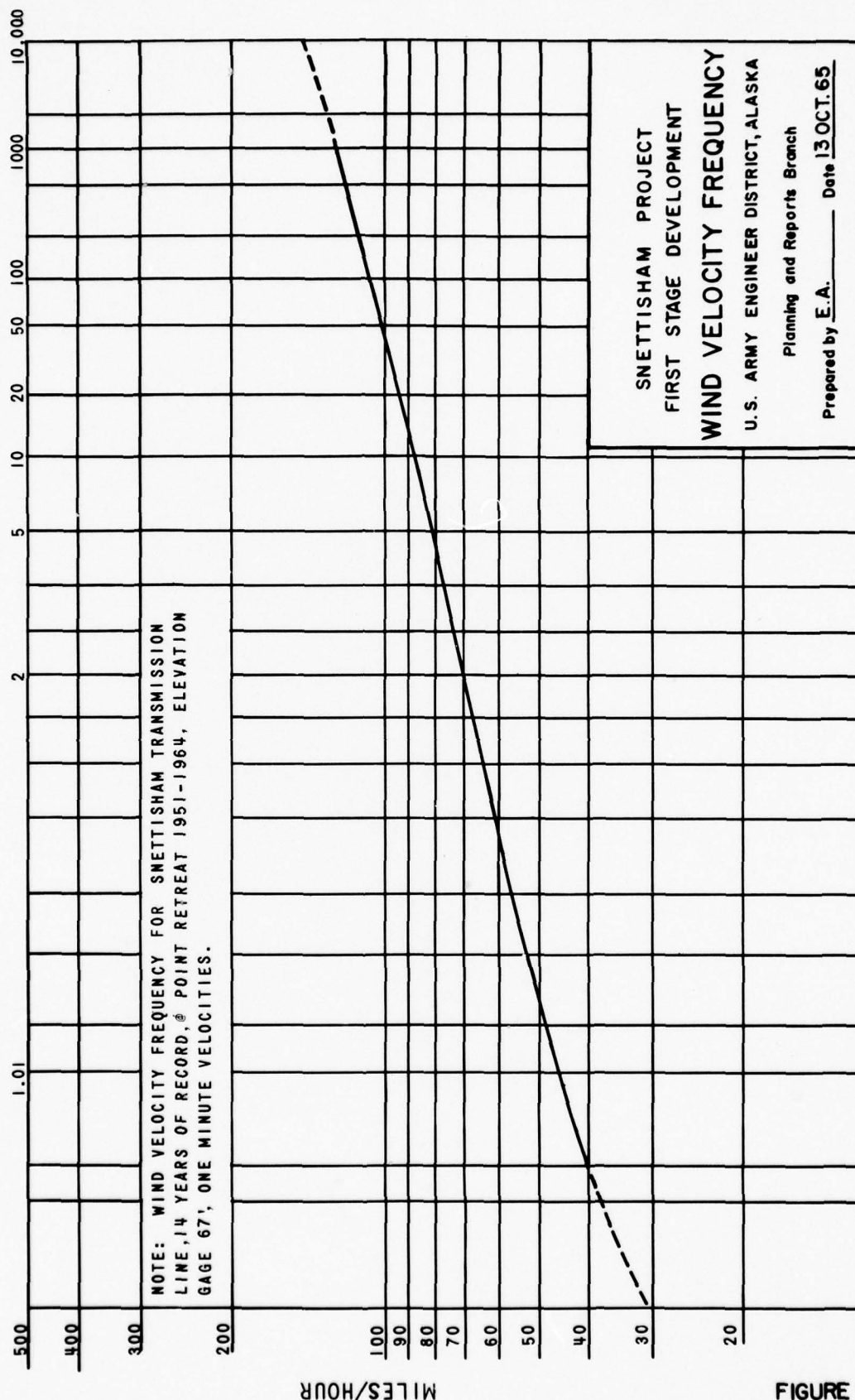
	ANCHOR						TOTAL
t)	Mat or AWG	Size BWG or AWG	No. of Wires	Total Area (MCU)	Res. M ² /ft	Loss w ³ /ft (w/ft)	Loss (w/ft)
	ST	6	30	1107	127	1.76	6.620
8	AL	4	30	1250	35.5	1.077	4.735
13	AL	4	33	1400	30.4	.932	3.233
19	ST	4	33	1777	79.0	1.23	3.860
21	ST	4	33	1770	79.3	.955	3.896
27	AL	4	33	1380	32.2	.643	3.172
28	AL	4	30	1304	29.6	.605	2.896
31	ST	6	30	1330	106	.932	3.534
34	ST	4	33	2040	68.7	.623	3.232
35	AL	4	38	1590	27.9	.412	2.762
39	AL	4	40	1676	26.5	.380	2.535
35	AL	4	42	1755	25.3	.374	2.443

TABLE 7



FIGURES

RECURRENCE INTERVAL IN YEARS



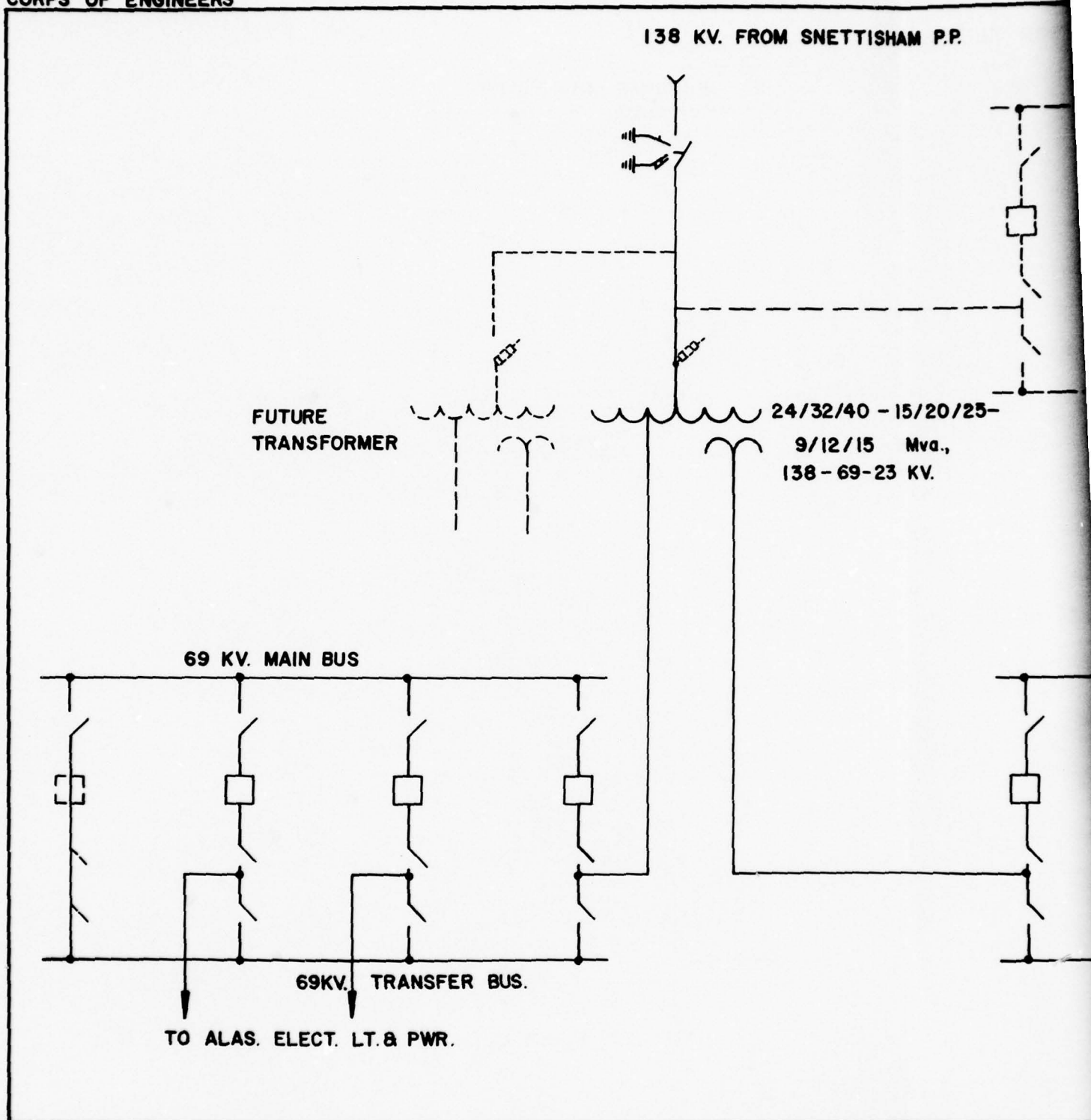
MILES/HOUR

FIGURE 1

SNETTISHAM PROJECT
FIRST STAGE DEVELOPMENT
WIND VELOCITY FREQUENCY
U.S. ARMY ENGINEER DISTRICT, ALASKA
Planning and Reports Branch
Prepared by E.A. Date 13 OCT. 65

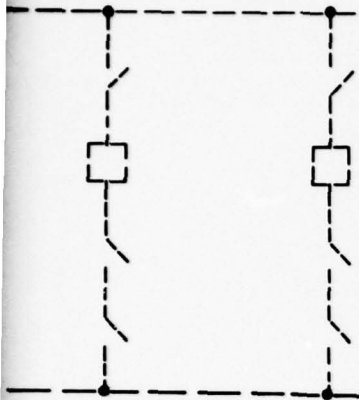
MEMORANDUM NO. 7 APPENDIX B FIGURE 1

CORPS OF ENGINEERS

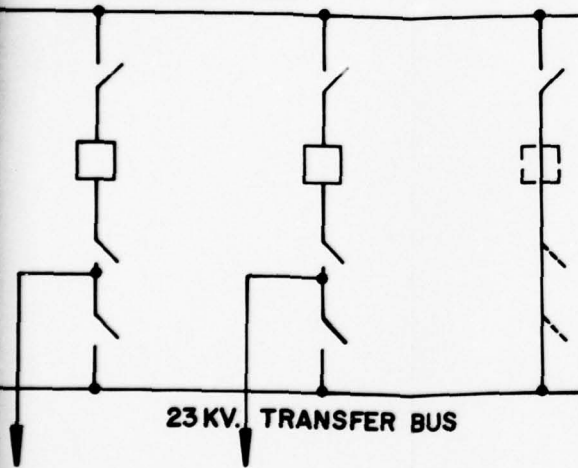


U.S. ARMY

UTURE 138 KV. SWYD.



23 KV. MAIN BUS



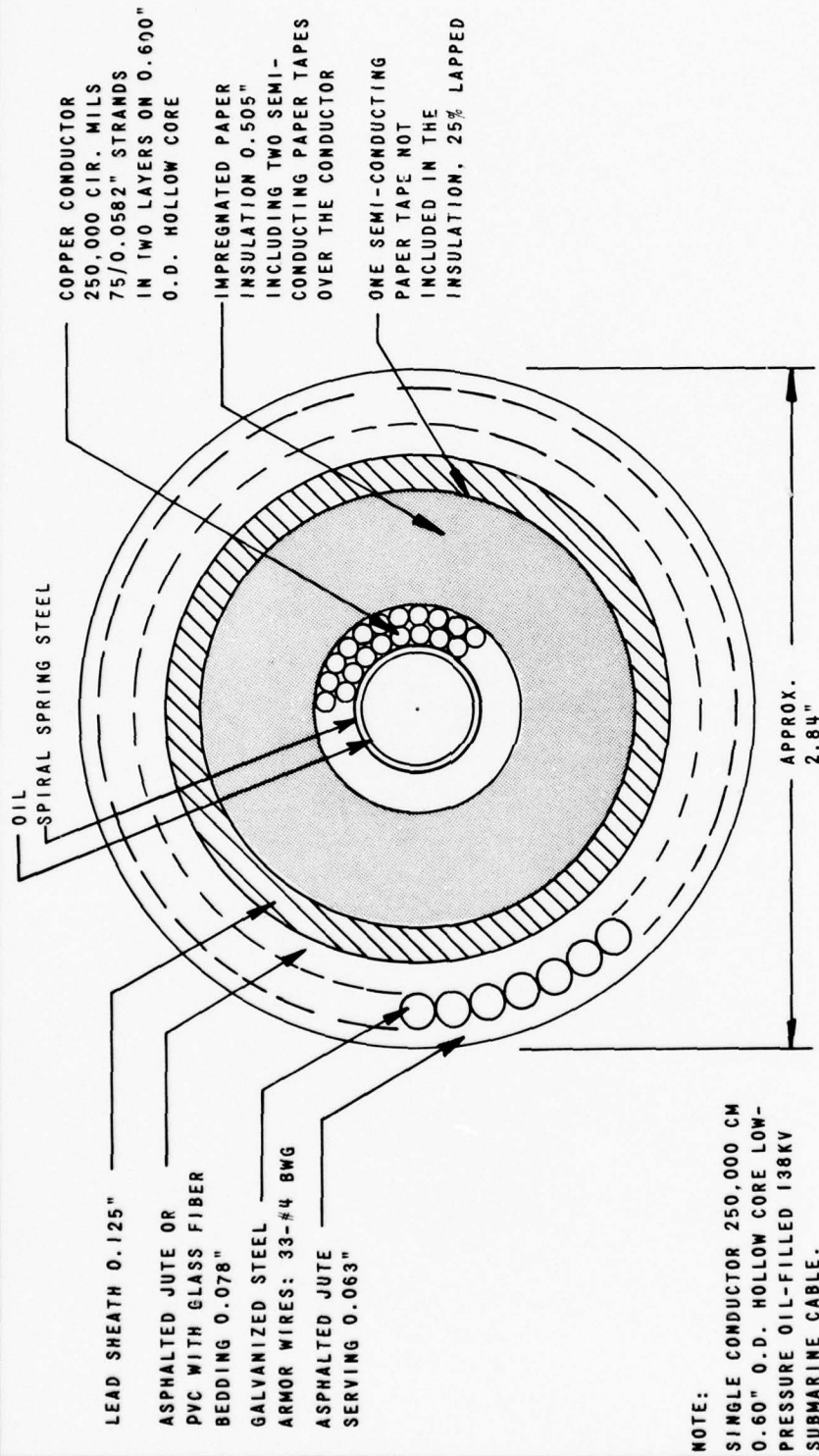
23 KV. TRANSFER BUS

TO ALAS. ELECT. LT. & PWR.

SNETTISHAM PROJECT
FIRST STAGE DEVELOPMENT
JUNEAU SUBSTATION
SWITCHING DIAGRAM
U.S. ARMY ENGINEER DISTRICT, ALASKA
Planning and Reports Branch
Prepared by T. L. Date 7 OCT 65

DESIGN MEMORANDUM NO. 7 APPENDIX B FIGURE 2

2



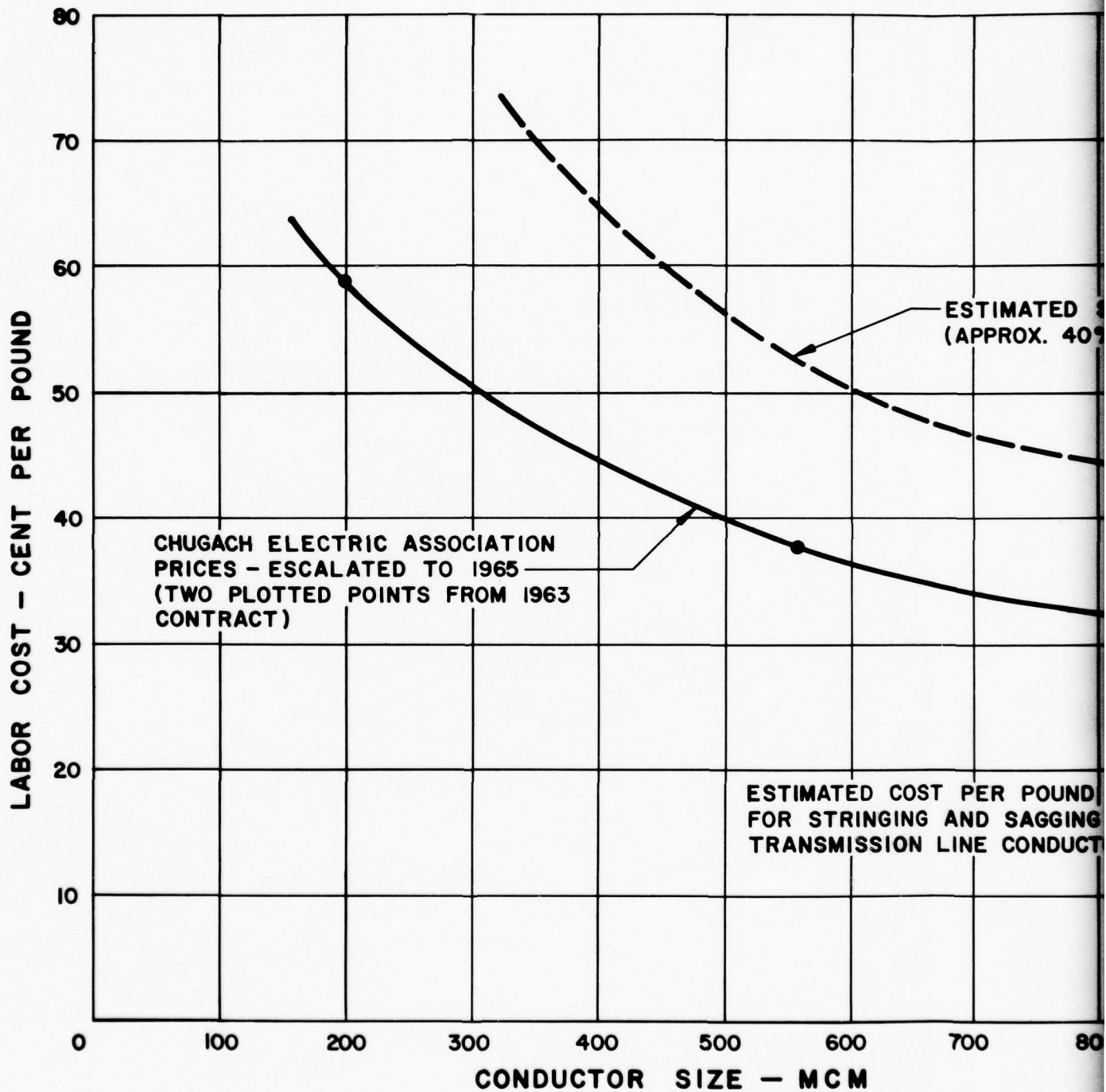
SNETTISHAM PROJECT
FIRST STAGE DEVELOPMENT
**OIL FILLED
SUBMARINE CABLE**

U.S. ARMY ENGINEER DISTRICT, ALASKA
Planning and Reports Branch

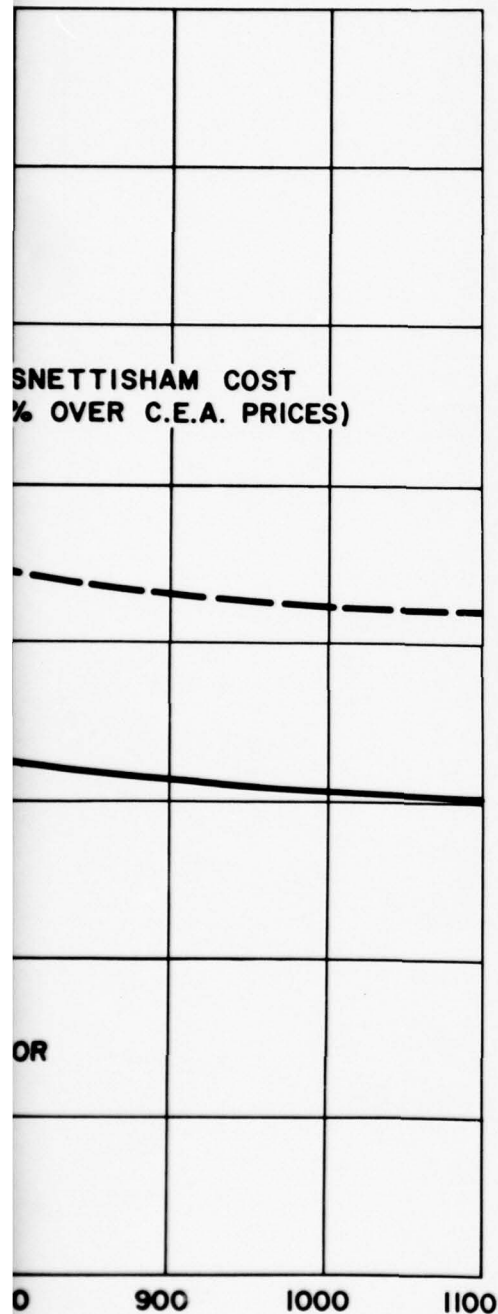
Prepared by E.A. Date 13 OCT 65

FIGURE 3

CORPS OF ENGINEERS



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SNETTISHAM PROJECT
FIRST STAGE DEVELOPMENT
CONDUCTOR LABOR
COST ESTIMATE

U. S. ARMY ENGINEER DISTRICT, ALASKA

Planning and Reports Branch

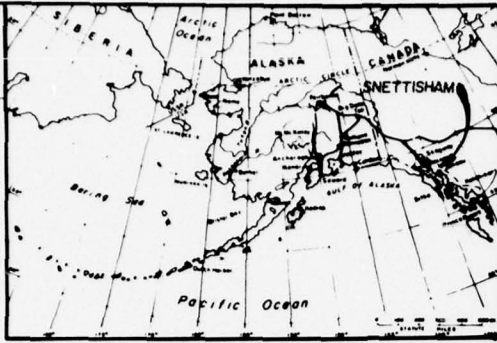
Prepared by T. L. Date 11 OCT 65

MEMORANDUM NO. 7 APPENDIX B FIGURE 5

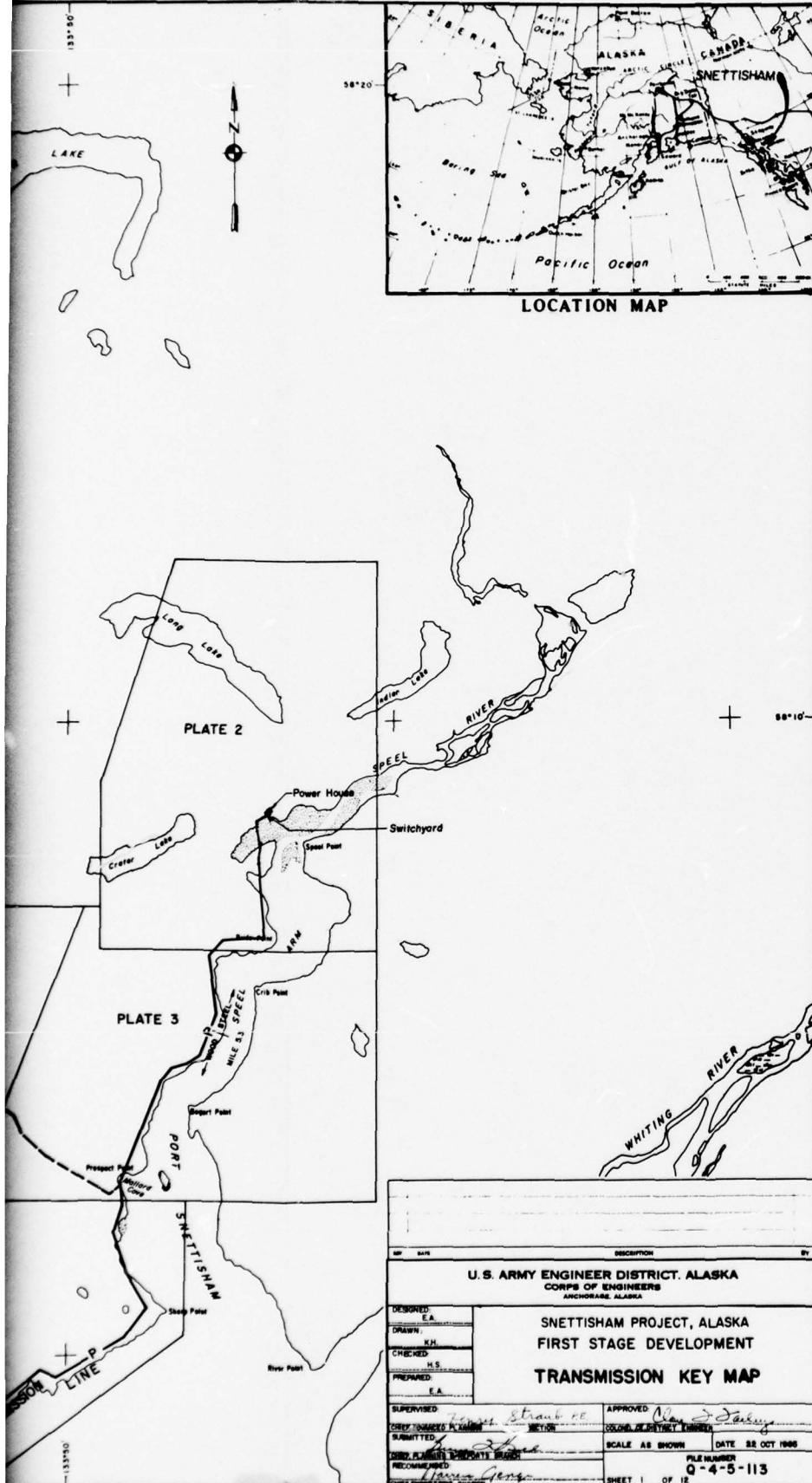
2

PLATES

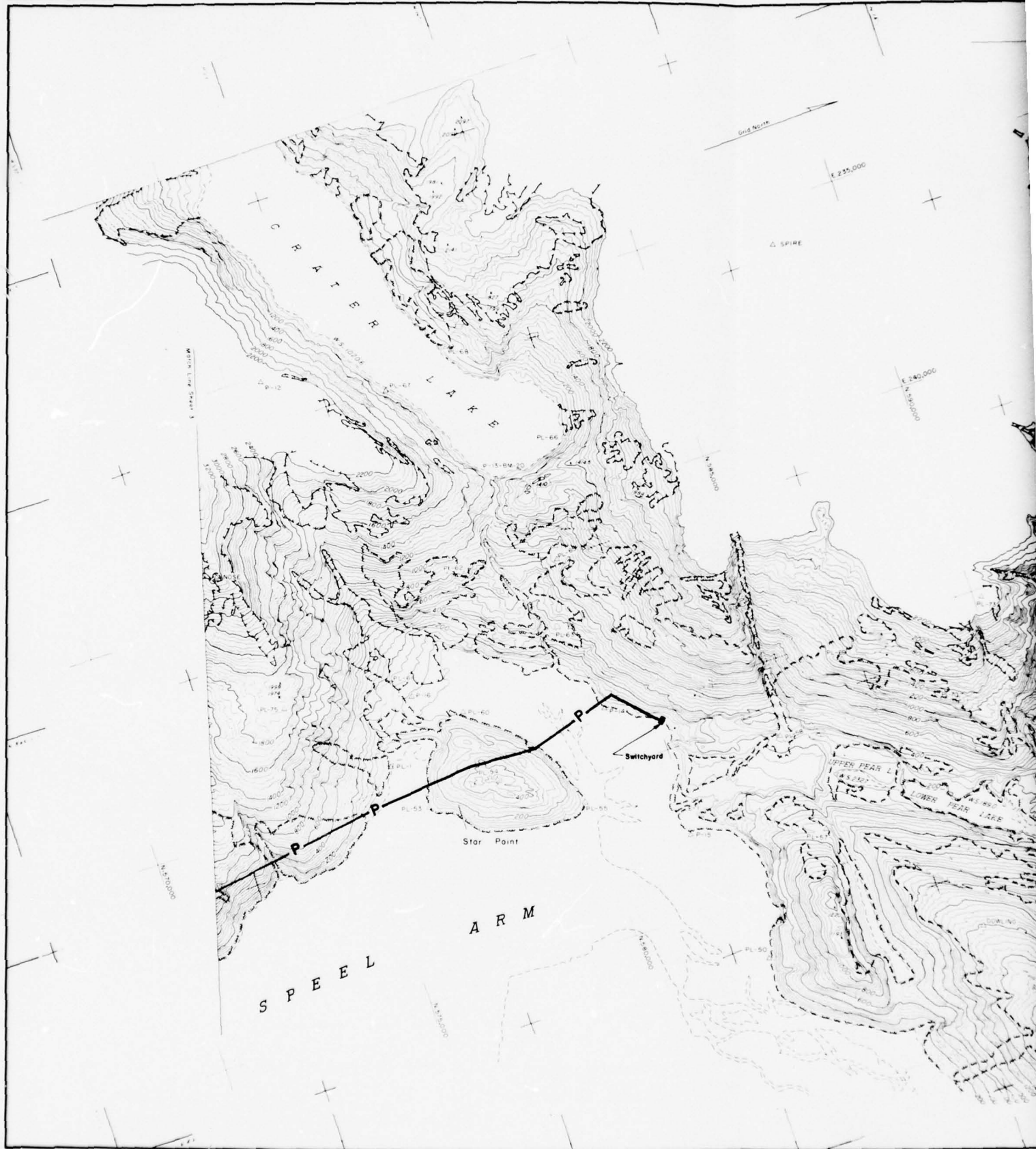
U. S. ARMY



LOCATION MAP



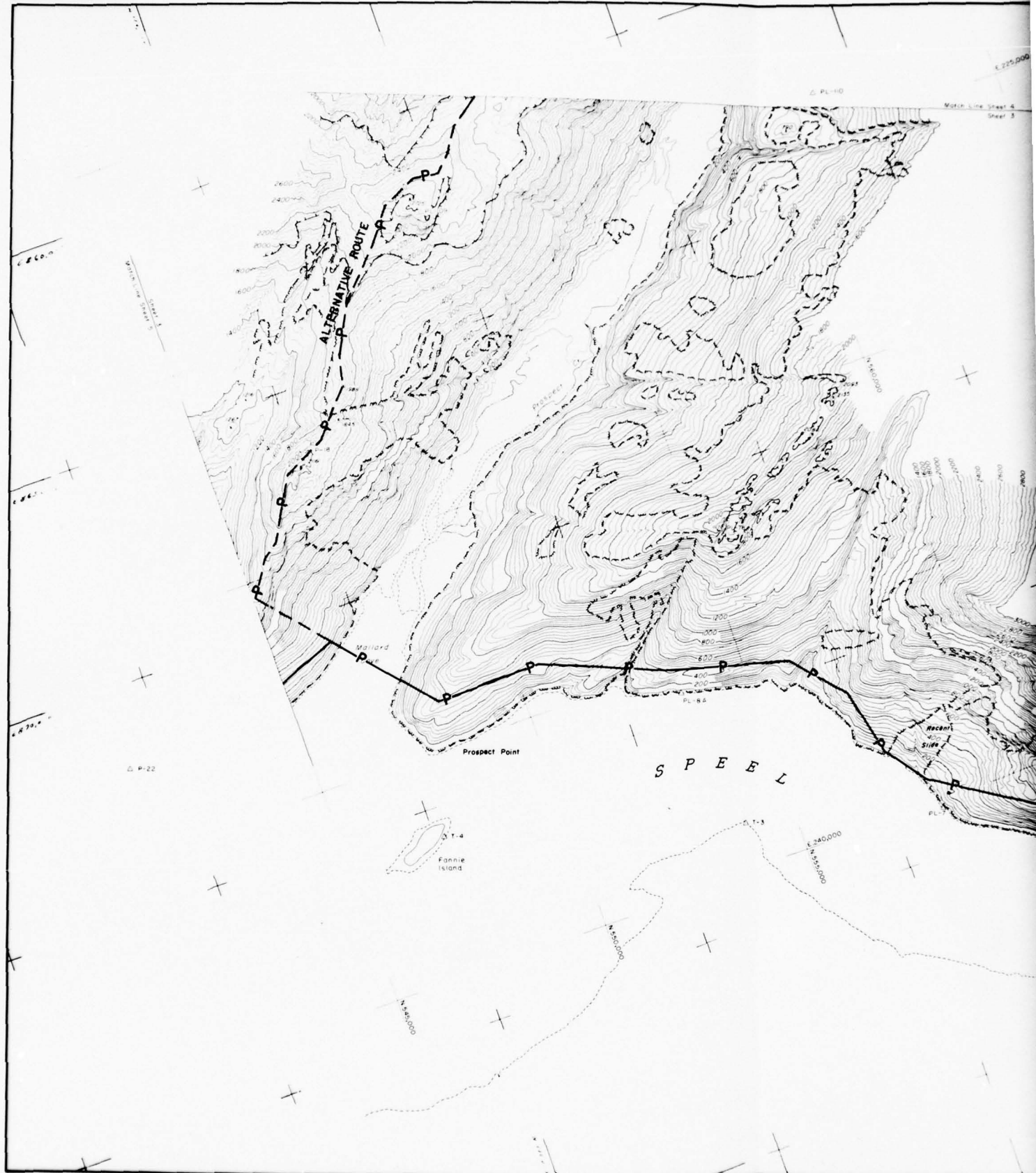
U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: E.A. DRAWN: K.H. CHECKED: H.S. PREPARED: E.A.	
SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT TRANSMISSION KEY MAP	
SUPERVISED: <i>Frank Straub</i> CHECKED: <i>John P. Smith</i> SUBMITTED: <i>John P. Smith</i> RECOMMENDED: <i>John P. Smith</i> CHIEF ENGINEER, DISTRICT	APPROVED: <i>John P. Smith</i> COLONEL, U. S. ARMY SCALE: AS SHOWN DATE: 22 OCT 1966 FILE NUMBER: Q-4-5-113 SHEET 1 OF 12



SCALE IN FEET
0 1000 2000

LEGEND
Woodland
Snow or Ice

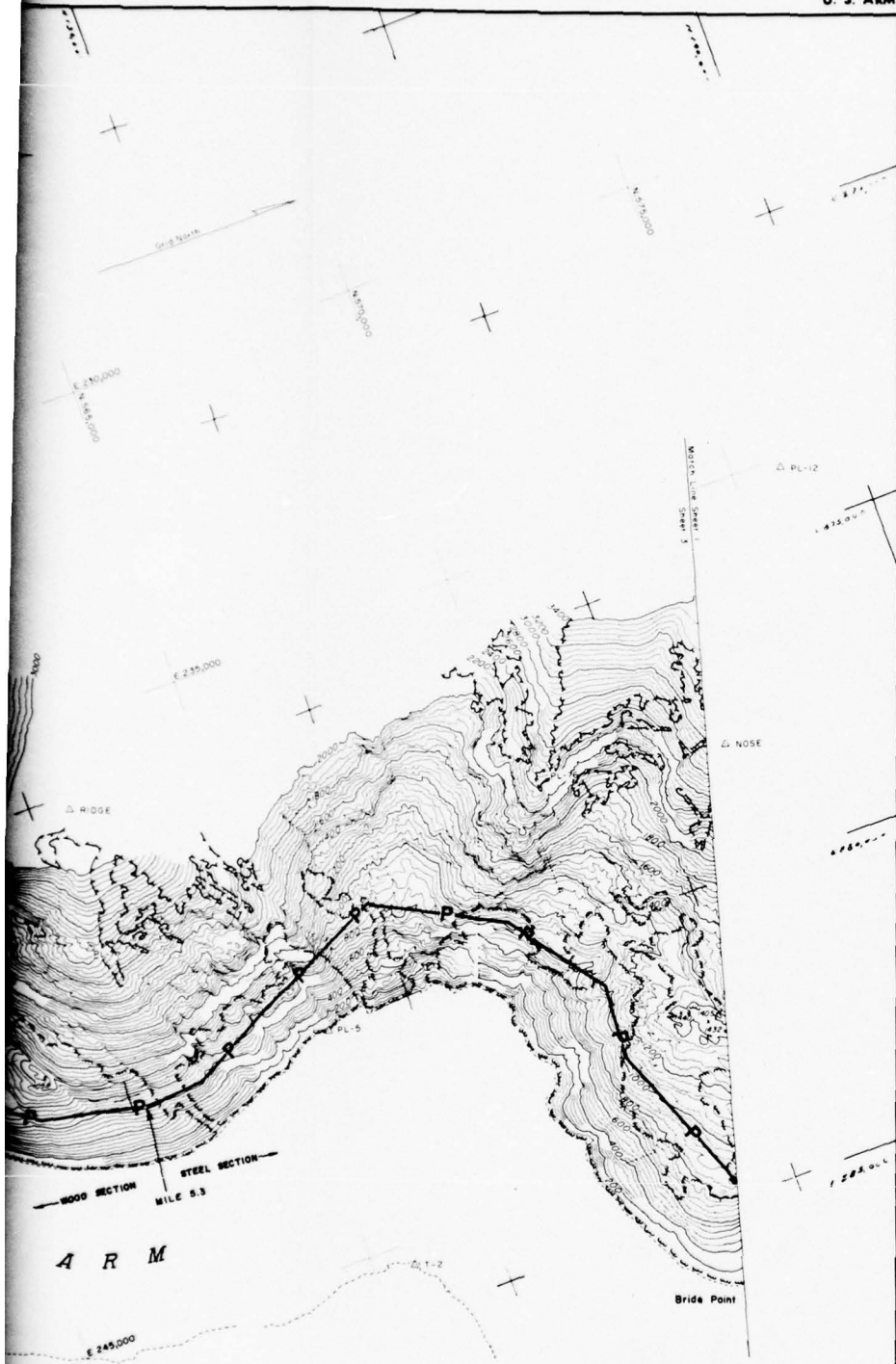




SCALE IN FEET
1000 0 1000 2000

LEGEND
Woodland
Snow or Ice

U. S. ARMY



U. S. ARMY ENGINEER DISTRICT, ALASKA
CORPS OF ENGINEERS
ANCHORAGE, ALASKA

SNETTISHAM PROJECT, ALASKA

FIRST STAGE DEVELOPMENT
TRANSMISSION LINE LOCATION

DESIGNED: E.A.T.L.	SUPERVISOR: <i>Henry Stinson P.E.</i>	APPROVED: <i>Wm. J. Farley</i>	
DRAWN: E.H.		SCALE: AS SHOWN	DATE: 22 OCT 1960
CHECKED: H.S.		FILE NUMBER: Q-4-5-113	
PREPARED: E.A.		SHEET 3 OF 3	
SUBMITTED: <i>James H. Hoad</i>	RECOMMENDED: <i>James H. Hoad</i>		



SCALE IN FEET
1000 0 1000 2000

LEGEND
--- Woodland
--- Snow or Ice

U. S. ARMY



U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: E.A. & T.L.	SNETTISHAM PROJECT, ALASKA
DRAWN: T.L.S.	FIRST STAGE DEVELOPMENT
CHECKED: H.S.	TRANSMISSION LINE LOCATION
PREPARED: E.A.	
SUPERVISED: Henry Strait, PE	APPROVED: [Signature]
SUBMITTED: [Signature]	SCALE AS SHOWN
FILE NUMBER: Q-4-5-113	DATE 22 OCT 1965
RECOMMENDED: [Signature]	SHEET 4 OF 4

DESIGN MEMORANDUM NO. 7 APPENDIX B PLATE 4



SCALE IN FEET
1000 0 1000 2000

LEGEND
--- Woodland
--- Snow or Ice

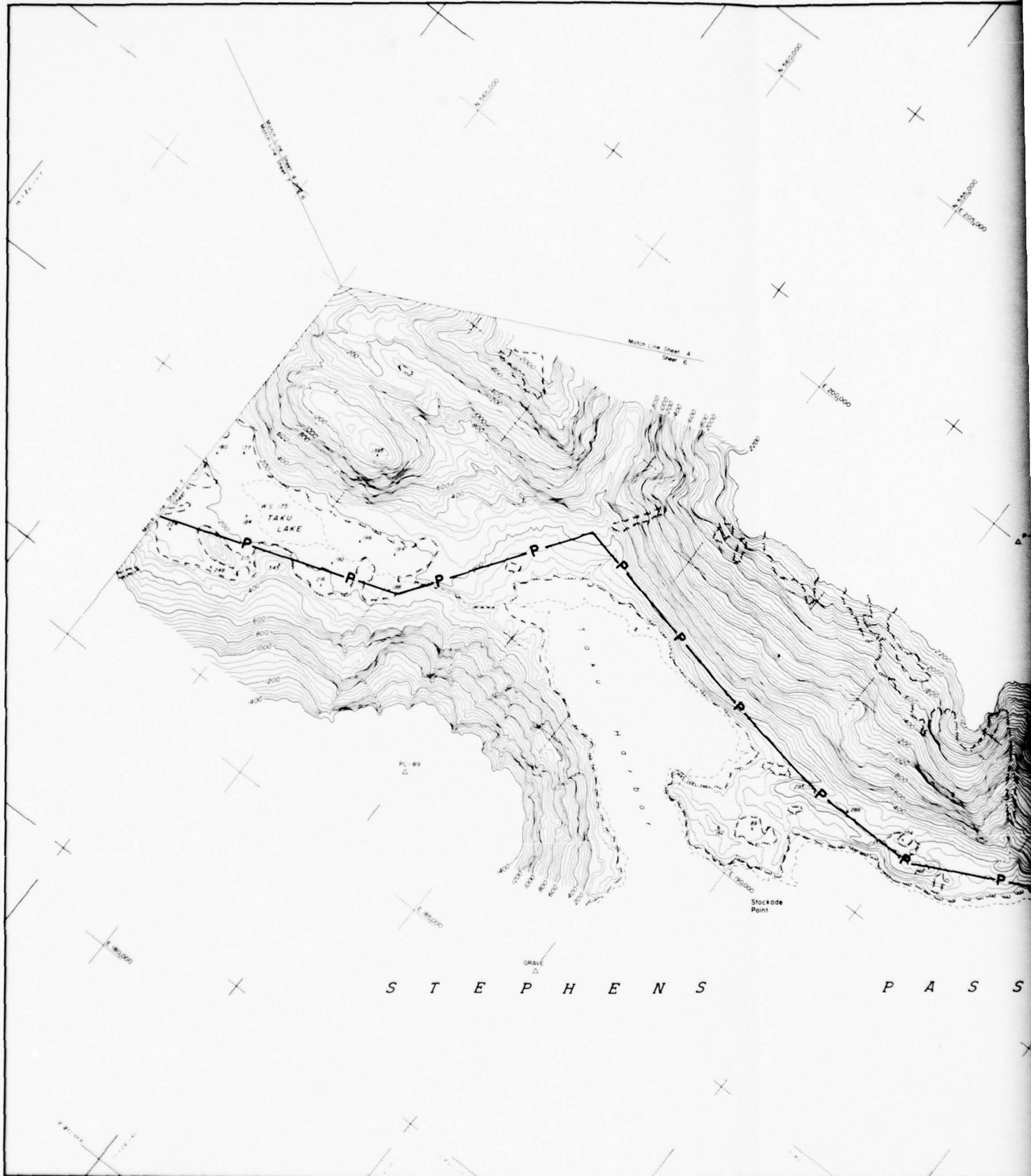
U. S. ARMY



U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED EAST L	SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT TRANSMISSION LINE LOCATION
DRAWN RM	
CHECKED HS	
PREPARED EA	
SUPERVISED JERRY STRICKLAND, PE	APPROVED JERRY STRICKLAND, PE
DESIGNED BY JERRY STRICKLAND, PE	SCALE AS SHOWN
DATE 22 OCT 1965	FILE NUMBER Q-4-5-113
SHEET 3 OF	

DESIGN MEMORANDUM NO. 7 APPENDIX B PLATE 5

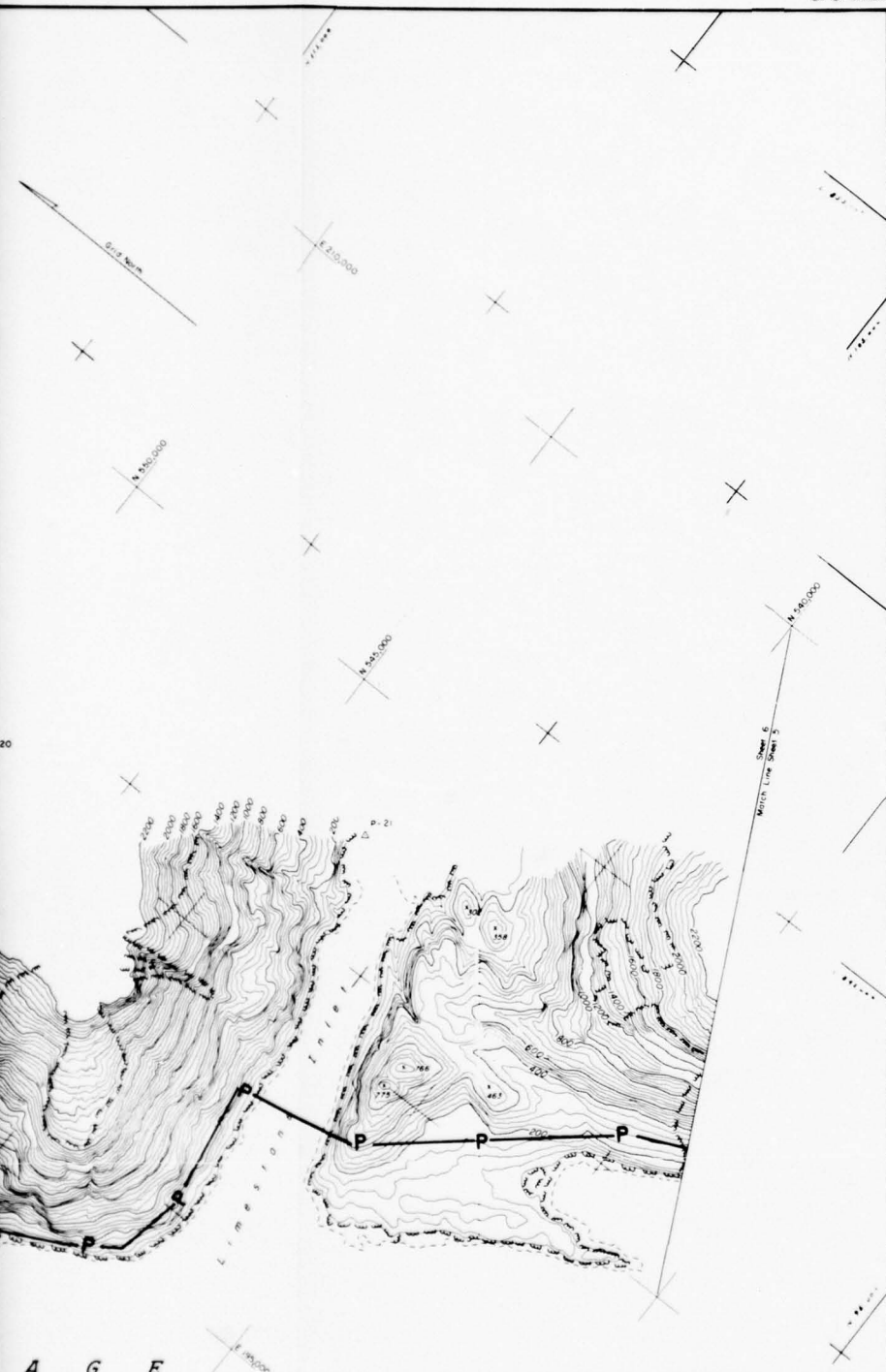
CORPS OF ENGINEERS



SCALE IN FEET
1000 0 1000 2000

LEGEND
Woodland
Snow or Ice

U. S. ARMY



U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
SNETTISHAM PROJECT, ALASKA	
FIRST STAGE DEVELOPMENT	
TRANSMISSION LINE LOCATION	
DESIGNED E.A. 9 TL	APPROVED <i>[Signature]</i>
DRAWN T.L.S.	SCALE AS SHOWN
CHECKED H.S.	DATE 22 OCT. 1965
PREPARED E.A.	FILE NUMBER Q-4-5-113
SUPERVISED <i>[Signature]</i>	SHEET 6 OF 6
RECOMMENDED <i>[Signature]</i>	

DESIGN MEMORANDUM NO. 7 APPENDIX B PLATE 6

This is a detailed topographic map of the Stephens Pass area. The map features a grid system with coordinates labeled as follows:

- Horizontal (East) coordinates: E 200,000, E 195,000, E 190,000, E 185,000, E 180,000.
- Vertical (North) coordinates: N 595,000, N 590,000, N 585,000, N 580,000, N 575,000.

Key geographical features and labels include:

- Contour Lines:** Representing elevation, with labels such as 200, 400, 600, 800, 1000, 1200, 1400, 1600, 1800, 2000, 2200, 2400, 2600, 2800, 3000, 3200, 3400, 3600, 3800, 4000, 4200, 4400, 4600, 4800, 5000, 5200, 5400, 5600, 5800, 6000, 6200, 6400, 6600, 6800, 7000, 7200, 7400, 7600, 7800, 8000, 8200, 8400, 8600, 8800, 9000, 9200, 9400, 9600, 9800, 10000.
- Labels:** "MESA", "Siacum Inlet", "STEPHENS PASS", "P A S S A".
- Grid North:** Indicated by an arrow pointing towards the top of the map.
- Other Markings:** "PL-91", "CIRCU", "TAP-2", "B", "A", "P", "Q", "R", "S", "T", "U", "V", "W", "X", "Y", "Z", "AA", "AB", "AC", "AD", "AE", "AF", "AG", "AH", "AI", "AJ", "AK", "AL", "AM", "AN", "AO", "AP", "AQ", "AR", "AS", "AT", "AU", "AV", "AW", "AX", "AY", "AZ", "BA", "BB", "BC", "BD", "BE", "BF", "BG", "BH", "BI", "BJ", "BK", "BL", "BM", "BN", "BO", "BP", "BQ", "BR", "BS", "BT", "BU", "BV", "BW", "BX", "BY", "BZ", "CA", "CB", "CC", "CD", "CE", "CF", "CG", "CH", "CI", "CJ", "CK", "CL", "CM", "CN", "CO", "CP", "CQ", "CR", "CS", "CT", "CU", "CV", "CW", "CX", "CY", "CZ", "DA", "DB", "DC", "DD", "DE", "DF", "DG", "DH", "DI", "DJ", "DK", "DL", "DM", "DN", "DO", "DP", "DQ", "DR", "DS", "DT", "DU", "DV", "DW", "DX", "DY", "DZ", "EA", "EB", "EC", "ED", "EE", "EF", "EG", "EH", "EI", "EJ", "EK", "EL", "EM", "EN", "EO", "EP", "EQ", "ER", "ES", "ET", "EU", "EV", "EW", "EX", "EY", "EZ", "FA", "FB", "FC", "FD", "FE", "FF", "FG", "FH", "FI", "FJ", "FK", "FL", "FM", "FN", "FO", "FP", "FQ", "FR", "FS", "FT", "FU", "FV", "FW", "FX", "FY", "FZ", "GA", "GB", "GC", "GD", "GE", "GF", "GG", "GH", "GI", "GJ", "GK", "GL", "GM", "GN", "GO", "GP", "GQ", "GR", "GS", "GT", "GU", "GV", "GW", "GX", "GY", "GZ", "HA", "HB", "HC", "HD", "HE", "HF", "HG", "HH", "HI", "HJ", "HK", "HL", "HM", "HN", "HO", "HP", "HQ", "HR", "HS", "HT", "HU", "HV", "HW", "HX", "HY", "HZ", "IA", "IB", "IC", "ID", "IE", "IF", "IG", "IH", "II", "IJ", "IK", "IL", "IM", "IN", "IO", "IP", "IQ", "IR", "IS", "IT", "IU", "IV", "IW", "IX", "IY", "IZ", "JA", "JB", "JC", "JD", "JE", "JF", "JG", "JH", "JI", "JJ", "JK", "JL", "JM", "JN", "JO", "JP", "JQ", "JR", "JS", "JT", "JU", "JV", "JW", "JX", "JY", "JZ", "KA", "KB", "KC", "KD", "KE", "KF", "KG", "KH", "KI", "KJ", "KK", "KL", "KM", "KN", "KO", "KP", "KQ", "KR", "KS", "KT", "KU", "KV", "KW", "KX", "KY", "KZ", "LA", "LB", "LC", "LD", "LE", "LF", "LG", "LH", "LI", "LJ", "LK", "LL", "LM", "LN", "LO", "LP", "LQ", "LR", "LS", "LT", "LU", "LV", "LW", "LX", "LY", "LZ", "MA", "MB", "MC", "MD", "ME", "MF", "MG", "MH", "MI", "MJ", "MK", "ML", "MM", "MN", "MO", "MP", "MQ", "MR", "MS", "MT", "MU", "MV", "MW", "MX", "MY", "MZ", "NA", "NB", "NC", "ND", "NE", "NF", "NG", "NH", "NI", "NJ", "NK", "NL", "NM", "NN", "NO", "NP", "NQ", "NR", "NS", "NT", "NU", "NV", "NW", "NX", "NY", "NZ", "OA", "OB", "OC", "OD", "OE", "OF", "OG", "OH", "OI", "OJ", "OK", "OL", "OM", "ON", "OO", "OP", "OQ", "OR", "OS", "OT", "OU", "OV", "OW", "OX", "OY", "OZ", "PA", "PB", "PC", "PD", "PE", "PF", "PG", "PH", "PI", "PJ", "PK", "PL", "PM", "PN", "PO", "PP", "PQ", "PR", "PS", "PT", "PU", "PV", "PW", "PX", "PY", "PZ", "QA", "QB", "QC", "QD", "QE", "QF", "QG", "QH", "QI", "QJ", "QK", "QL", "QM", "QN", "QO", "QP", "QQ", "QR", "QS", "QT", "QU", "QV", "QW", "QX", "QY", "QZ", "RA", "RB", "RC", "RD", "RE", "RF", "RG", "RH", "RI", "RJ", "RK", "RL", "RM", "RN", "RO", "RP", "RQ", "RR", "RS", "RT", "RU", "RV", "RW", "RX", "RY", "RZ", "SA", "SB", "SC", "SD", "SE", "SF", "SG", "SH", "SI", "SJ", "SK", "SL", "SM", "SN", "SO", "SP", "SQ", "SR", "SS", "ST", "SU", "SV", "SW", "SX", "SY", "SZ", "TA", "TB", "TC", "TD", "TE", "TF", "TG", "TH", "TI", "TJ", "TK", "TL", "TM", "TN", "TO", "TP", "TQ", "TR", "TS", "TT", "TU", "TV", "TW", "TX", "TY", "TZ", "UA", "UB", "UC", "UD", "UE", "UF", "UG", "UH", "UI", "UJ", "UK", "UL", "UM", "UN", "UO", "UP", "UQ", "UR", "US", "UT", "UU", "UV", "UW", "UX", "UY", "UZ", "VA", "VB", "VC", "VD", "VE", "VF", "VG", "VH", "VI", "VJ", "VK", "VL", "VM", "VN", "VO", "VP", "VQ", "VR", "VS", "VT", "VU", "VV", "VW", "VX", "VY", "VZ", "WA", "WB", "WC", "WD", "WE", "WF", "WG", "WH", "WI", "WJ", "WK", "WL", "WM", "WN", "WO", "WP", "WQ", "WR", "WS", "WT", "WU", "WV", "WW", "WX", "WY", "WZ", "XA", "XB", "XC", "XD", "XE", "XF", "XG", "XH", "XI", "XJ", "XK", "XL", "XM", "XN", "XO", "XP", "XQ", "XR", "XS", "XT", "XU", "XV", "XW", "XX", "XY", "XZ", "YA", "YB", "YC", "YD", "YE", "YF", "YG", "YH", "YI", "YJ", "YK", "YL", "YM", "YN", "YO", "YP", "YQ", "YR", "YS", "YT", "YU", "YV", "YW", "YX", "YY", "YZ", "ZA", "ZB", "ZC", "ZD", "ZE", "ZF", "ZG", "ZH", "ZI", "ZJ", "ZK", "ZL", "ZM", "ZN", "ZO", "ZP", "ZQ", "ZR", "ZS", "ZT", "ZU", "ZV", "ZW", "ZX", "ZY", "ZZ".

A horizontal number line with arrows at both ends. It is marked with the numbers -1000, -500, 0, 500, 1000, 1500, and 2000. The distance between each consecutive tick mark is 500 units.

Woodland
Snow or ice

[illegible]

U.S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: EASTL DRAWN: KH CHECKED: HLG PREPARED: EA	SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT TRANSMISSION LINE LOCATION
SUPERVISED: FIELD ENGINEER, ALASKA SUBMITTED: FIELD ENGINEER, ALASKA RECOMMENDED: DISTRICT ENGINEER, ALASKA	APPROVED: DISTRICT ENGINEER, ALASKA SCALE AS SHOWN DATE 22 OCT, 1968 FILE NUMBER Q-4-5-113 SHEET 7 OF

DESIGN MEMORANDUM NO. 7 APPENDIX B PLATE 7

2

CORPS OF ENGINEERS



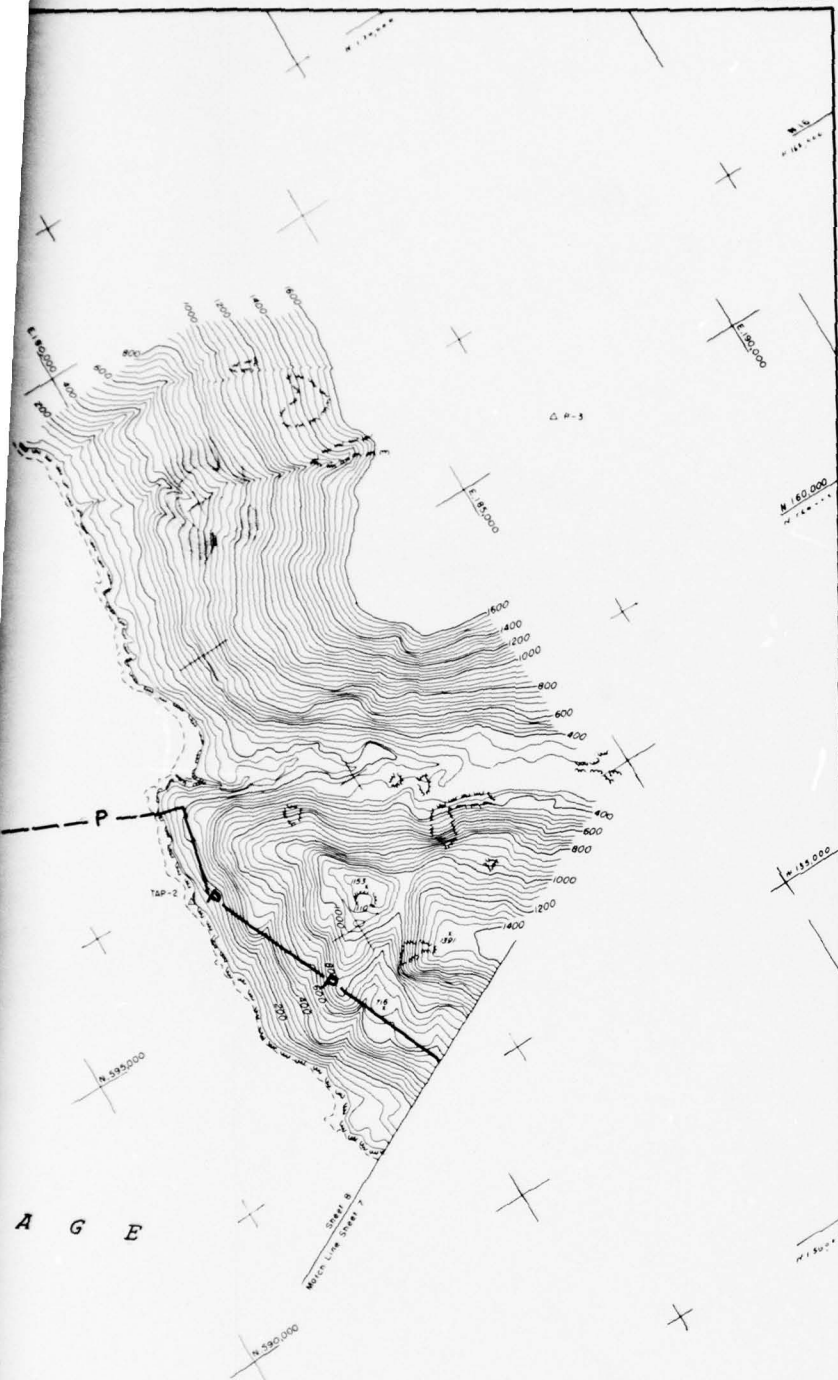
S T E P H E N S

P A S

SCALE IN FEET
1000 0 1000 2000

LEGEND
--- Woodland
--- Snow or Ice

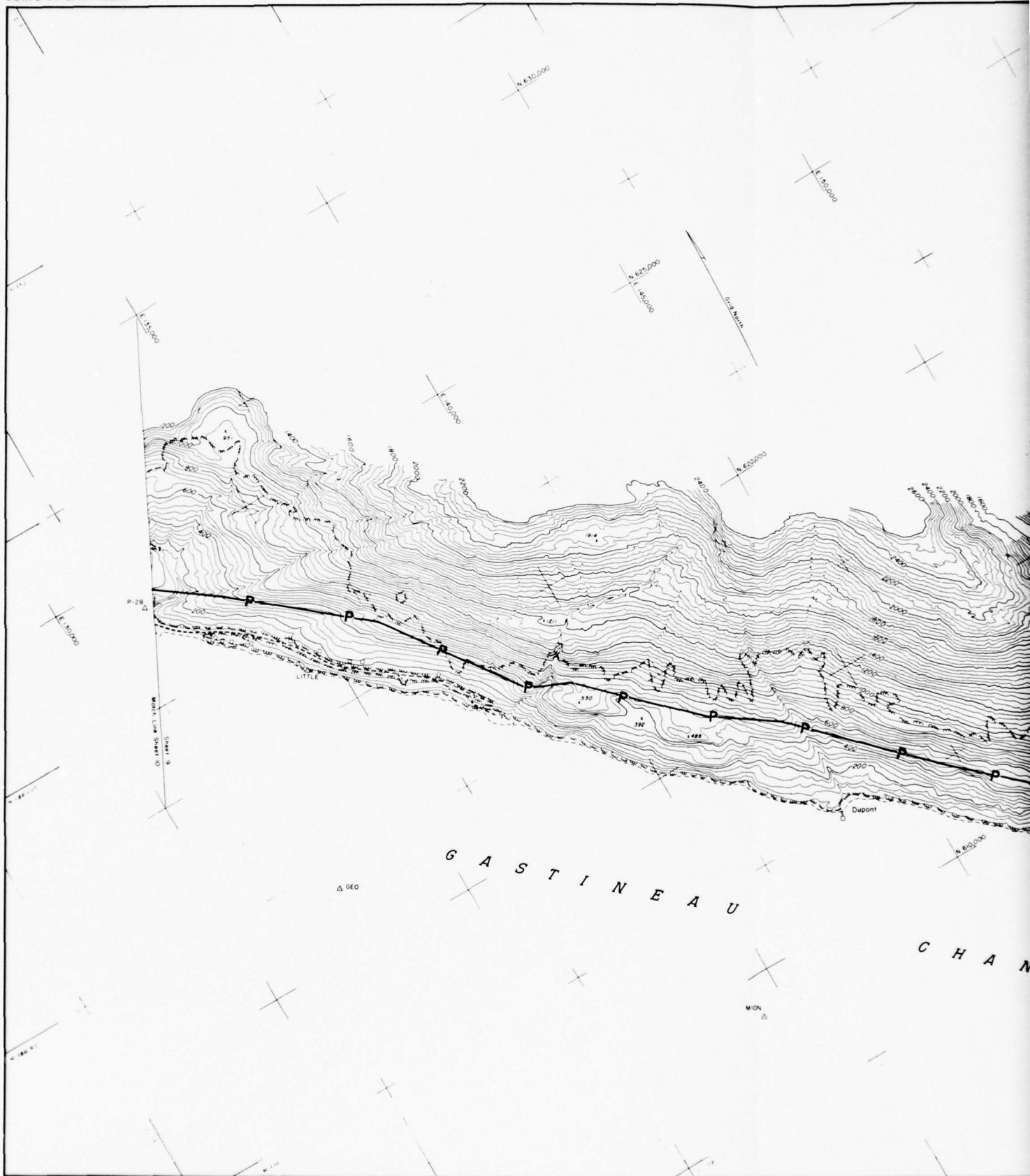
U. S. ARMY



A G E

U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: E. A. T. L.	SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT TRANSMISSION LINE LOCATION
CHECKED: H. S.	
PREPARED: E. A.	
SUPERVISED: Henry Straub, P.E.	
APPROVED: [Signature]	DATE 22 OCT, 1965
SCALE AS SHOWN	FILE NUMBER Q-4-5-113
SHEET 8 OF	

DESIGN MEMORANDUM NO. 7 APPENDIX B PLATE 8



SCALE IN FEET
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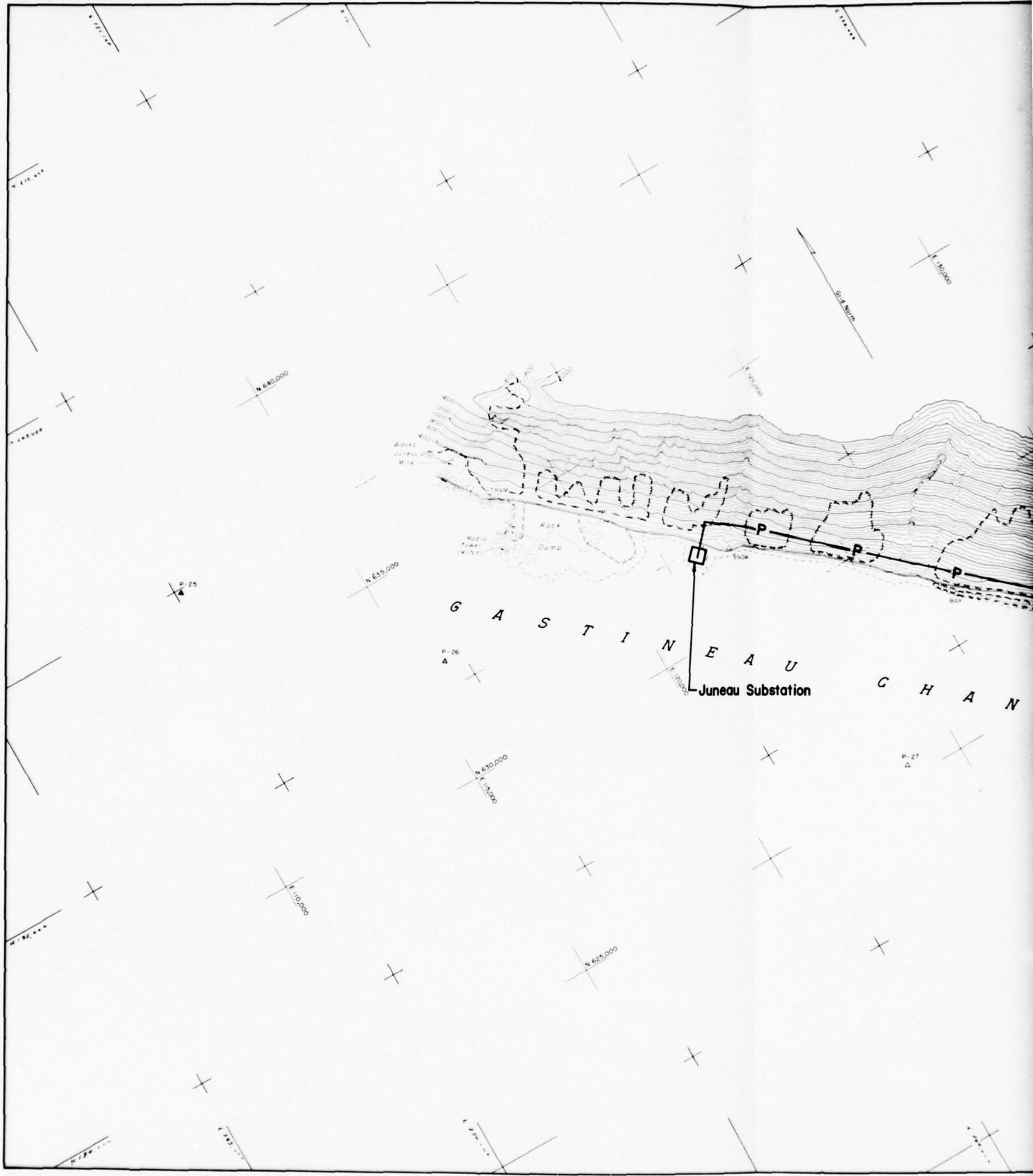
LEGEND
Woodland
Snow or Ice

U. S. ARMY



U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: EA:BTL	SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT TRANSMISSION LINE LOCATION
DRAWN: K.H.	
CHECKED: HS	
PREPARED: EA	
SUPERVISED: Henry Strickland	
APPROVED: [Signature]	DATE: 22 OCT, 1965
SUBMITTED: [Signature]	SCALE AS SHOWN
RECOMMENDED: [Signature]	FILE NUMBER 9-4-5-113
SHEET 9 OF	

DESIGN MEMORANDUM NO. 7 APPENDIX B PLATE 9



SCALE IN FEET
1000 0 1000 2000

LEGEND
Woodland
Snow or Ice

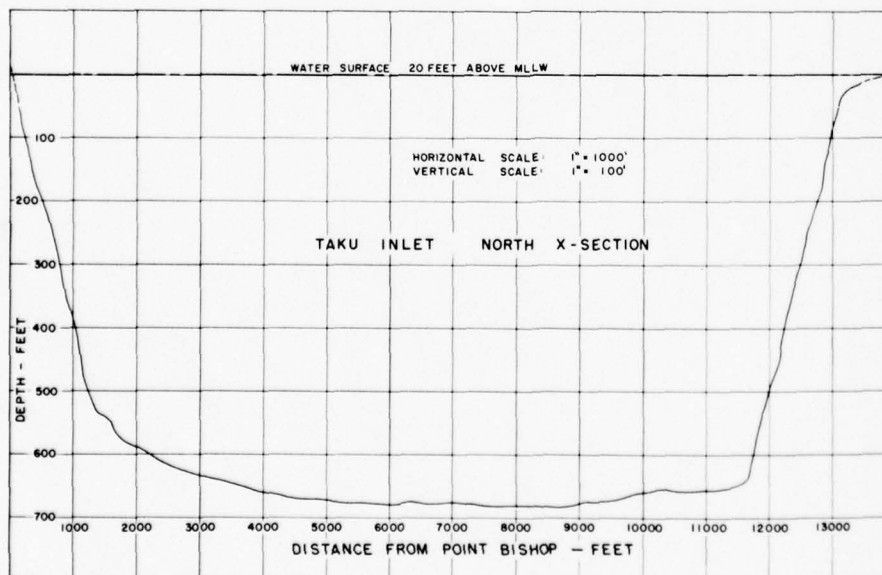
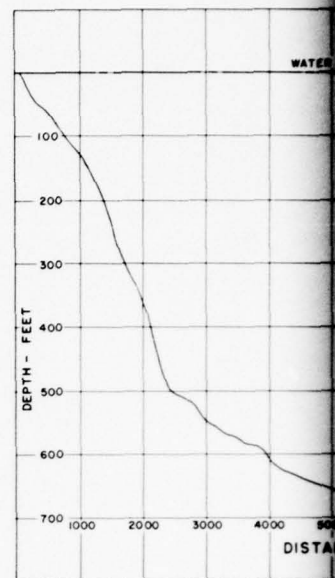
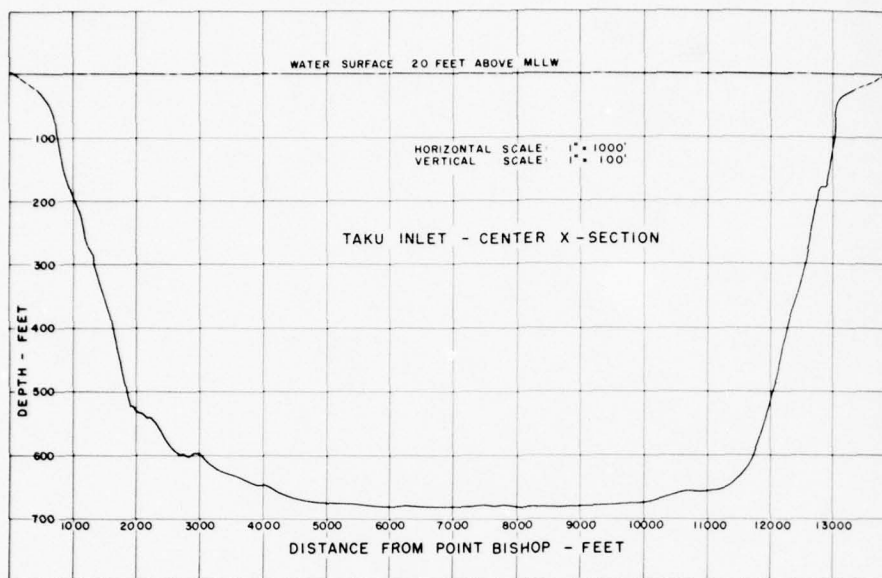
U. S. ARMY

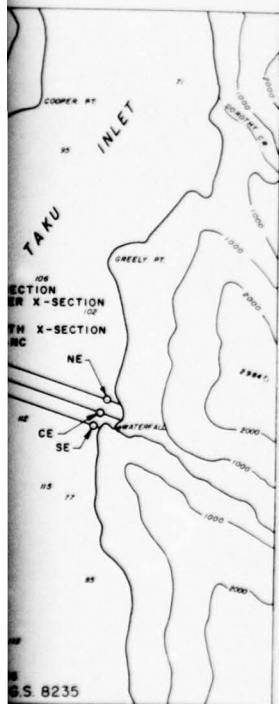
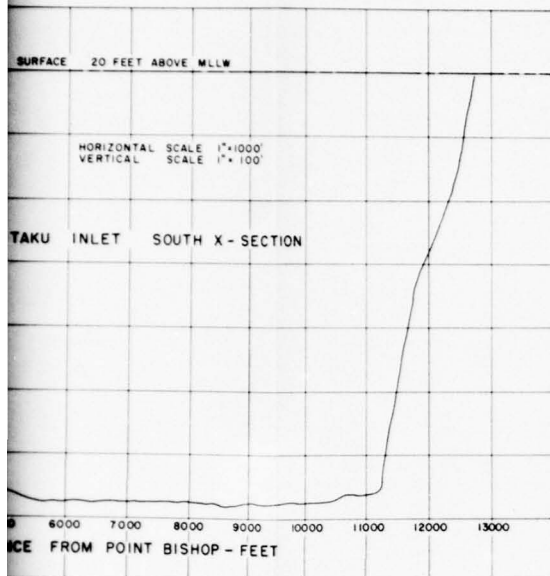


U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
SNETTISHAM PROJECT, ALASKA	
FIRST STAGE DEVELOPMENT	
TRANSMISSION LINE LOCATION	
DESIGNED E. A. S. T. L.	APPROVED: <i>Clayton J. Farley</i>
DRAWN	
TLS	
CHECKED	
HS	
PREPARED	SCALE AS SHOWN
E. A.	
SUPERVISED	
DATE PLANNED	
DATE PLANNED	
RECOMMENDED	DATE 22 OCT, 1965
FILE NUMBER	Q-4-5-113
SHEET 10 OF	

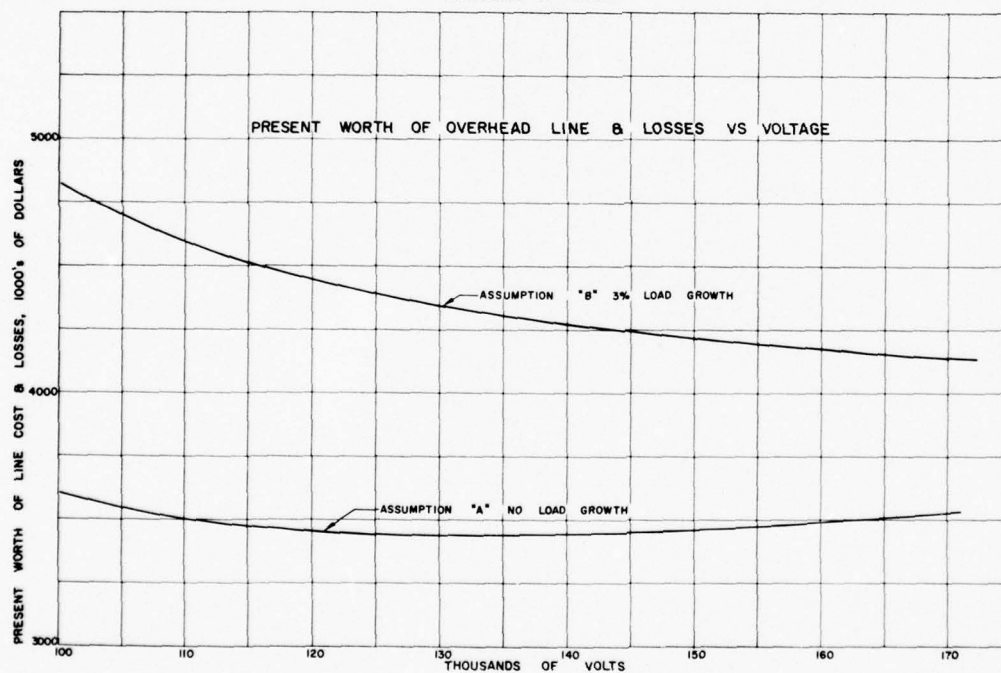
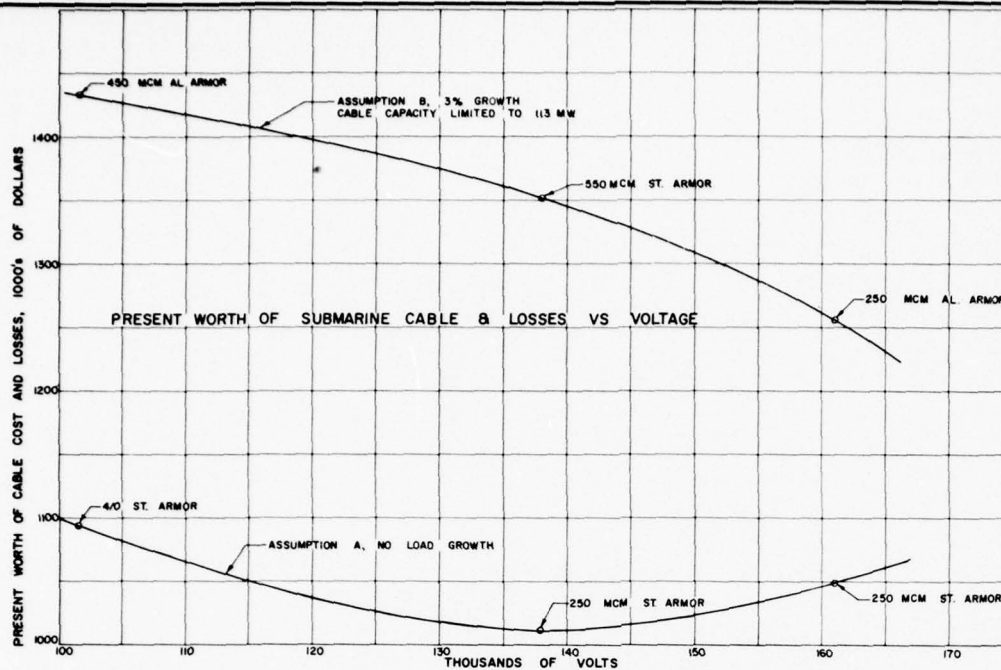
DESIGN MEMORANDUM NO. 7 APPENDIX B PLATE 10

2

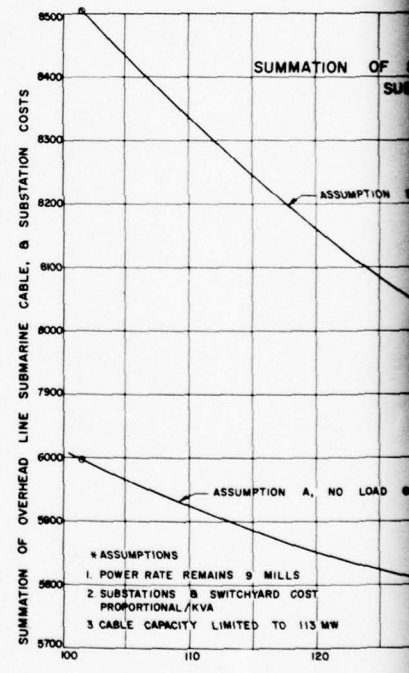
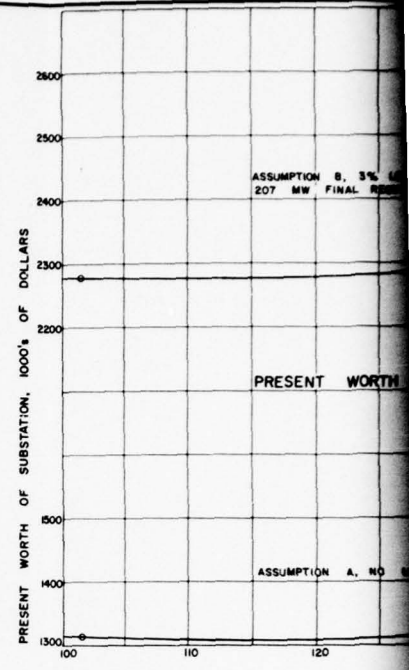


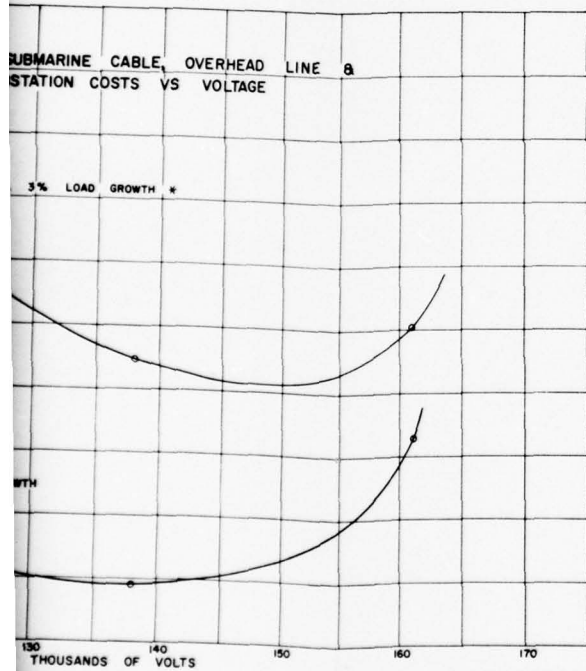
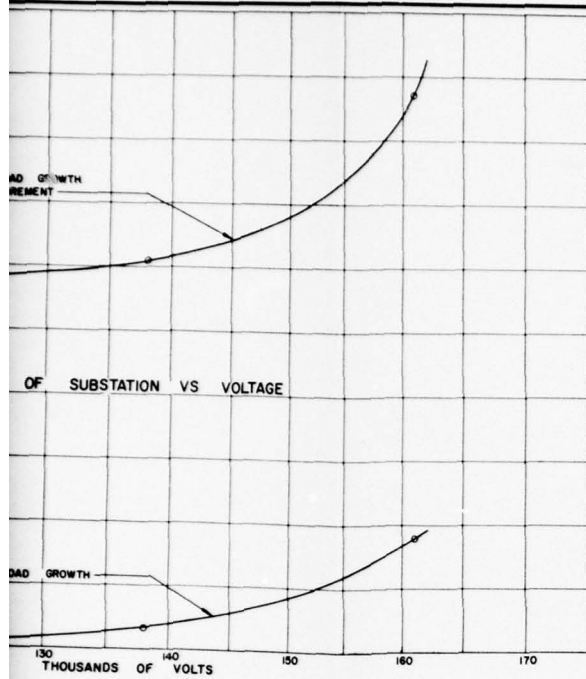


DESIGNED:		U. S. ARMY ENGINEER DISTRICT, ALASKA	
EARTH:		CORPS OF ENGINEERS	
DRAWN:		ANCHORAGE, ALASKA	
P.G.		SNETTISHAM PROJECT, ALASKA	
CHECKED:		FIRST STAGE DEVELOPMENT	
HS		SUBMARINE CABLE PROFILE	
PREPARED:			
E.A.			
SUPERVISED:		APPROVED:	
CHIEF ASSISTANT ENGINEER		CHIEF OF DISTRICT	
SUBMITTED:		SCALE AS SHOWN	
CHIEF ENGINEER		DATE 22 OCT, 1965	
RECOMMENDED:		FILE NUMBER	
CHIEF DISTRICT		Q-4-5-113	
		SHEET 11 OF	



ASSUMPTION "A" - NO LOAD GROWTH AFTER POWER PLANT CAPACITY IS FULLY UTILIZED IN 13 YEARS.
 ASSUMPTION "B" - FULL PLANT CAPACITY REACHED AT 13 YEARS, 3% PER YEAR GROWTH FOR 37 YEARS THERE AFTER. THIS UNREALISTICALLY LOW LOAD GROWTH WAS SELECTED TO ILLUSTRATE THE SHIFT TOWARD HIGHER VOLTAGE WITH GREATER LOAD.





DESIGNED:	SNETTISHAM PROJECT, ALASKA	
DRAWN:	FIRST STAGE DEVELOPMENT	
CHECKED:	COSTS VS VOLTAGE COMPARISONS	
PREPARED:		
TL & EA:		
SUPERVISED:	APPROVED:	
CHIEF, ENGINEER, PLANNING SECTION	COLONEL, DE BRANCH ENGINEER	
SUBMITTED:	SCALE AS SHOWN	
CHIEF, T. & E. SECTION	DATE 22 OCT. 1965	
RECOMMENDED:	FILE NUMBER	
CHIEF, ENGINEERING SECTION	Q-4-5-113	
	SHEET 12 OF	

PHOTOGRAPHS

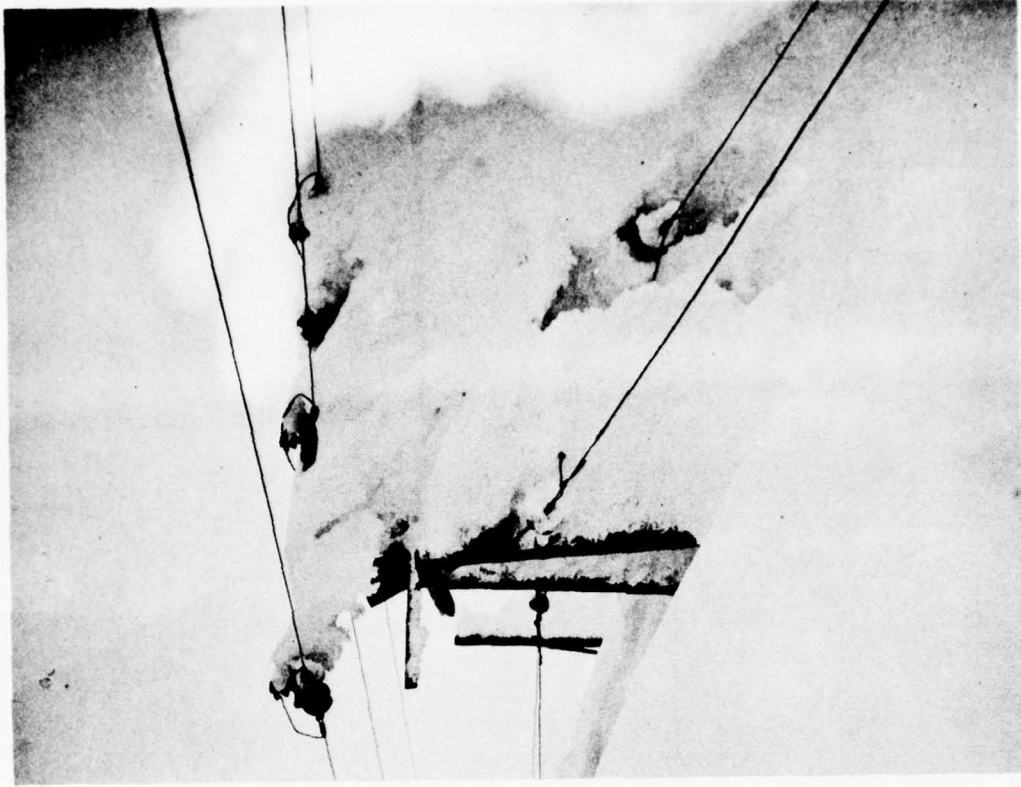


PHOTO # 2 - Snow and Ice formation on A-J Industries
23kv line

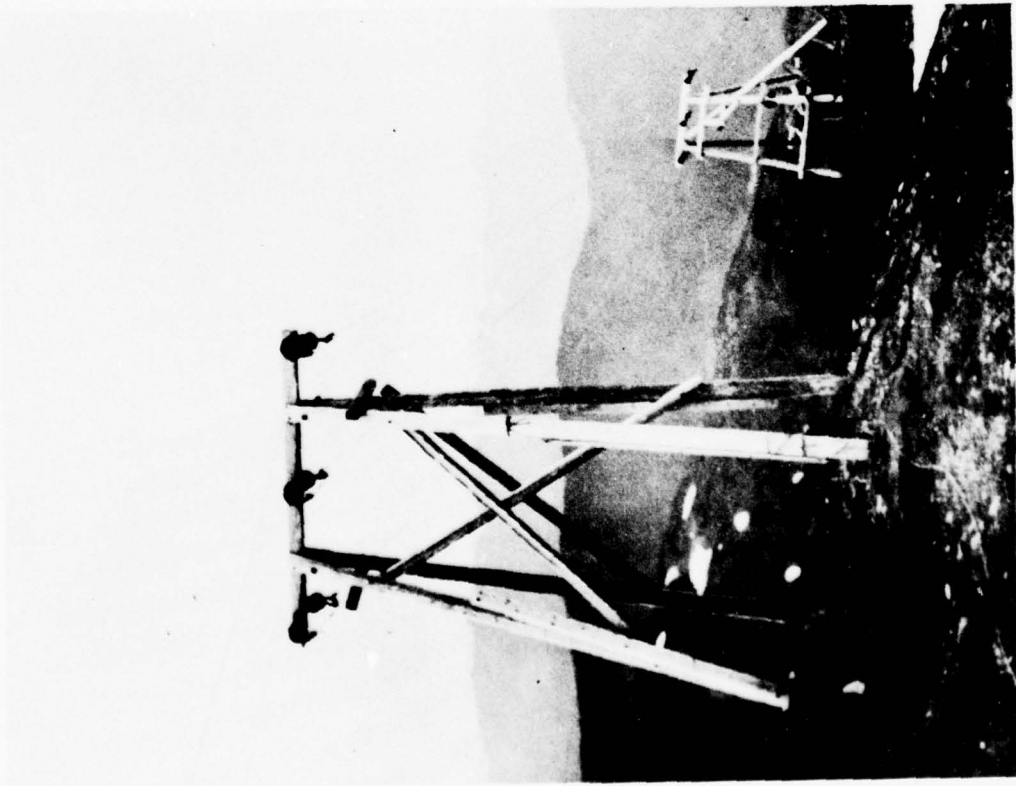


PHOTO # 1 - Typical type construction on A-J Industries
23kv line

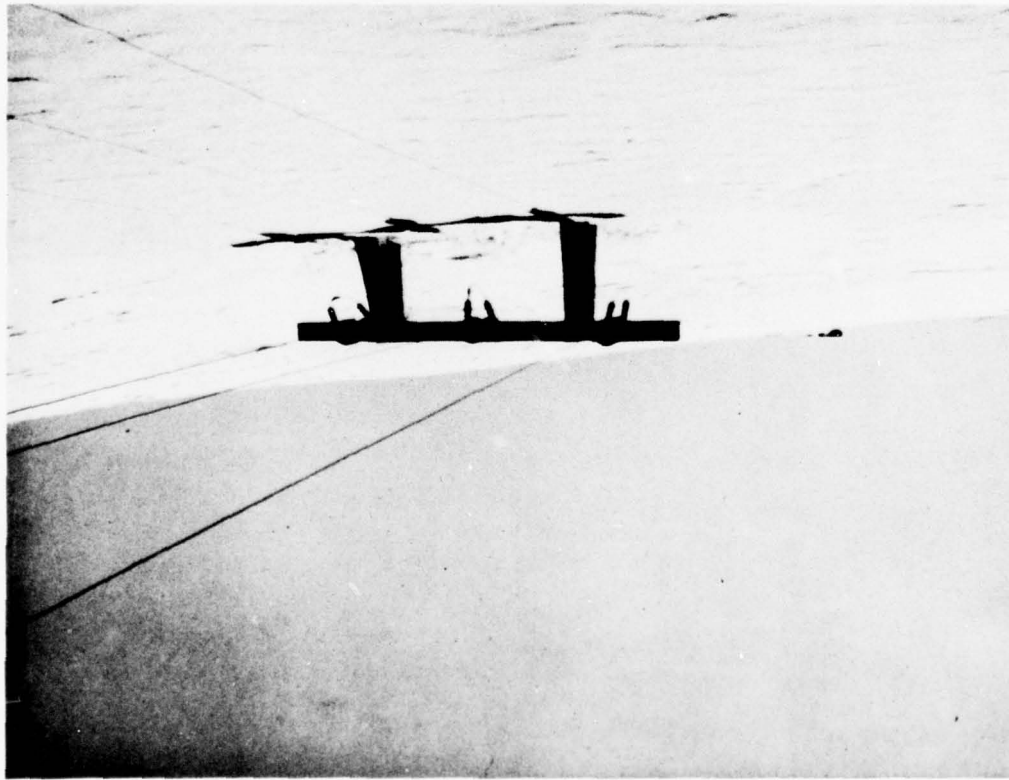


PHOTO # 3 - Conductor passing through drift on
A-J Industries 23kv line

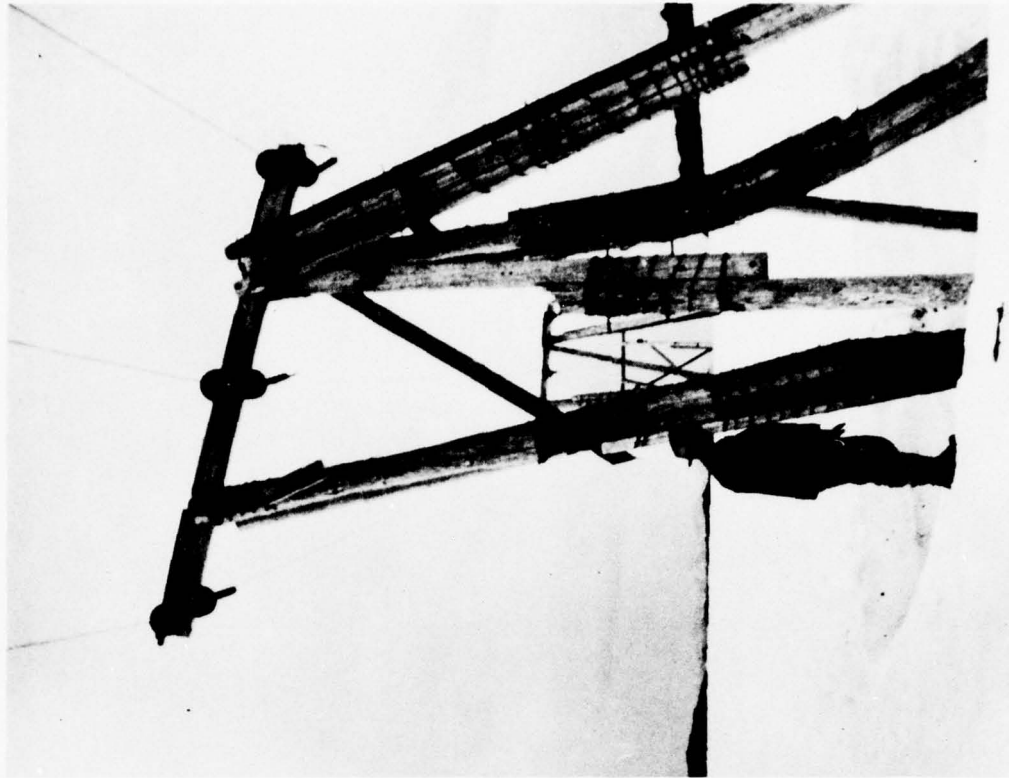


PHOTO # 4 - Emergency repairs to creep damaged
structures on A-J Industries 23kv line

APPENDIX C

SNETTISHAM PROJECT, ALASKA

DESIGN MEMORANDUM NO. 7

Appendix C - Foundation and Material Data

APPENDIX C

- PART 1 - CONCRETE AGGREGATE - AREA A
- PART 2 - CONCRETE AGGREGATE - AREA B
- PART 3 - CONCRETE AGGREGATE - PETROGRAPHIC A & B
- PART 4 - CONCRETE AGGREGATE - TRIAL MIX - C
- PART 5 - CONCRETE AGGREGATE - COST STUDY A, B & C

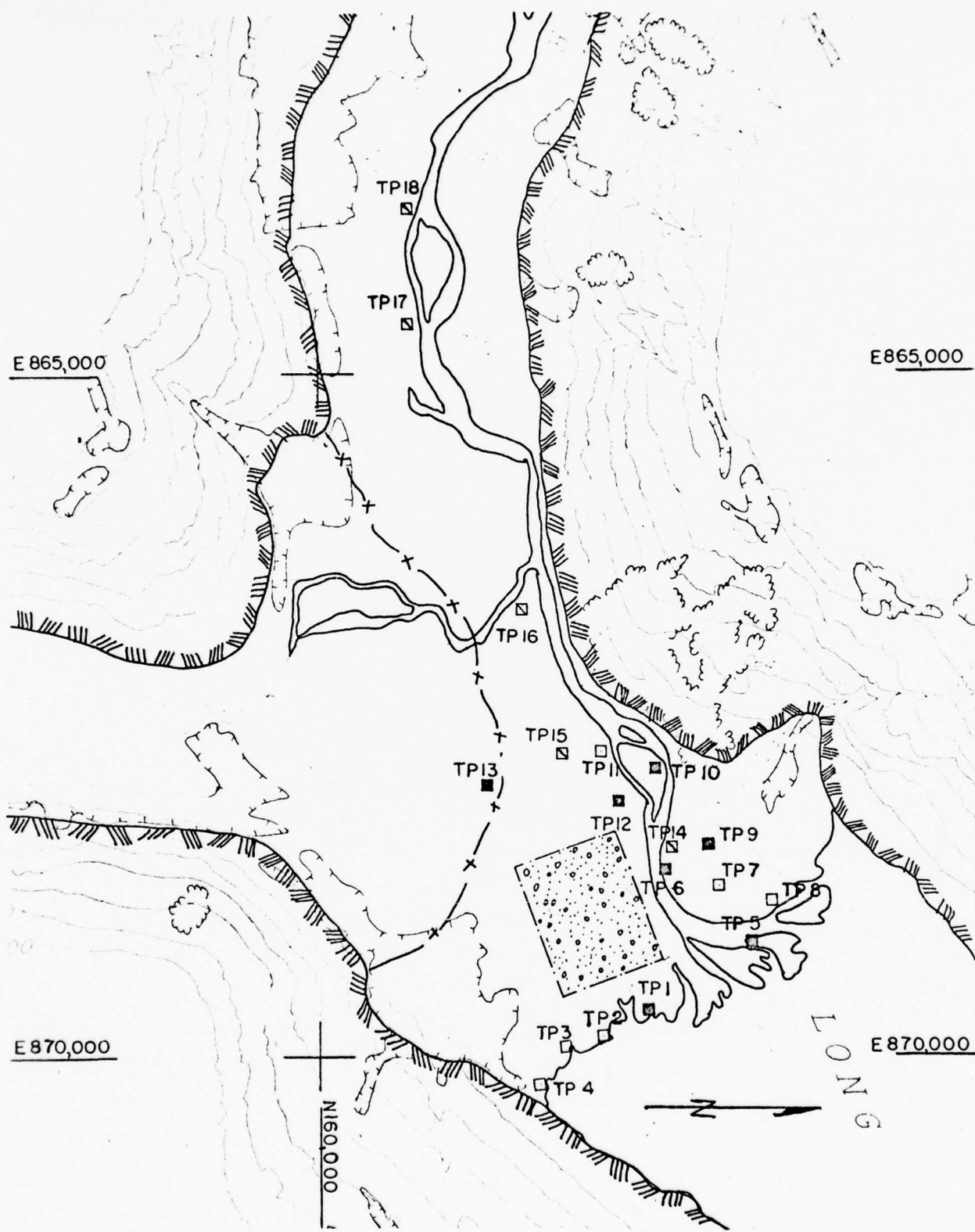
- PART 6 - LOG RECORDS - TEST PITS
- PART 7 - LOG RECORDS - CORE HOLES
- PART 8 - LOG RECORDS - AUGER HOLES
- PART 9 - LOG RECORDS - WASH PROBES
- PART 10 - LOG RECORDS - VANE SHEAR

- PART 11 - DIVING RECONNAISSANCE IN LONG LAKE

PART I

CONCRETE AGGREGATE

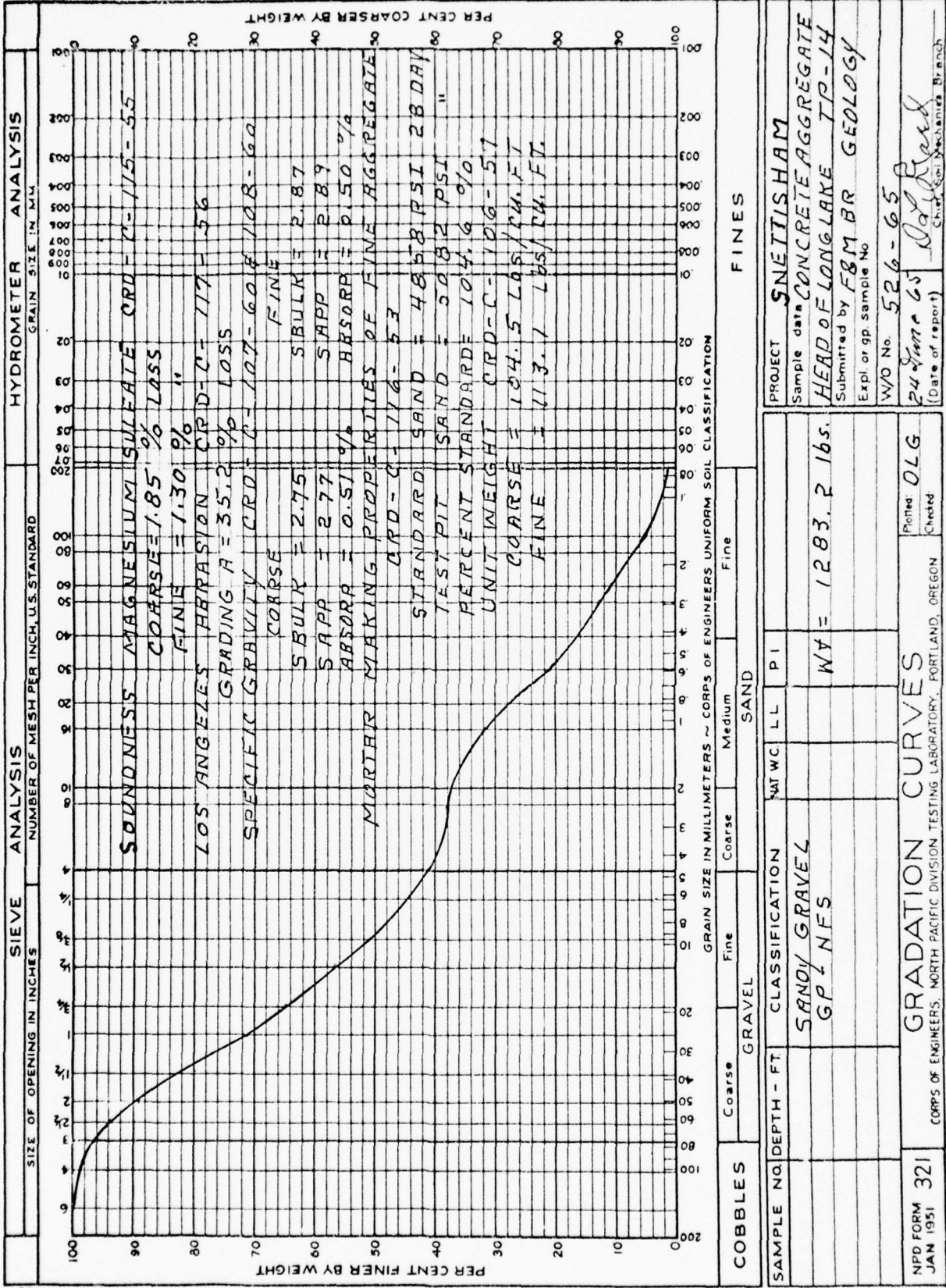
AREA A

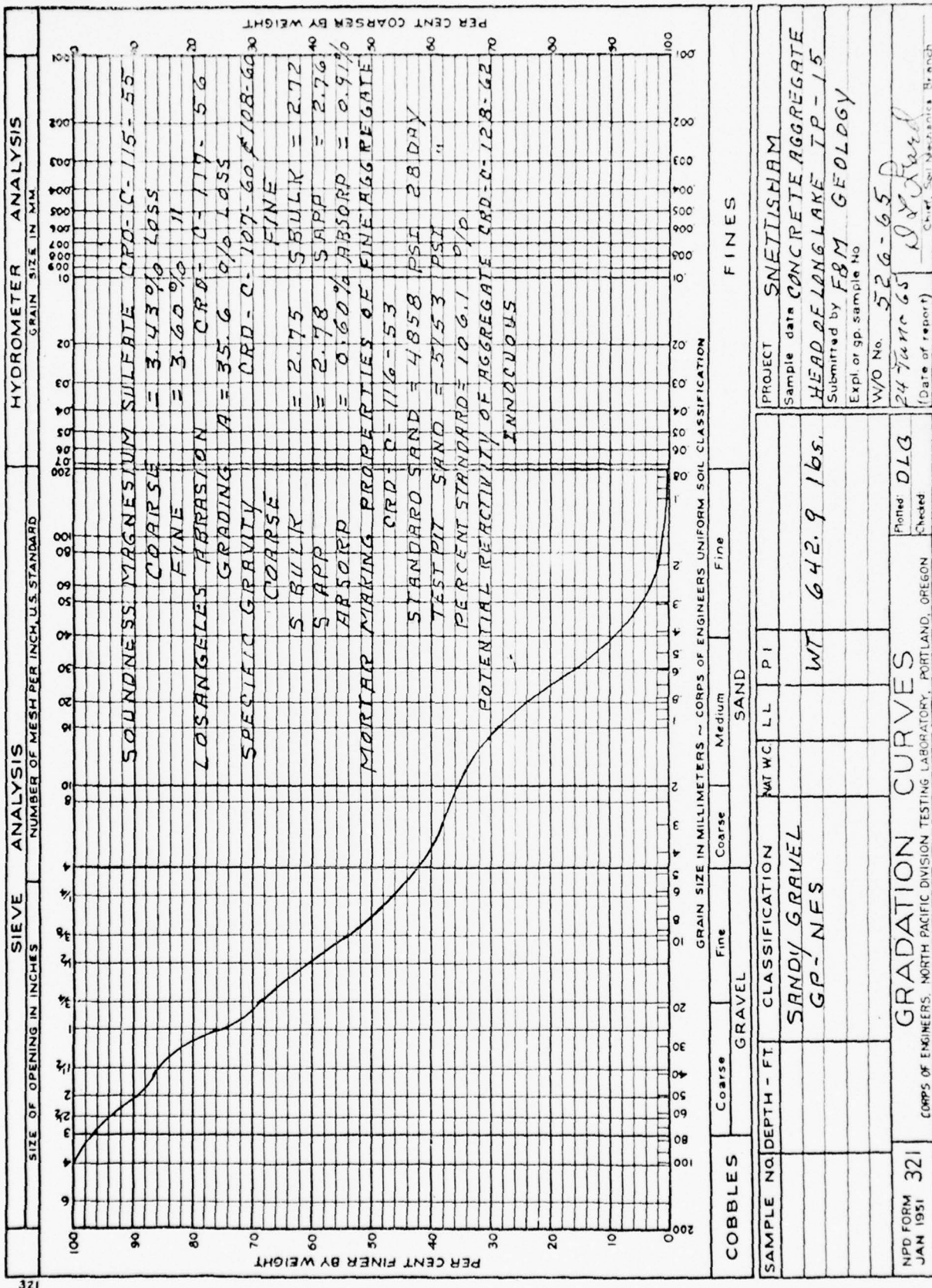


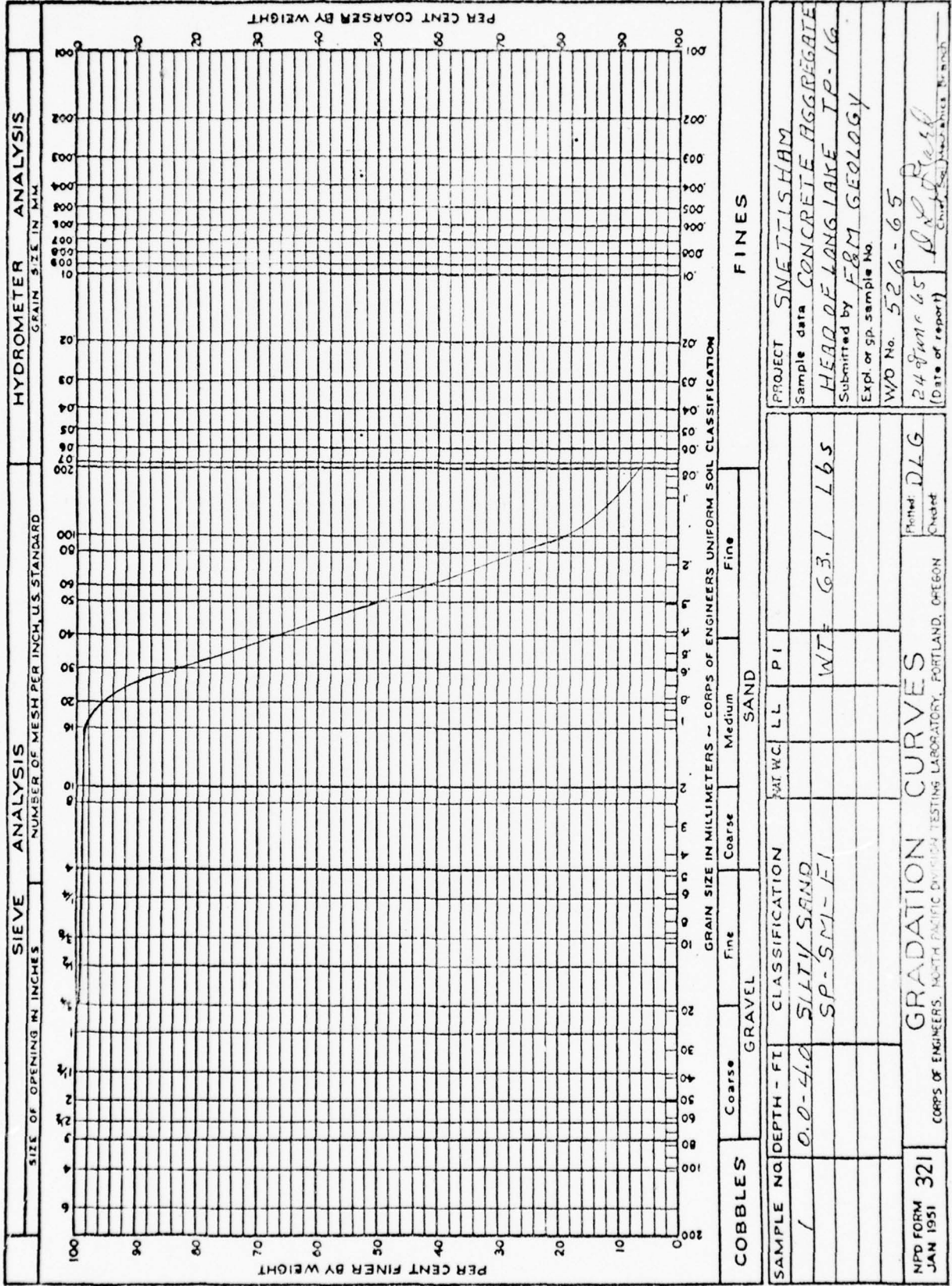
- TP2 TEST PIT NO SAMPLES
- TP1 TEST PIT PRELM. TEST
- ▣ TP15 TEST PIT SECONDARY TEST
- ▨ AREA OF DETAILED AGGREGATE STUDY
- x- MORaine FRONT EDGE

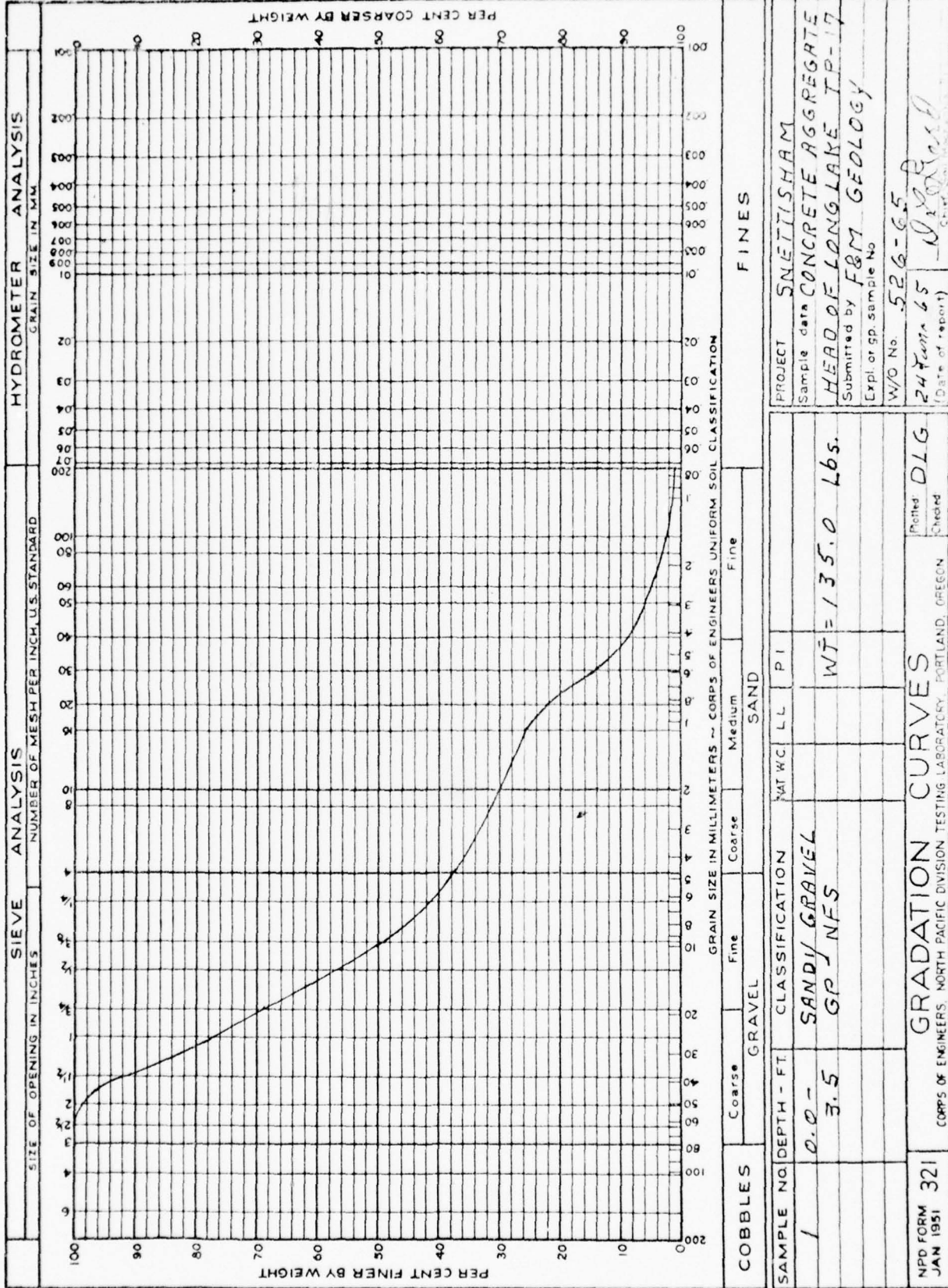
CONCRETE AGGREGATES
AREA A

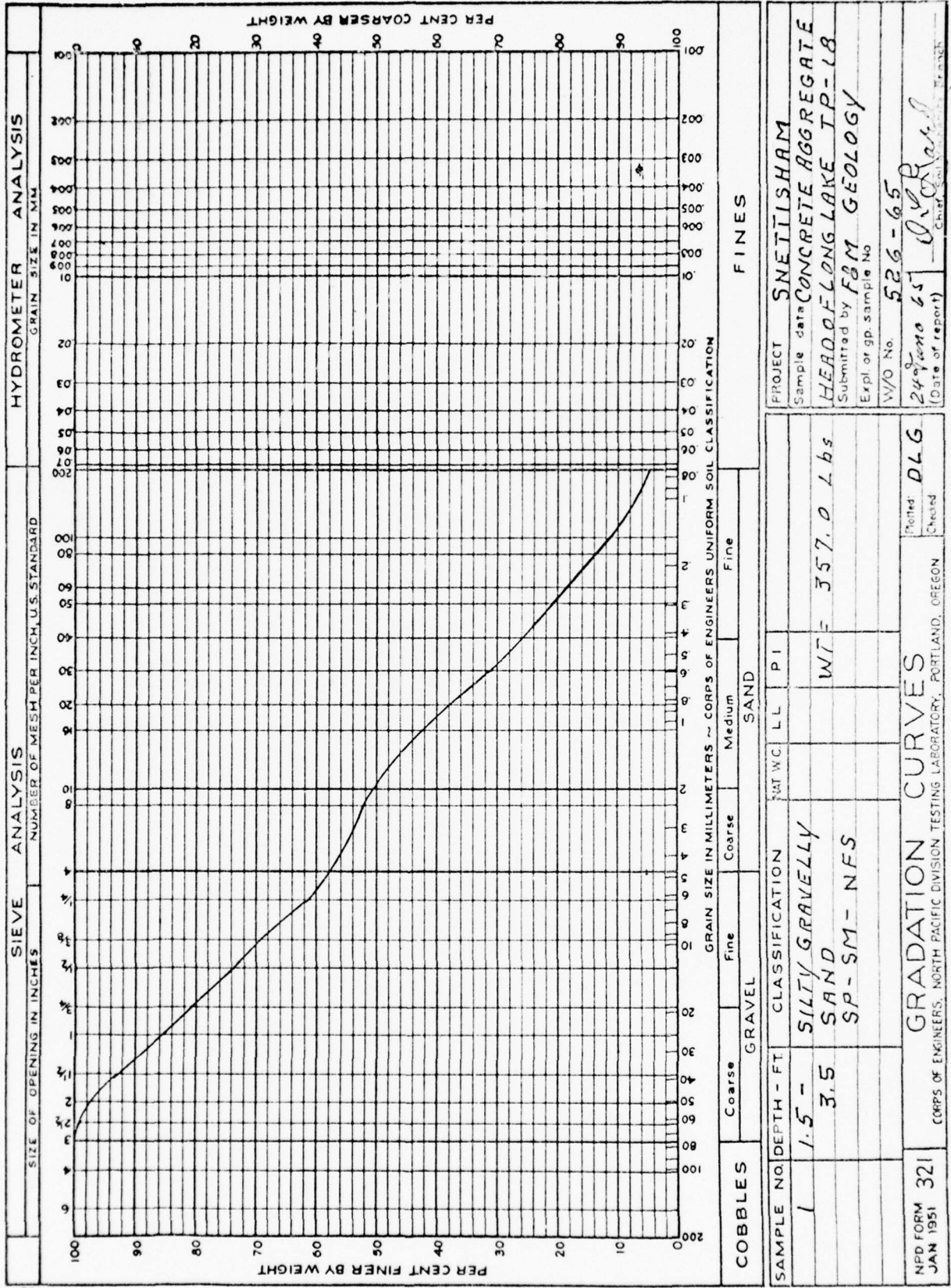
1" = 1,000'











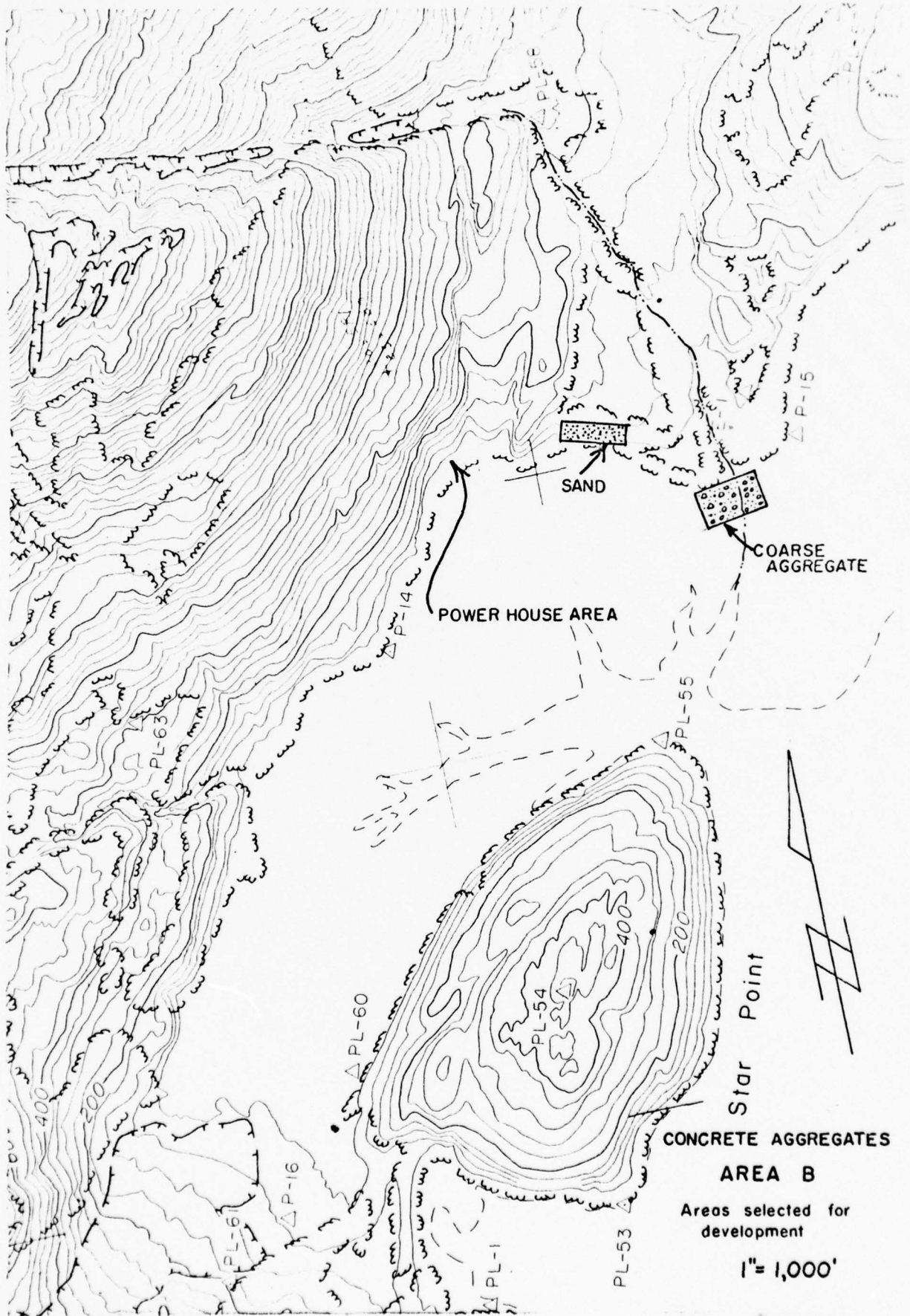
PROJECT **SNETTISHAM**
Sample data **CONCRETE AGGREGATE**
HEAD OF LONG LAKE TP-18
Submitted by **F8M GEOLOGY**
Expl. or gp. sample No
W/O No. **526-65**
24 Jan 65
(Date of report) **O. R. ...**

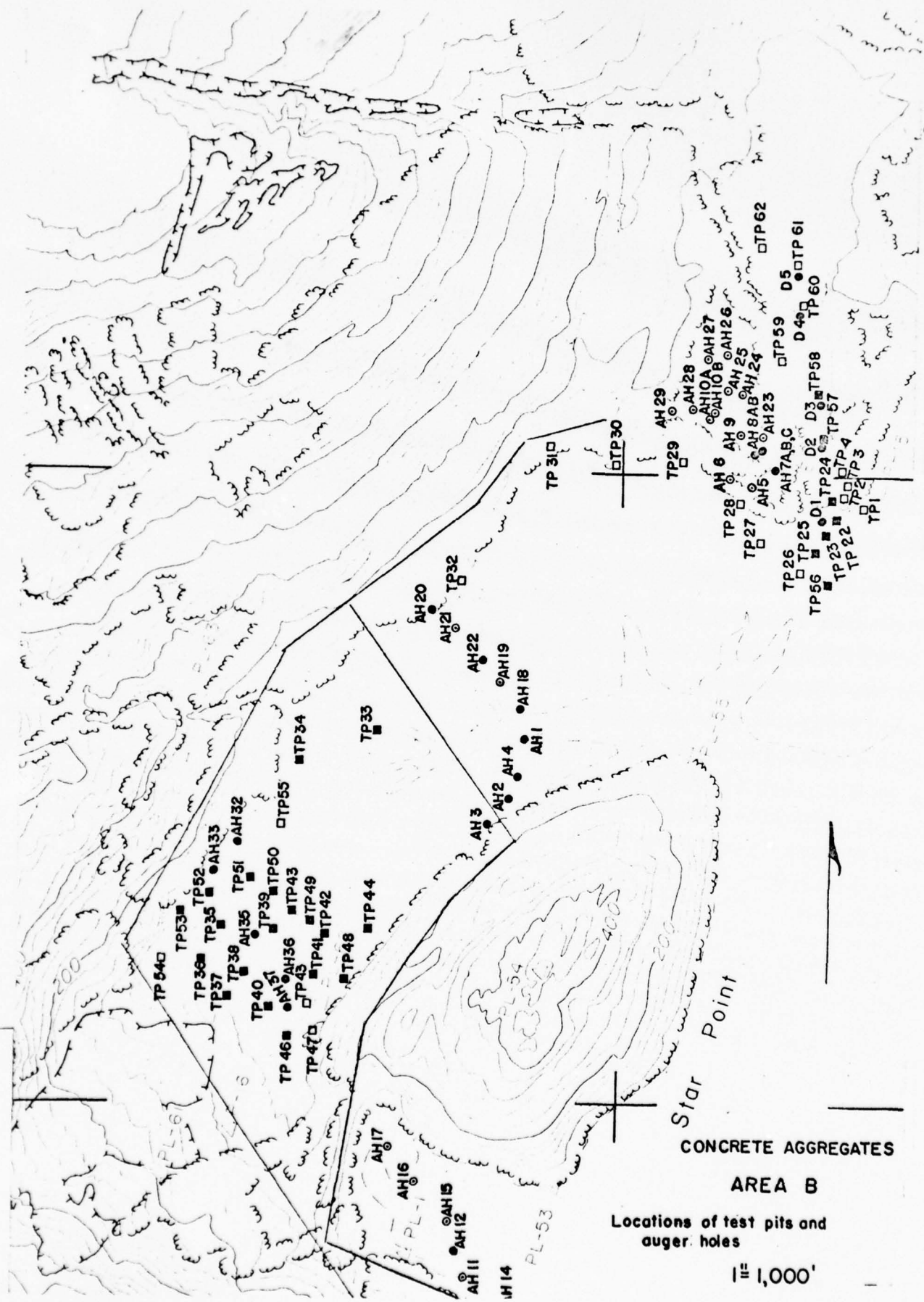
SAMPLE NO.	DEPTH - FT	CLASSIFICATION	WAT WC		PI
			LL	LL	
1	1.5 - 3.5	SILTY GRAVELLY SAND SP-SM-NFS			
			WT = 357.0 Lbs		
GRADATION CURVES			Plotted DLG Checked		
NPD FORM JAN 1951	321	CORPS OF ENGINEERS, NORTH PACIFIC DIVISION, TESTING LABORATORY, PORTLAND, OREGON			

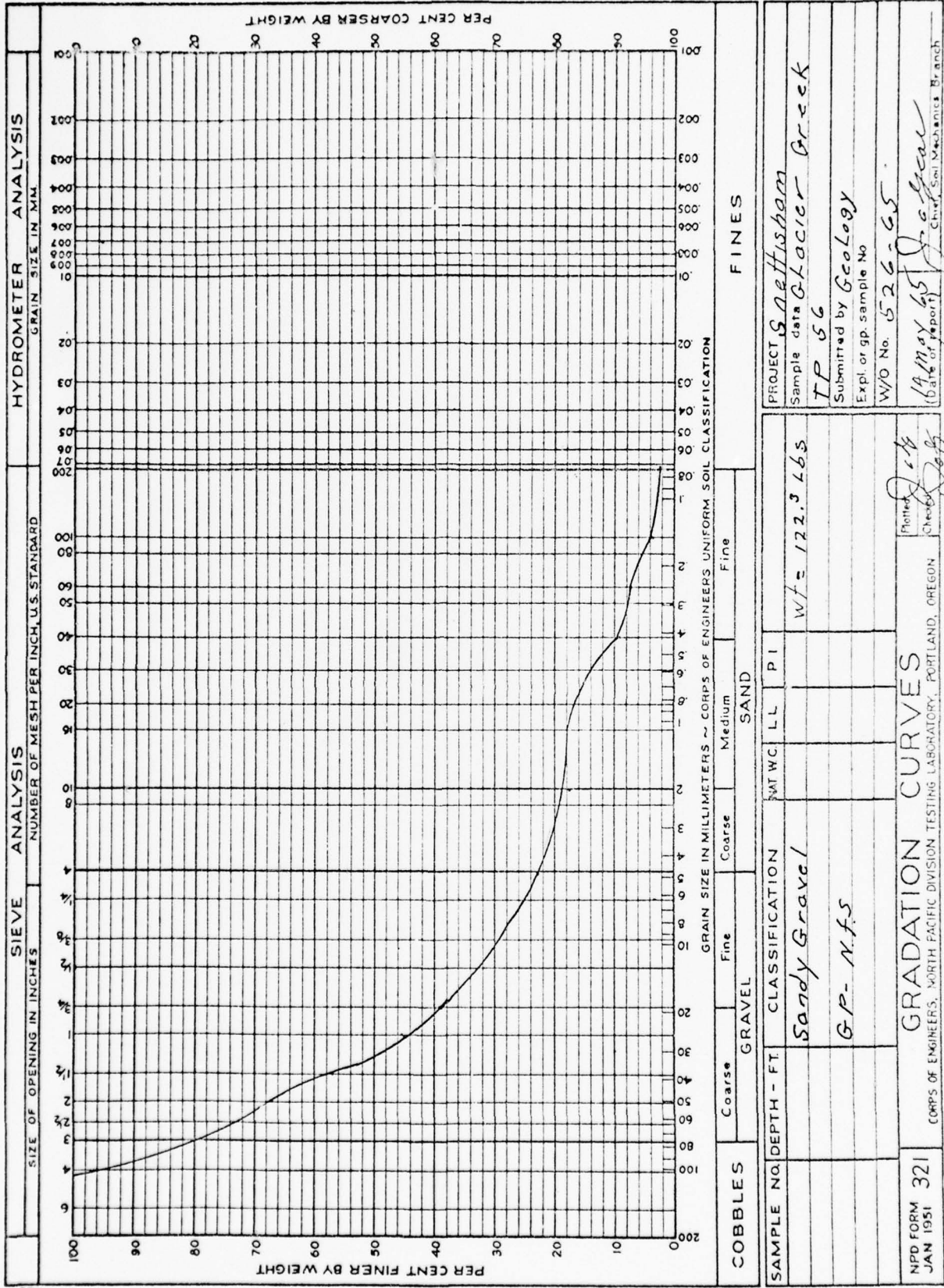
PART 2

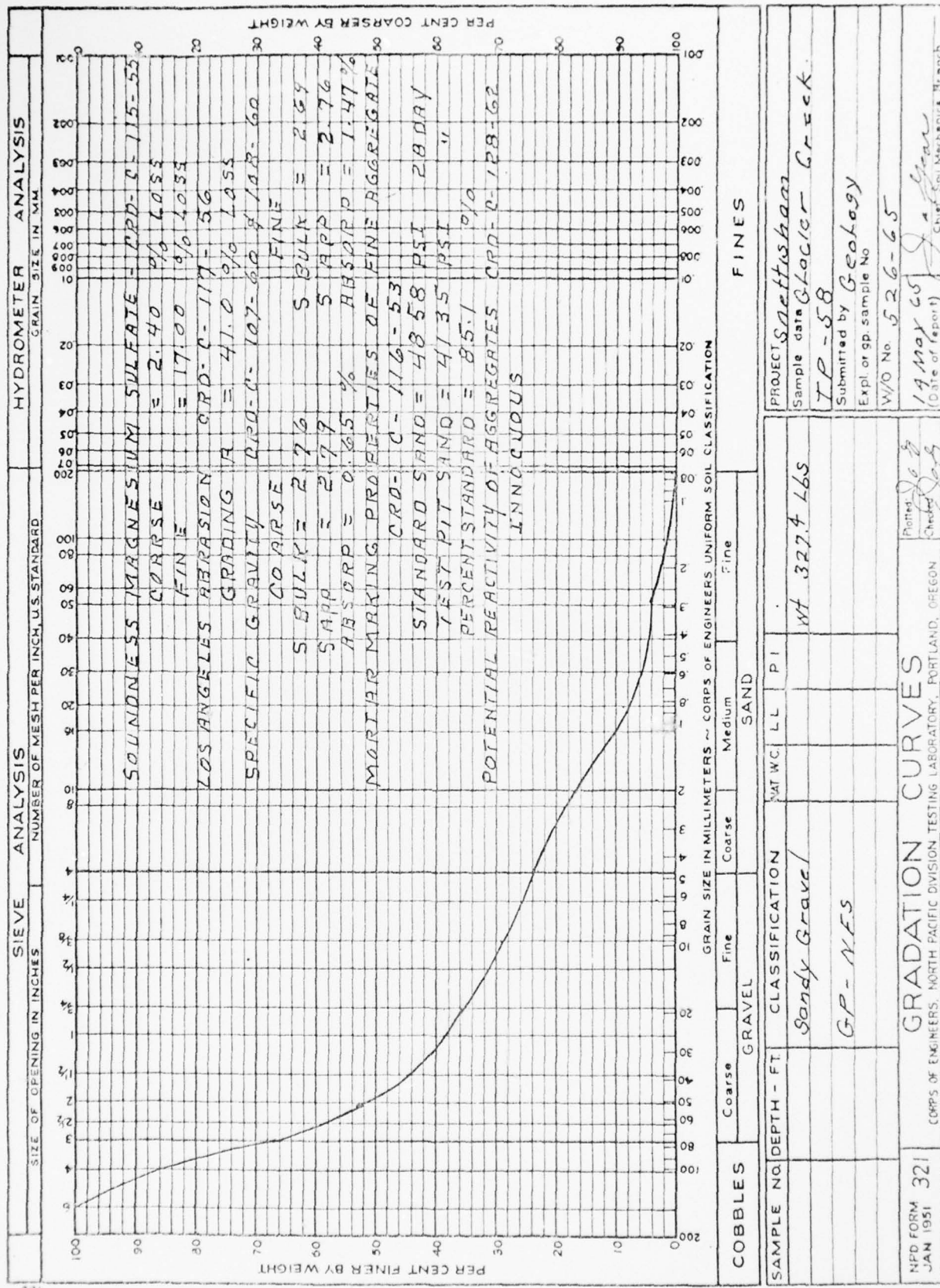
CONCRETE AGGREGATE

AREA B

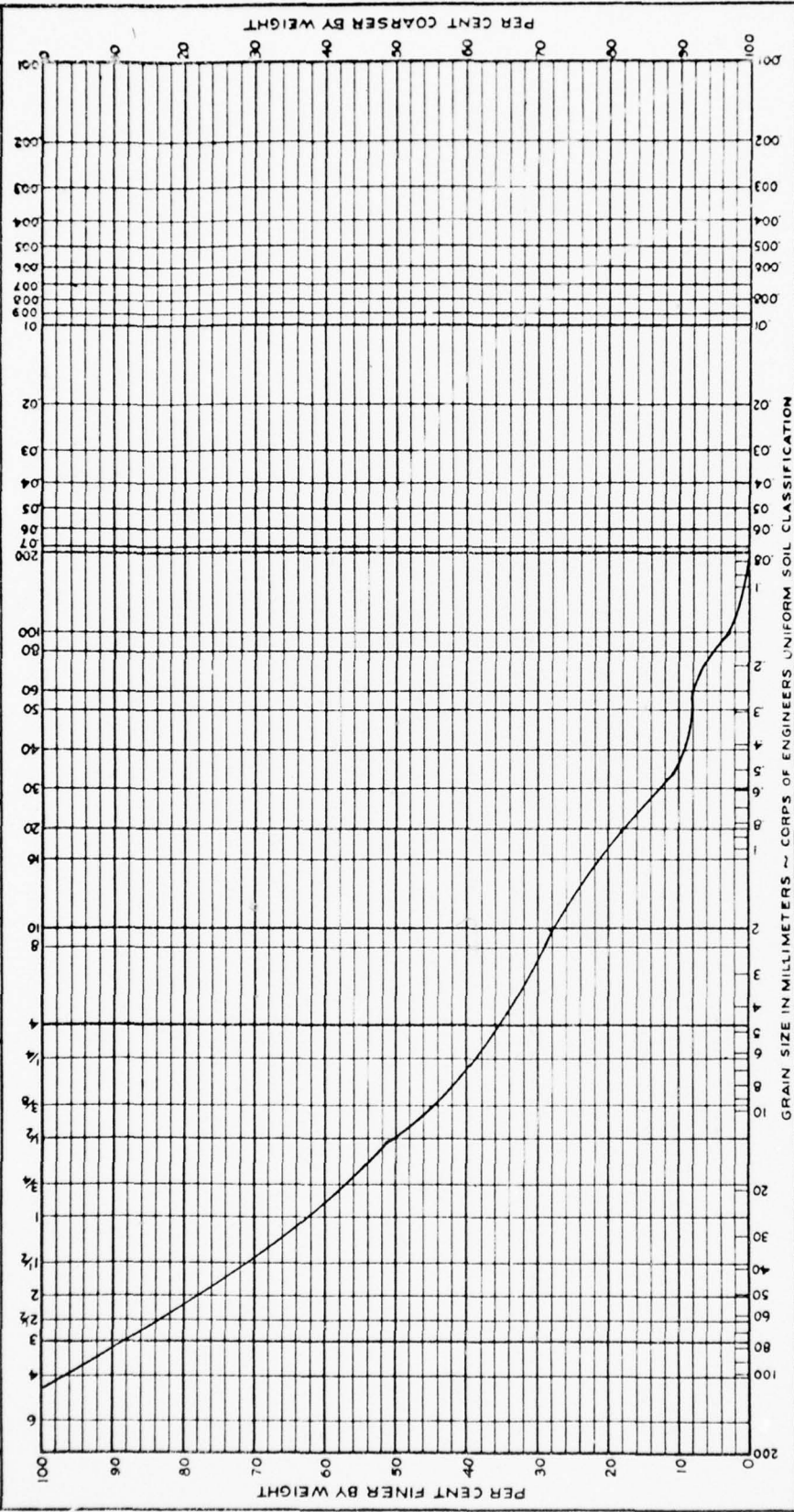




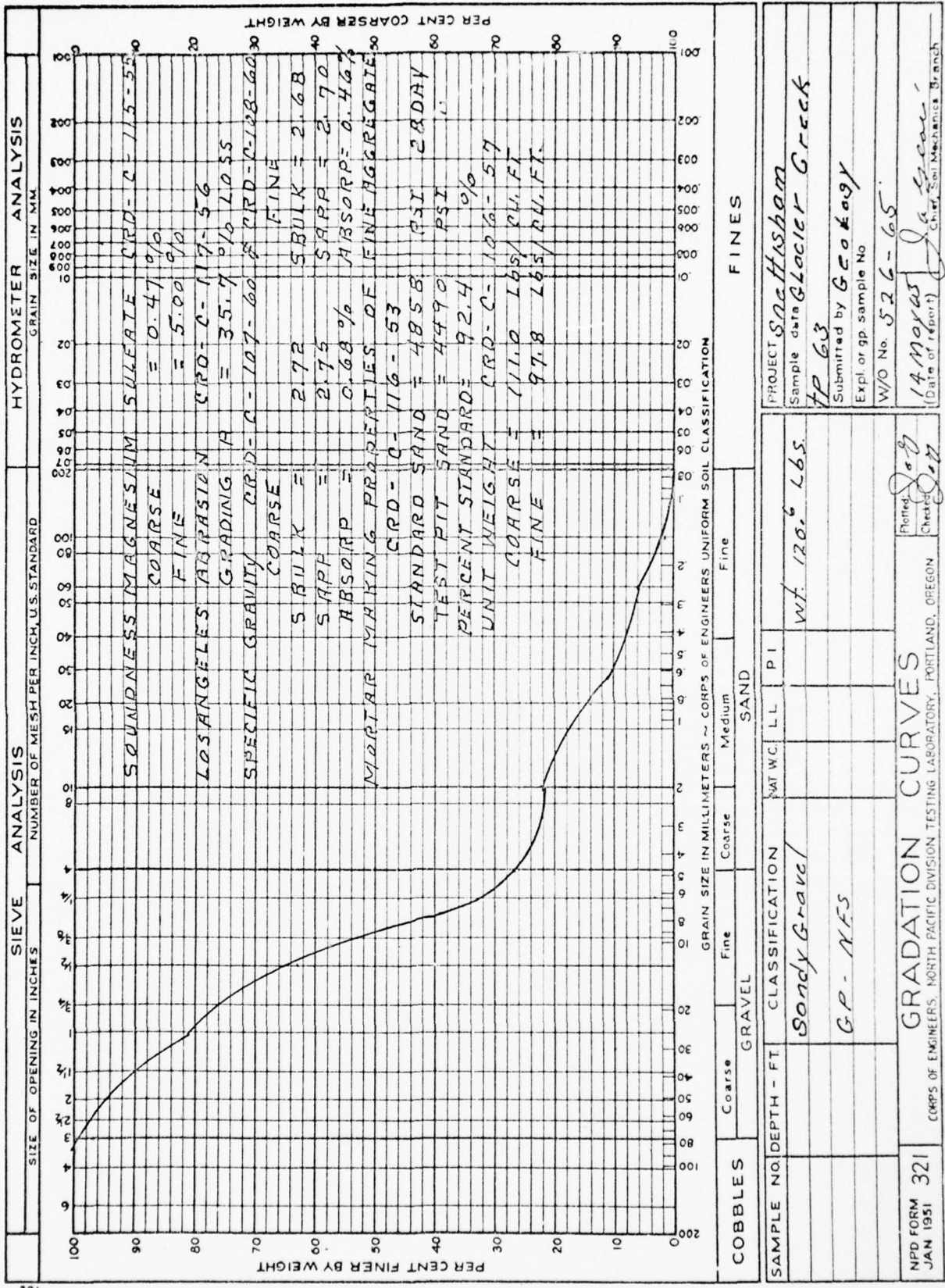




SIEVE ANALYSIS		HYDROMETER ANALYSIS	
SIZE OF OPENING IN INCHES		GRAIN SIZE IN MM	
NUMBER OF MESH PER INCH, U.S. STANDARD			



COBBLES		GRAVEL		SAND		FINES	
Coarse	Medium	Fine	Coarse	Medium	Fine		
DEPTH - FT		CLASSIFICATION		PI		PROJECT	
		Sandy Gravel		wt = 116.7 lbs		S. Nettlesham	
		GP - N.F.S.				Sample data Glacier Creek	
						Submitted by Geology	
						Expl or gp sample No	
						W/O No. 526-65	
NPD FORM 321 JAN 1951		GRADATION CURVES		CORPS OF ENGINEERS, NORTH PACIFIC DIVISION TESTING LABORATORY, PORTLAND, OREGON		14 MAR 65 (Date of report)	
						Chief, Soil Mechanics Branch	



PART 3 CONCRETE AGGREGATE - PETROGRAPHIC A & B

ADDRESS REPLY TO
DIRECTOR
(NOT TO INDIVIDUALS)

U. S. ARMY ENGINEER DIVISION, NORTH PACIFIC
CORPS OF ENGINEERS
NORTH PACIFIC DIVISION MATERIALS LABORATORY
RT. 2, BOX 12A
TROUTDALE, OREGON

NPDEN-GS-L (65-P-81)

19 August 1965

SUBJECT: Petrographic Analysis of Snettisham Project Natural Aggregates

TO: District Engineer
U. S. Army Engineer District, Alaska
ATTN: NPAEN-FM-G

1. Reference is made to:

a. Your Disposition Form dated 19 May 1965 requesting petrographic tests on natural aggregates from Long Lake, Glacier Creek and Crater Creek test pit sources.

b. Telephone conversation your Mr. Dixon and our Mr. Higgs on 4 August 1965 wherein it was agreed that only 3/4"-#4 and sand nominal sizes of all samples be tested because of insufficient amounts of plus 3/4" materials.

2. Inclosed and completing all work requested by above reference 1a is petrographic report containing test results on five test pit samples from Glacier Creek, Crater Creek and Long Lake sources.

- 2 Incl (in dupe)
1. Petro Rpt
2. Tabulations (I and II)

J. L. Bargar
J. L. BARGAR
Director

CORPS OF ENGINEERS, U.S. ARMY OFFICE OF THE DIVISION ENGINEER NORTH PACIFIC DIVISION 500 PITTOCK BLOCK PORTLAND 5, OREGON NPD TESTING LABORATORY		PETROGRAPHIC REPORT	
		DATE AUG 19 1965	EXPL. NO. SAMPLE NO. Test Pits 33, 34, 58, 63 & 64
		PETROGRAPHER N. B. Higgs	W/O NO. 65-P-81
SAMPLED BY District Personnel	PROJECT Snettisham Project, Juneau, Alaska	SOURCE Natural Sand and Gravel from Test Pits TP-33 and TP-34, Crater Creek; TP-58 and TP-63, Glacier Creek; and TP-64, Long Lake.	
SUBMITTED BY (DIST OR AGENCY) Alaska District			

1. Samples and Tests

Natural sand and gravel material from test pit excavations TP-64 near Long Lake, TP-58 and TP-63 near Glacier Creek, and TP-33 and TP-34 near Crater Creek were quartered, prescreened into separate screen sizes, and then samples from the separated screen fractions were tagged in small bags that were placed by test pit designation in five large sack samples and shipped to NPD Materials Laboratory for a brief petrographic examination (P-A-16). Although gravel sizes up to four inch were present in samples submitted, examination of material from each of the five pit sources revealed that insufficient plus 3/4" material was available for petrographic testing; therefore, and as agreed to by Alaska District personnel, only 3/4"-#4 and sand nominal sizes of material were tested. Test method CRD-C-127, Examination of Aggregates for Concrete, requires that 300 particles per screen fraction of gravel materials be examined for reliable results. Approximate minimal weights of materials required for 300 particles in screen fractions up to the 2"-1 1/2" are indicated on page 10 of this test method under note to paragraph 4 (a). Attached tabulation sheet I contains percent constituents per screen fraction of gravels from the five test pit sources and footnoted thereto are the screen fractions that contained amounts of materials less than recommended by CRD-C-127. As the gravels were prescreened prior to submittal, arithmetic averages rather than weighted average percentages based upon gradation of materials were used in computing total composition of each gravel sample. Field gradations along with given percentages of fraction data can be used to compute weighted average percent compositions as desired. The submitted sand samples from the test pits had been prescreened and preweighed to sand screen sizes other than those usually used for concrete sand gradations; therefore, all sand materials from each test pit were composited and rescreened so that examination could be performed on standard screen size fractions. Contained in tabulation II are the weighted average percent constituents in five sand samples as computed using percent retained figures obtained from near the midpoint of the percent retained range for screen sizes given in standard civil works concrete sand specification gradations and at a fineness modulus of 2.77. Soft particles in gravels were determined by test method CRD-C-130 and the NPD Red Devil Shaker test was used to find amounts of soft constituents in sands. Over-all view is that amounts of materials examined are considered sufficient for a brief petrographic examination of investigatory materials and, although samples are insufficient for a complete petrographic analysis, the data now gleaned should serve as a basis for tentative comparisons of current and future samples. A large bulk sample of natural sand and gravel materials is now being obtained at Snettisham Project and this sample should furnish detailed petrographic data.

PETROGRAPHIC REPORT

(CONTINUED)

DATE AUG 19 1965	EXPL NO	SAMPLE NO 33, 34, 58, 63 & 64	Test Pits w/o NO
PETROGRAPHER N. B. Higgs		65-P-81	

2. Composition

a. Gravels: All five samples consist essentially of granitic stones and represent glacial out-wash gravels derived from granite rocks of the Coast Range batholith. These granite rocks form a major portion of the country rock at Snettisham project and at Long Lake Damsite the granite is a quartz diorite that commonly has a gneissoid or gneissic structure with randomly alternating, subparallel, light and dark colored bands or laminations containing varying proportions of quartz, feldspar, hornblende and biotite mica minerals. The variably foliated nature of these granitic rocks is shown in the photograph to Exhibit I of Design Memorandum No. 3, Selection of Plan of Development, Snettisham Project, Alaska. The white bands in the foliated granite consist primarily of equidimensional quartz and feldspar with minor dark minerals; whereas, the darker colored bands contain dark platy biotite mica and tubular to prismatic hornblende as well as the equidimensional quartz-feldspar minerals. As the gravels submitted are derived from similar gneissoid granitic rocks, the rock types present in the samples represent specific portions of granite foliations, dike and vein materials, and associated metamorphics and cataclastites. Quartz-feldspar rock consists of fine grained vein and aplite dike material, medium to coarse grained white rock from foliation bands and an occasional coarse grained pegmatitic stone. Quartz-feldspar rocks all contain less than 10% of dark-colored biotite-hornblende minerals. Light-colored, speckled black and white, granite rocks of attached tabulation I are the typical granite materials from the batholith. They are prevailing medium to coarse grained and by megascopic examination contain less than 30% and more commonly less than 20% of dark biotite-hornblende minerals. Quartz-feldspar and light-colored granite stones both show only slight foliation. Dark-colored biotite hornblende rocks are derived chiefly from the darker colored bands in foliated intervals of the granite, contain dark minerals in amounts greater than 30%, and are prevailing fine grained. Stones having a distinct gneissose to schistose foliated structure are concentrated in the dark-colored biotite-hornblende fraction and average about 28% and 29% of test pits 34 and 58 and less than 20% of the other test pit materials. Easily split or cleaved chlorite-talc, mica, or quartz-mica schists were not observed in any of the gravel samples. The dark biotite-hornblende rock represents schistose intervals in the gneissoid granite and these stones in sample are of hard and non-splittable type. Miscellaneous particles include hornfel and granulite metamorphic stones, epidosite granites, vein metamorphic materials and a few hard cataclastic breccia stones. The minor amounts of dark, fine grained basalt particles found in samples is similar to basalt dike material in a core segment previously from foundation material at Long Lake Damsite. A few of the basalt and possibly some of the miscellaneous stones may be meta-graywacke, however, thin section studies would be required for positive identification. Test pits 58 of Glacier Creek and 34 of Crater Creek contain greater amounts of biotite-hornblende rock; otherwise, compositions of test pit samples are similar. Gravels of Long Lake Test Pit 64 have the least variable lithology and consist essentially of light-colored granite stone.

PETROGRAPHIC REPORT

(CONTINUED)

DATE AUG 14 1965	EXPL. NO.	SAMPLE NO. Test Pits 33, 34 58, 63 & 64
PETROGRAPHER N. B. Higgs		W/O NO 65-P-81

b. Sand: Except for discrete minerals derived from disintegration of constituent rock types, the sands are all similar in composition to the gravels and of granitic lithology. Only significant variations in composition between sand samples is the greater amounts of dark-colored biotite-hornblende rock in TP-34 of Crater Creek and TP-58 of Glacier Creek (see tabulation II). The gravels from these test pits also contained greater amounts of this type material, however, quantities of biotite-hornblende rock in sands from these two test pits are substantially less than in the gravels. Discrete hornblende exceeds discrete biotite grains in all sand samples and test pits with greater amounts of biotite-hornblende rock have discrete hornblende two to three times as abundant as biotite mica flakes. Discrete biotite and hornblende together comprise between 10 and 20 percent of a sample and occur primarily in material passing the No. 30 sieve. Mica flakes are most abundant in material passing the No. 100 sieve size. Plagioclase feldspar is the major discrete mineral constituent and quartz approximates either hornblende or mica contents. Epidote, garnet, and magnetite or ilmenite are the prominent accessory minerals.

c. Deleterious Constituents: Alkali-reactive constituents were not observed in sand and gravel materials of the five test pit samples. The trace to one percent amounts of basalt in samples show the effects of granite assimilation and their contribution to alkali-reactivity is considered nil. Pyrite was observed in a few of the vein stone and epidosite granite materials but forms only trace amounts of test pit samples.

3. Physical Quality

a. Gravels: Except for the gravels from TP-33 of Crater Creek, test pit gravel materials are slightly to moderately weathered and stained, contain less than 10% of moderately well weathered stones, and have only trace to three percent amounts of soft particles (see tabulation I). Approximately one-third of TP-33 3/4"-#4 gravel material consists of stones that are pronouncedly stained with iron oxides and of reddish brown color. The iron oxide stain material is in the form of coatings at particle exteriors and occurs as thin films of tightly adhering material that usually covers more than 50% of a stones surface. Interiors of iron oxide coated or stained particles are generally only slightly to moderately altered. The iron oxide stains or coatings were usually not found to be soft to the brass rod scratch test, CRD-C-130. Soft particles in all gravel samples are largely friable stones that are sufficiently weathered so that grain bond is lost and granulation occurs. Granite stones in samples with progressive weathering tend to form granulated friable grain surfaces before altering to soft clay materials. Occasional soft stones that could be scratched by a brass rod were observed in samples but these were usually vuggy vein type materials. A few of the schistose biotite-hornblende stones broke along the foliation when weathered; however, during hammering of these stones most of the particles cleaved across the foliation and degree of weathering rather than rock type tends to govern the physical quality of these materials. Closely foliated and easily cleaved or splittable strongly micaceous schists were not observed in samples submitted.

PETROGRAPHIC REPORT (CONTINUED)	DATE	EXPL NO	SAMPLE NO	Test Pits
	AUG 19 1965		33, 34, 58, 63 & 64	
	PETROGRAPHER		W/O NO	
	N. B. Higgs		65-P-81	

Total undesirable particles - i.e., soft, friable, crumbly, fractured and cleavable stones - amount to less than 10% of gravel samples. Fractured and crumbly stones are particles having planes of weakness of such extent that these particles break or disintegrate when hit a few light blows with a 100 gram tack hammer. Stones with fractures or joints that are not easily cleaved are thus not included within this category. Most of the fractured and crumbly stones were found in the moderately well weathered materials. Particle shapes become more angular with decreasing screen size and are rounded to subrounded for plus 1/2" materials and prevailing subrounded to subangular for materials passing this screen. Particle surfaces average moderately rough for all test pit materials with most of the granite stones having pitted exteriors. Thin and elongate particles range from one to three percent of test pit samples. The gravels examined are from small sack samples and are thus only indicators of the quantity and types of particles present in test pit sources. However, physical quality comparisons of samples submitted would indicate the following: (1) Long Lake TP-64 gravels are of better physical quality than materials from other sources; (2) Crater Creek TP-33 gravels are inferior in physical quality to other materials; (3) gravels of Crater Creek TP-34 and Glacier Creek TP-58 are similar in quality and about average for materials submitted; and (4) gravels from the Glacier Creek TP-63 source are closest to Long Lake gravels in physical quality. Based upon samples submitted, over-all view is that Crater Creek gravels are inferior to gravels from other sources.

b. Sands: All test pit sand samples are slightly to moderately weathered and contain less than 6% of weathered grains by particle count (see tabulation II). Soft constituents as determined by the Red Devil Shaker test amount to 2.4% of TP-33, 3.6% of TP-34, 2.8% of TP-58 and 1.8% of TP-63 sand materials. Insufficient sample was present for performance of shaker test on Long Lake TP-64 sand; however, judging from the quantity of weathered grains counted in this sample, amounts of soft constituents present would probably be similar to those found in TP-63 Glacier Creek sand. In comparison, a rod-mill sand made from granite ledge rock at Long Lake Damsite contained 3% soft constituents and most of the sands used in major NPD concrete structures have had less than 6% of soft constituents. Both discrete mica flakes and soft constituents are most abundant in Crater Creek TP-34 sand. Particle shapes are subangular to subrounded for material retained on the No. 30 sieve and subangular to angular for material passing this sieve. Particle surfaces are rough. Mica flakes constitute the majority of thin and elongate particles.

NELSON B. HIGGS
Chief, Petrography Branch

NPDEN-GS-L
TABULATION I

PERCENT BY SCREEN FRACTION OF CONSTITUENTS IN NAT
FROM TEST PITS OF GLACIER CREEK, CRATER CREEK AND LONG LAKE SOU

Screen Size	Quartz-Feldspar	Light Colored Speckled Granite, Less Than 30% Dark Minerals	Dark-Colored Biotite-Hornblende Rock, More Than 30% Dark Minerals	Miscellaneous	Basalt	Megascopically Foliated, Gneissoid-Schistose Rock	Slightly Weathered and Stained
<u>Glacier Creek TP 58</u>							
1½"-3/4"	7	46	37	9	1	47	36
3/4"-½"	5	60	28	6	1	32	16
½"-3/8"	10	57	29	4	Trace	20	19
3/8"-#4	18	56	21	4	1	15	9
Arith.							
Average	10	54	29	6	1	29	20
<u>Glacier Creek TP 63</u>							
1½"-3/4"	10	59	24	7		19	46
3/4"-½"	11	61	16	12		16	48
½"-3/8"	10	62	18	9	1	16	40
3/8"-#4	12	68	14	6	Trace	11	26
Arith.							
Average	11	63	18	8	Trace	16	40
<u>Crater Creek TP 34</u>							
3/4"-½"	10	47	41	1	1	34	18
½"-3/8"	10	46	41	2	1	34	10
3/8"-#4	10	56	30	3	1	17	5
Arith.							
Average	10	50	37	2	1	28	11
<u>Crater Creek TP 33</u>							
3/4"-½"	8	66	19	4	3	26	18
½"-3/8"	6	82	10	1	1	10	12
3/8"-#4	6	82	10	1	1	8	8
Arith.							
Average	7	77	13	2	1	15	13
<u>Long Lake TP 64 (LL TP 14)</u>							
1½"-3/4"	5	86	2	7	-	6	85
3/4"-½"	5	85	6	4	-	8	82
½"-3/8"	1	85	11	3	-	9	89
3/8"-#4	3	89	5	3		4	80
Arith.							
Average	4	86	6	4		7	84

- (1) Soft particle test, CRD-C-130, does not require testing of 3/8"-#4 material.
- (2) Data is total weight of as-received material. Sizes footnoted contained less than 300 particles and in and note thereto on page 10 of test method.
- (3) Percentages of soft and friable particles in adjacent column are included in constituents of this fraction physically undesirable particles in samples.

NATURAL GRAVELS
SOURCES, SNETTISHAM PROJECT

AUG 19 1965

<u>Moderately Weathered and Stained</u>	<u>Moderately Well Weathered and Stained</u>	<u>Flat & Elong</u>	<u>Soft & Friable, CRD-130</u>	<u>Soft, Cleavable Fractured & Crumbly (3)</u>	<u>Weight Tested, Grams</u>
61	3	1	3	7	2,828 (2)
74	10	1	3	8	1,308 (2)
69	12	2	4	7	1,085
80	11	4	(1)	(1)	337
71	9	2	3	7	
47	7	1	4	6	2,762 (2)
48	4	Trace	2	4	1,243 (2)
54	6	1	3	5	1,023
68	6	2	(1)	(1)	297
54	6	1	3	5	
75	7	4	3	6	1,015 (2)
84	6	4	3	9	743
90	5	2	(1)	(1)	252
83	6	3	3	7	
52	30	1	2	6	517 (2)
50	38	1	4	7	552 (2)
68	24	3	(1)	(1)	265
57	31	2	3	6	
15	-	-	-	-	1,995 (2)
17	1	-	1	1	749 (2)
10	1	2	-	2	341 (2)
18	2	1	(1)	(1)	314
15	1	1	Trace	1	

insufficient weights of material by CRD-C-127, paragraph 4 (a)

tion. Except for iron oxide stained stones, represents total

APPENDIX C. PART II. A-B

2

NPDEN-GS-L
TABULATION II

WEIGHTED AVERAGE PERCENTAGE BY COUNT OF CONSTITUENTS IN NA
 FROM TEST PITS OF GLACIER CREEK, CRATER CREEK AND LONG LAKE SOURCES

<u>Screen Size</u>	<u>Quartz-Feldspar and Discrete Minerals Other Than Mica</u>	<u>Light-Colored Speckled Granite, Less Than 30% Dark Minerals</u>	<u>Dark-Colored Biotite-Hornblende Rock, More than 30% Dark Minerals</u>	<u>Miscellaneous</u>
<u>Glacier Creek TP 58</u>				
Plus #8	2.7	8.6	3.2	0.5
#8-#16	5.6	8.0	1.9	0.5
16-30	12.4	9.1	1.0	0.5
30-50	15.9	6.2	0.5	-
50-100	13.1	0.8	-	-
Pan	4.1	-	-	-
Total:	53.8	32.7	6.6	1.5
<u>Glacier Creek TP 63</u>				
Plus #8	2.1	9.6	1.8	1.5
#8-#16	6.3	7.6	1.3	0.8
16-30	16.3	5.8	0.7	0.4
30-50	19.3	2.8	0.2	-
50-100	13.0	1.1	-	-
Pan	4.3	-	-	-
Total:	61.3	26.9	4.0	2.7
<u>Crater Creek TP 34</u>				
Plus #8	2.2	8.4	3.9	0.5
#8-#16	4.8	8.0	2.6	0.5
16-30	10.6	11.0	1.4	-
30-50	14.4	6.2	0.7	-
50-100	12.8	0.6	-	-
Pan	4.1	-	-	-
Total:	48.9	34.2	8.6	1.0

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PAGE 1 OF 2

NATURAL SANDS

ES, SNETTISHAM PROJECT

<u>Discrete Mica Flakes</u>	<u>Moderately Well Weathered and Stained</u>	<u>Percent *<!--<br/-->Retained</u>
-	0.8	15
-	0.8	16
1.0	1.2	24
1.4	1.0	24
2.1	0.5	16
0.9	0.2	5
5.4	4.5	100%
-	0.6	15
-	0.6	16
0.3	1.0	24
1.7	0.8	24
1.9	0.5	16
0.7	0.1	5
5.1	3.6	100%
-	1.1	15
0.1	0.8	16
1.0	1.4	24
2.7	1.2	24
2.6	0.8	16
0.9	0.2	5
7.3	5.5	100%

APPENDIX C, PART II, A-B

NPDEN-GS-L
TABULATION II - Continued

WEIGHTED AVERAGE PERCENTAGE BY COUNT OF CONSTITUENTS IN
FROM TEST PITS OF GLACIER CREEK, CRATER CREEK AND LONG LAKE SOUR

<u>Screen Size</u>	<u>Quartz-Feldspar and Discrete Minerals Other Than Mica</u>	<u>Light-Colored Speckled Granite, Less Than 30% Dark Minerals</u>	<u>Dark-Colored Biotite-Hornblende Rock, More Than 30% Dark Minerals</u>	<u>Miscellaneous</u>
<u>Crater Creek TP 33</u>				
Plus #8	3.0	10.2	1.4	0.4
#8-#16	5.1	9.0	1.6	0.3
16-30	14.2	8.4	1.0	0.2
30-50	15.4	7.2	0.2	Trace
50-100	12.2	1.3	0.4	-
Pan	4.4	-	-	-
Total:	54.3	36.1	4.6	0.9
<u>Long Lake TP 64 (LL TP-14)</u>				
Plus #8	1.2	12.5	0.9	0.4
#8-#16	5.6	9.0	1.1	0.3
16-30	15.1	7.4	0.7	0.5
30-50	20.9	1.9	-	-
50-100	14.1	0.3	-	-
Pan	4.5	-	-	-
Total:	61.4	31.1	2.7	1.2

* Percent retained data used in computing weighted percentages are from near midpoint of percent civil works sand gradation specification and at FM 2.77.

AD-A067 895

CORPS OF ENGINEERS ANCHORAGE AK ALASKA DISTRICT
SNETTISHAM PROJECT ALASKA. FIRST STAGE DEVELOPMENT. DESIGN MEMO--ETC(U)
OCT 65

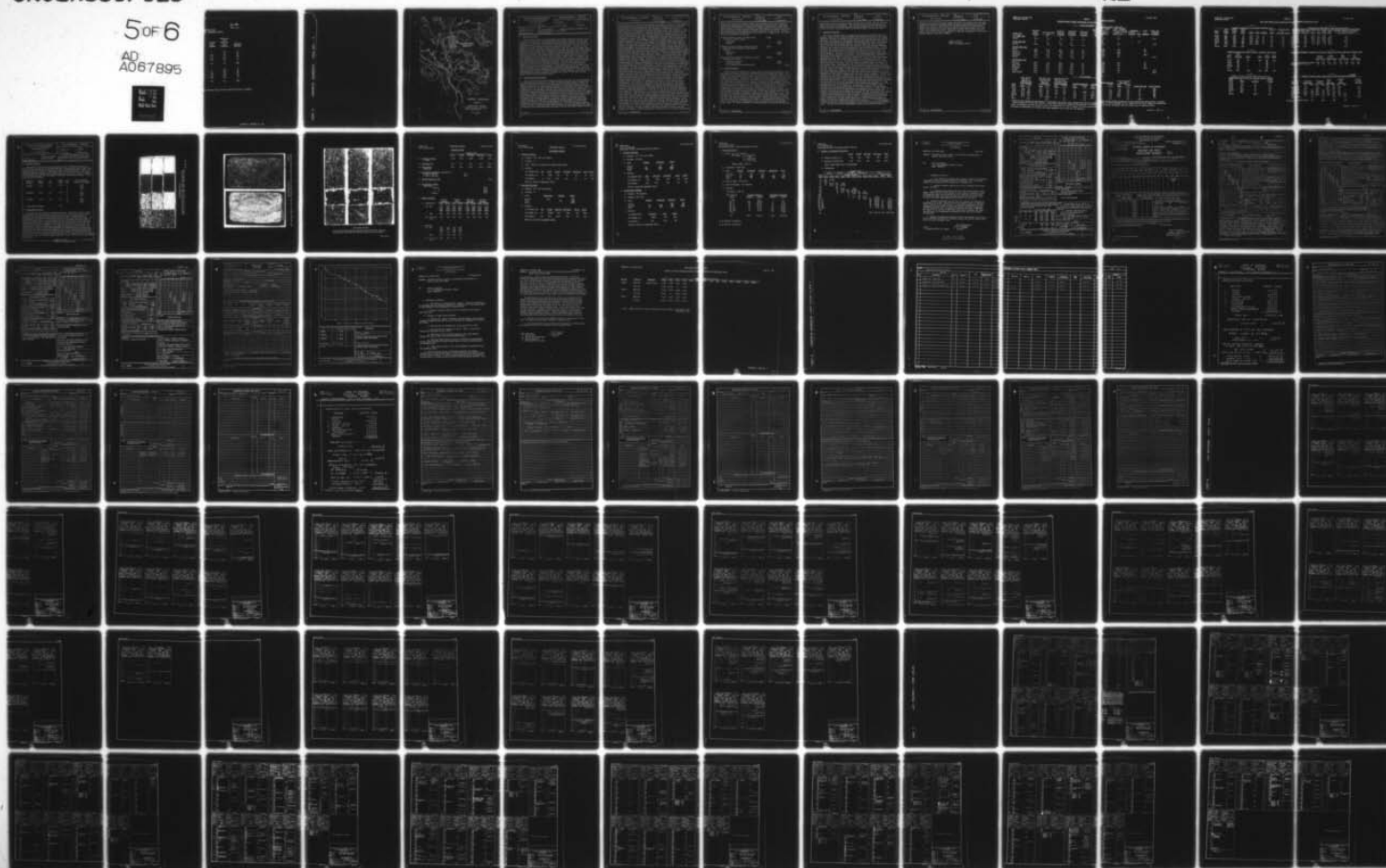
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5 of 6

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NL



AUG 19 1965

PAGE 2 OF 2

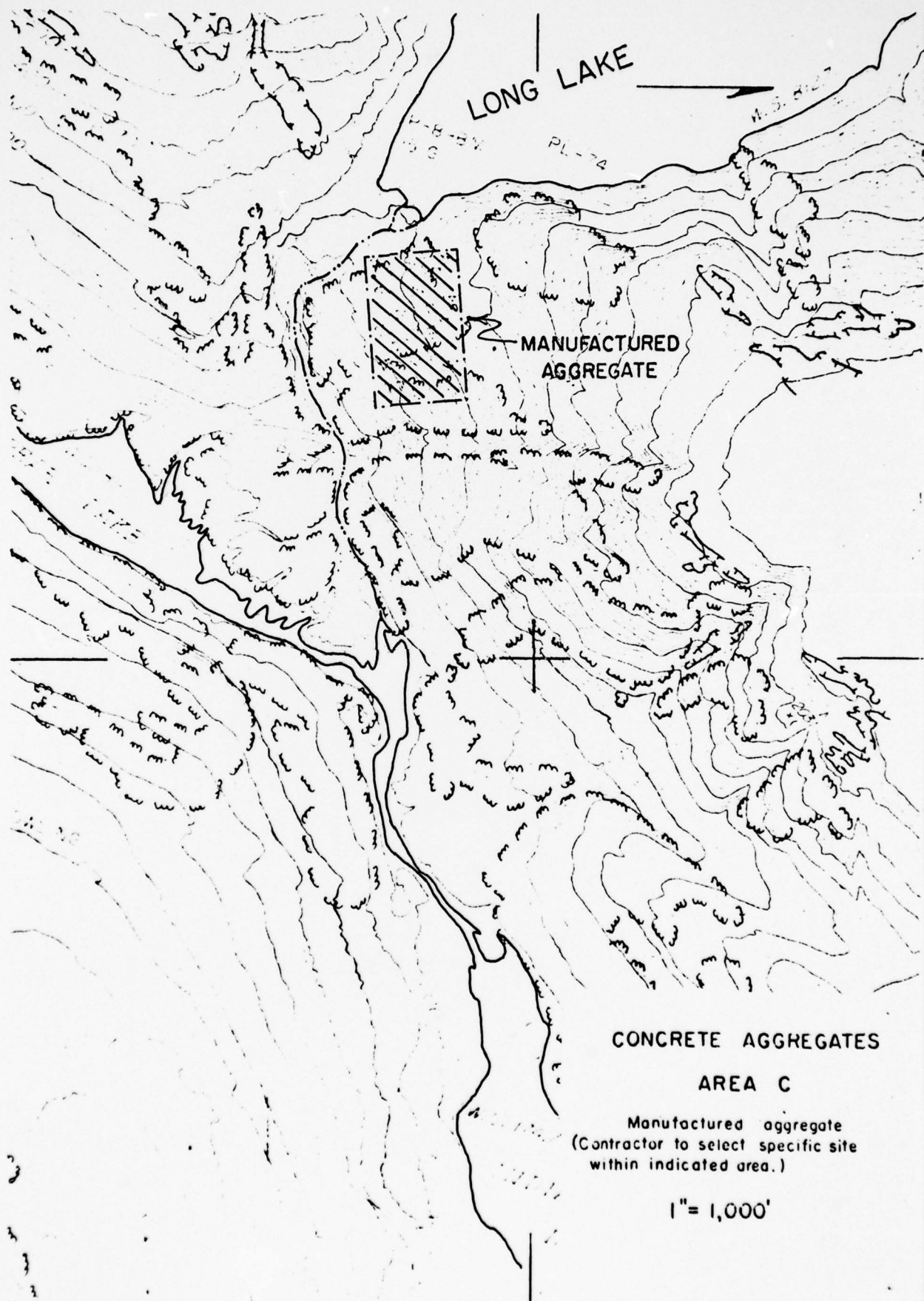
NATURAL SANDS

ES, SNETTISHAM PROJECT

<u>Discrete Mica Flakes</u>	<u>Moderately Well Weathered and Stained</u>	<u>Percent *<!--<br/-->Retained</u>
-	1.5	15
-	1.1	16
0.2	1.2	24
1.2	1.0	24
2.1	0.5	16
0.6	0.1	5
4.1	5.4	100%
-	0.4	15
-	0.5	16
0.3	0.5	24
1.2	0.5	24
1.6	0.3	16
0.5	0.2	5
3.6	2.4	100%

Percent retained range for each screen size given in standard

PART 4 CONCRETE AGGREGATE - TRIAL MIX - C



LONG LAKE

PL-74

MANUFACTURED
AGGREGATE

CONCRETE AGGREGATES

AREA C

Manufactured aggregate
(Contractor to select specific site
within indicated area.)

1" = 1,000'

CORPS OF ENGINEERS, U.S. ARMY OFFICE OF THE DIVISION ENGINEER NORTH PACIFIC DIVISION 500 PITTECK BLOCK PORTLAND 5, OREGON NPD TESTING LABORATORY		PETROGRAPHIC REPORT	
DATE 30 Apr 65		EXPL. NO. SAMPLE NO. NPD Lab No. 12937	
PETROGRAPHER N. B. Higgs		W/O NO. 64-CP-188	
SAMPLED BY District Personnel		PROJECT Long Lake Damsite, Surface Exploratory Quarry, Long	
SUBMITTED BY (DIST OR AGENCY) Alaska District		Snettisham Project, Lake Damsite Juneau, Alaska	

1. Samples and Tests

Approximately 3600 pounds (six barrels) of blasted ledge rock from the above source was submitted for processing into nominal sizes of coarse aggregate and rod-mill sand for various elementary tests, freeze-thaw soundness, petrographic examination to include a study of mica in the rod-mill sand and mortar tests for evaluation of the influence of mica on compressive strength. Selected NX core samples of similar gneissoid quartz diorite rock from the island and right abutment sources were previously examined and the results are detailed in Petrographic Report dated 12 November 1964. Although of somewhat different origin, the gneissoid quartz diorite from Long Lake has a remarkable resemblance both in texture and in composition to the quartz diorite gneiss or granite gneiss rock found in the vicinity of Dworshak Dam (N. Fork Clearwater River) Idaho. The gneiss rock at Dworshak has been extensively investigated for use as concrete aggregate and, for comparison purposes, inclosed are two WES Forms 726 from TM 6-370 containing test data relative to the Dworshak aggregates. The Long Lake aggregates were examined by CRD C-119 and CRD C-120, respective tests for flat and elongate particles in coarse and fine aggregate. Test procedures used in studies of mica in the rod-mill sand are given below in paragraphs pertinent thereto. Attached are: (1) Table I giving weighted average percent constituents in fine and coarse aggregate and (2) Table II containing results of mica studies. Also inclosed are two photographs showing sink-float separates and blends of high and low mica sands used for mortar strength studies.

2. Composition and Mica Study

Except for about 27% discrete-quartz-feldspar particles, the coarse aggregate material consists predominantly of medium grained gneissoid quartz diorite or a granite rock having a foliated structure and having characteristics similar to those described in the petrographic examination of previously submitted NX core samples. That is, the major rock type present is gneissoid quartz diorite having a slightly to well foliated structure with randomly alternating, subparallel, light and dark colored bands or lenticles containing varying proportions of quartz, feldspar, hornblende and biotite mica minerals (see photograph to previous report). The white or light-colored bands, patches or lenticles in the quartz diorite consist essentially of equidimensional quartz and feldspar with minor biotite-hornblende dark minerals; whereas, the darker colored of speckled bands have greater amounts of hornblende and biotite in reduced amounts of light-colored quartz and feldspar. The bulk sample differs in composition from previously examined NX core materials in the following ways. (1) The bulk sample contains greater amounts of quartz-feldspar rock in the form of fine-grained aplitic material derived from large thick quartz-feldspar folia in the quartz diorite and from

PETROGRAPHIC REPORT (CONTINUED)	DATE	EXPL. NO.	SAMPLE NO. NPD Lab No.
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aplite dikes such as were observed cutting the foliation in plus 3" hand samples; (2) Dark-colored rock facies, fine grained hornblende granulite and gneissoid coarse grained biotite-hornblende rock, observed as samples in the NX core material were not recognized as discrete particles in the bulk material. A few small xenoliths or inclusions of the dark granulite facies were observed in some of the larger chunks, however. Previous thin section studies of the NX core samples indicated that the quartz diorite contains an estimated average of 18% dark minerals (13% biotite mica, 5% hornblende); 79% light-colored minerals (60% plagioclase feldspar, 15% quartz, 4% orthoclase feldspar); and 3% miscellaneous accessory minerals. Quartz feldspar folia or the white bands in the NX core samples contained from five to ten percent dark minerals consisting largely of biotite with only minor hornblende. Mineral composition of the bulk aggregate sample as obtained from particle counts of the rod-mill sand is shown in Table I. The fine aggregate, by particle count, contains weighted average percentages of about 56% discrete and rock particle light-colored quartz-feldspar minerals, 28% quartz-feldspar rock particles with minor dark minerals, 2% dark biotite-hornblende rock particles with minor light felsic minerals, 3% discrete dark prismatic hornblende, and 11% discrete dark biotite mica flakes. Total mica-hornblende content, using average estimates of these minerals in the dark rock particles and in the speckled dark quartz-feldspar rock particles, is between 16 and 18 percent or slightly less than the amounts of these minerals found by thin section studies of the NX core samples. Under heading No. 1 of Table II are given the weighted average percent of biotite-hornblende minerals that were removed on a weight basis by 2.86 bromoform sink-float separations performed on 24,000 grams of representative material screened in a Ty-Lab shaker to provide the amounts per screen fraction shown. The total weighted average percent of biotite-hornblende sinks is 10.8% and is less than the amounts found by grain counts for two reasons: (1) The bromoform separates are on a weight basis, whereas, the particle counts are on a volume basis; (2) Sink-float specific gravity separations do not completely separate heavier biotite and hornblende particles from lighter quartz and feldspar particles because minor amounts of biotite and hornblende are entrapped in the float quartz-feldspar particles. Most of the discrete biotite and hornblende grains, either by weight or by volume, are found in the minus No. 16 sieve material where rock grains are sufficiently divided so as to release mineral constituents. The sink-float separates on the rod-mill sand are, therefore, indicative of total amounts of discrete biotite-hornblende plus heavy biotite-hornblende rock particles rather than total amount of biotite and hornblende present. Data under heading No. 5 contains the results of sink-float separates made on material in which the No. 8, No. 16, No. 30 and No. 50 sieve fractions were ground to a fine powder so as to release entrapped biotite and hornblende heavy minerals. This resulted in an increase in the percent sinks per these sieve sizes and a total increase in weighted average percent sinks from 10.8% to 13.8%. Thus, total amounts of biotite and hornblende are between 16%-18% by volume grain counts and about 14% by sink float separates of finely ground material. Grain counts were also made of the heavy separates from the original rod-mill sand and the results are contained under heading No. 6 of Table II. The sinks, using combined weight and volume count parameters, contain 17% discrete hornblende, 62% discrete biotite, 18% mixed biotite-hornblende dark particles and 3% of accessory heavies consisting largely of epidote with minor amounts of zircon, apatite, sphene, and opaques. Recalculating these percentages so as to exclude accessories

PETROGRAPHIC REPORT

(CONTINUED)

DATE 30 Apr 65	EXPL. NO.	SAMPLE NO. 12937	NPD Lab No.
PETROGRAPHER N. B. Higgs	W/O NO 64-CP-188		

along with estimates of percent biotite and hornblende in dark rock particles give a total composition of about 22% hornblende and 78% biotite. Using these percentages of hornblende to biotite, the original rod-mill sand without subsequent grinding contains 2.4% hornblende and 8.4% biotite on a weight basis; while the rod-mill sand after grinding to release particles in the plus No. 50 sieve fractions contains 3% hornblende and 10.8% biotite on a weight basis. The above data is recapitulated as follows:

Weighted average % sinks on weight basis in natural rod-mill sand.	10.8%
Hornblende	2.4%
Biotite	8.4%
Weighted average % sinks on weight basis in rod-mill sand with plus No. 50 material reground	13.8%
Hornblende	3.0%
Biotite	10.8%
Total biotite-hornblende in original rod-mill sand by grain count.	16-18%
Discrete hornblende	3.0%
Discrete biotite	11.0%
Total discrete biotite-hornblende	14.0%

3. Mortar Strengths

Biotite mica is a mineral that can be flaked along numerous potential cleavage planes of weakness to thinner subdivided particles having flat shapes. The amount of discrete mica flakes formed is dependent upon interlock of this mineral in the rock mass and the type of processing used to make aggregate from rock containing this mineral. The manner of occurrence of this mineral in aggregates is the significant feature; that is, the number of discrete mica flakes present in the sand rather than mica held in aggregate particles has the greatest influence relative to mortar strengths. In the rod-mill sand made in this laboratory, most of the discrete mica flakes occur in the material passing the No. 16 sieve and comprise about 11% of sample by count. Because of the occurrence of this potentially weak mineral in the Long Lake aggregate three series of three mortar cubes containing sands with natural, high, and low mica contents were made from the sink-float separate fractions in amounts shown under heading No. 1, 2, and 3 of Table II. The high mica content cubes contain blended sand with 48.2% sinks and about 30% discrete biotite. Low mica cubes contain 100% floats, no sinks, and have only minor traces of discrete biotite. Results of the mortar strength tests are shown under heading No. 4 of Table II and indicate the following: (1) Mortar strengths for all combinations of mica are greater than with standard Ottawa sand. (2) Mortar strengths of cubes containing low mica content sand are

PETROGRAPHIC REPORT (CONTINUED)	DATE 30 Apr 65	EXPL NO. SAMPLE NO NPD Lab No. 12937
	PETROGRAPHER N. B. Higgs	W/O NO 64-CP-188

greater than those with either natural or high mica contents, showing that mica does influence mortar strengths. (3) Both the natural and high mica content mortar cubes have essentially the same strengths indicating that discrete mica at the 11% level has as much effect on mortar strength as at the 30% level.

4. Physical Properties

The coarse aggregate ranks as moderately weathered and contains about 52% slightly weathered and stained particles, 46% moderately weathered and stained particles and about 2% moderately well weathered and heavily iron oxide stained particles. Examination of rock chunks prior to crushing indicated that most of the alteration present is attributable to weathering along joints with interior of chunks being only slightly altered. Soft particles that can be scratched with a brass rod by CRD C-130 were not observed. Thin and elongate particles are minor and amount to about two percent of coarse aggregate sample. However, particles with significant open megafactures are present in the larger sized particles and comprise about 21% of the 6"-3" material and about 13% of the 3"-1½" material. The 1½"-3/4" and 3/4"-#4 nominal sizes contained only a few particles having fractures of this type. Soft and attritionable constituents in the rod-mill sand amount to about 3% by the Red Devil Shaker test and from examination of the wash water consist largely of a slurry of minute mica flakes. Particle counts of the No. 100 sieve material after shaking showed about 23% discrete mica flakes, or an increase of about 4% in the amounts found in this sieve size prior to shaking. Flat and elongate particles amount to about 9% of the rod-mill sand. These particles consist largely of discrete mica flakes in the minus No. 16 sieve material and about equal proportions of feldspar fragments and mica flakes in material retained on this sieve. Alkali-reactive constituents were not observed in sample. Small crystals of pyrite were observed in a few of the coarse aggregate particles, but form only trace amounts in sample. Physical properties of the foliated granite rock from Long Lake are largely determined by textures and structures. Specifically, two items warrant attention: (1) Possible splitting of rock along foliation planes and (2) granulation because of poor interlock between constituents and/or the presence of cleavable feldspar, biotite and hornblende minerals. In the sample examined, splitting along foliation planes does not appear to be a problem because flat particles are minor and less than one-quarter of the fractured particles in the larger coarse aggregate sizes had fractured parallel to the foliation. Granulation of this material during processing and subsequent mixing may be a problem, however. The Los Angeles abrasion test loss after 500 revolutions at A grading is 52%. Granite rocks are known to be subject to granulation and the following quote from Blanks and Kennedy in *The Technology of Cement and Concrete* (p. 285) illustrates the problem: "Highly siliceous igneous rock such as granite, crystallized progressively, so that the last-formed mineral (usually quartz) is molded against the relatively smooth surfaces of the early formed crystals. Consequently, the texture is poorly interlocked and granulation occurs easily, the low strength being most evident in particles comprising relatively few crystals . . .". Mixer grinding studies should show whether granulation is a problem with the Long Lake granite rock. As noted in the prior petrographic report, mixer grinding studies of

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coarse to medium grained granite gneiss aggregate from Dworshak Dam have shown significant changes in fineness modulus of charged aggregates before and after mixing. For example, the reduction in fineness modulus of Dworshak granite gneiss fine aggregate based upon nine mixer grinding tests ranged from 7.2 to 27.8%, the average being 15.9%. Maximum Los Angeles abrasion loss found for tests on the Dworshak coarse aggregate was about 39% as compared to 52% for the present sample of Long Lake granite.

Nelson B. Higgs
 NELSON B. HIGGS
 Chief, Petrography Branch

TABLE I

WEIGHTED AVERAGE PERCENT CONSTITUENTS IN FINE AND

Screen Size	Coarse Aggregate ^{3/}					Tabular From 3:1 Th
	Gneissoid Quartz Diorite	Quartz-Feldspar Rock	Slightly Weathered & Stained	Moderately Weathered & Stained	Moderately Well Weath. & Stained	
6"-3" Nom. Size	64	36	41	56	3	
Total ^{1/}	14.5	8.2	9.3	12.7	0.7	
3"-1 1/2" Nom. Size	77	23	41	57	2	
Total ^{1/}	17.6	5.3	9.4	13.1	0.4	
1 1/2"-3/4" Nom. Size						
1 1/2"-1"	33.9	12.1	24.1	21.3	0.6	
1"-3/4"	35.0	13.0	25.7	21.5	0.8	
Minus 3/4"	4.5	1.5	3.2	2.7	0.1	
Subtotal ^{1/}	73.4	26.6	53.0	45.5	1.5	
Total ^{1/}	11.2	4.0	8.1	6.9	0.2	
3/4"-#4 Nom. Size						
Plus 1/2"	17.4	6.6	14.9	8.5	0.6	
1/2"-3/8"	16.1	4.9	13.9	6.9	0.2	
Minus 3/8"	42.3	12.7	36.4	18.1	0.5	
Subtotal ^{1/}	75.8	24.2	65.2	33.5	1.3	
Total ^{1/}	29.7	9.5	25.6	13.1	0.5	
Grand Total ^{1/}	73.0	27.0	52.4	45.8	1.8	

Fine Aggregate

Sieve Size	Light Colored Qtz-Feld. Minerals and Rock Particles		Qtz-Feld. Rock Particles with Minor Dark Minerals		Dark Biot-Horn. Rock Particles with with Minor Light Colored Minerals		Discrete Hornblende		Discrete
	% of Sieve	Wt. Avg. %	% of Sieve	Wt. Avg. %	% of Sieve	Wt. Avg. %	% of Sieve	Wt. Avg. %	% of Sieve
#4-#8	27.2	2.2	71.6	5.7	1.2	0.1	-		-
8-16	34.0	6.1	63.2	2.4	2.4	0.5	0.2	trace	0.2
16-30	59.2	16.0	33.2	9.0	2.0	0.5	0.4	0.1	5.2
30-50	64.2	14.7	9.5	2.2	3.0	0.7	4.8	1.1	18.5
50-100	70.3	10.5	2.0	0.3	-		8.0	1.2	19.7
Pan	68.7	6.2	0.6	trace	1.7	0.2	6.3	0.6	22.7
Total		55.7		28.6		2.0		3.0	

^{1/}Subtotals give weighted average percent of constituents per nominal size. Amounts under total are weighted sizes recalculated to omit sand fraction. Grand total figures give weighted average percent of constituents.

^{2/}Flat and elongate particles consist of about 50% feldspar on No. 8 and 16 sieves, 74% biotite mica on No. 100 sieve.

^{3/}Percent of constituents is by weight for coarse aggregate and by count for fine aggregate.

30 April 1965

COARSE AGGREGATE

Particle Shapes - CRD C-119

r & Wedge 2:1 to Width to Thickness	Flat & Elongate Width to Thickness & Length to Width Ratio > 3:1	Fractured		% Ret. in Nom. Size	% Nom. Size Retained in Sample
		Parallel Foliation	Total		
0	4	5	21		
4.5	0.9				22.7
5	2	3	13		
3.4	0.5				22.9
2.6	0.2			46	
4.8	0.3			48	
0.6	trace			6	
8.0	0.5			100.0	
1.2	0.1				15.2
4.6	0.1			24	
1.9	0.4			21	
5.0	1.1			55	
1.5	1.6			100.0	
4.5	0.6				39.2
3.6	2.1				100.0

Flat & Elongate ^{2/} CRD C-120				
Biotite Mica				
Wt. Avg. %	% of Sieve	Wt. Avg. %	Percent Retained	Grains Counted
	2.8	0.2	8	500
trace	1.6	0.3	18	500
1.4	5.4	1.5	27	500
4.3	11.8	2.7	23	400
3.0	15.7	2.4	15	300
2.0	22.7	2.0	9	300
10.7		9.1	100.0	

ated average percent constituents of this nominal size using percent of nominal
ents in coarse aggregate and is a summation of the total figures per nominal size.
30 Sieve, and greater than 90% biotite mica on remaining sieves.

APPENDIX C, PART IV, C

2

MICA STUDY-MORTAR MAKING PROPERTIES WITH DIF

1

Percent of Sink-Floats in Original Rod-Milled Sand									
Sieve Size	Total Weight Grams	Weight Floats Grams	Weight Sinks Grams	% Floats	% Sinks	% Retained	Wt.Avg. % Sinks	% Sinks Using Wt. of Sink-Float Sample	(48) Sg
#4-#8	3600	3547	53	98.5	1.5	8	0.1	0.2	#4-#8
8-16	4800	4644	156	96.7	3.3	18	0.6	0.6	8-16
16-30	4800	4503	297	93.8	6.2	27	1.7	1.2	16-30
30-50	2700	4814	946	83.6	16.4	23	3.8	3.9	30-50
50-100	3360	2756	604	82.0	18.0	15	2.7	2.5	50-100
Pan	1680	1330	350	79.2	20.8	9	1.9	1.5	Pan
Totals	24000	21694	2406				10.8	9.9	Totals

3

Proportions of Low Mica Mortar Cubes - No Sinks & 100% Floats

Sieve Size	Total Grams Required	% Floats	% Sinks	Grams Floats	Grams Sinks
#4-#8	450	100	Zero	450	Zero
8-16	600	"	"	600	"
16-30	600	"	"	600	"
30-50	720	"	"	720	"
50-100	420	"	"	420	"
Pan	210	"	"	210	"
Totals	3000	100.0	Zero	3000	Zero

5

Percent of Sinks in Natural Sand After Sink-Float Separations of No. 8 thru No. 50 Sizes Ground to Powder

Sieve Size	% Sinks	% Retained	Wt.Avg. % Sinks
#4-#8	8.2	8	0.7
8-16	8.4	18	1.5
16-30	11.1	27	3.0
30-50	17.2	23	4.0
50-100	18.0	15	2.7
Pan	20.8	9	1.9
Totals		100.0	13.8

Wt.Avg. +

30 April 1965

DIFFERENT PROPORTIONS OF MICA

2

Proportions of Sink-Floats in High Mica Mortar Cubes

<u>2.2% sinks or 4.8 times amount of sinks per sieve size of original rod-mill sand)</u>						
<u>Level</u>	<u>Total Grams</u>	<u>%</u>	<u>%</u>	<u>Grams</u>	<u>Grams</u>	<u>Percent Sinks in Total Sample</u>
<u>Size</u>	<u>Required</u>	<u>Floats</u>	<u>Sinks</u>	<u>Floats</u>	<u>Sinks</u>	<u>per Screen Fraction</u>
#8	450	92.8	7.2	417.6	32.4	1.1
#16	600	84.2	15.8	505.2	94.8	3.2
#30	600	70.3	29.7	421.8	178.2	5.9
#50	720	21.3	78.7	153.4	566.6	18.9
#100	420	13.6	86.4	57.1	362.9	12.1
	210	0.2	99.8	0.0	210.0	7.0
als	3000					48.2

4

Mortar Strengths: Standard Ottawa-Natural, Heavy and Low Mica Snettisham Rod-Mill Sand

	<u>Standard</u>		<u>1</u>		<u>2</u>		<u>3</u>	
	<u>Ottawa</u>		<u>Natural</u>		<u>High</u>		<u>Low</u>	
	<u>7-Day</u>	<u>28-Day</u>	<u>7-Day</u>	<u>28-Day</u>	<u>7-Day</u>	<u>28-Day</u>	<u>7-Day</u>	<u>28-Day</u>
Compressive Strength	1540	2950	1740	3240	1775	2985	2220	3800
Percent of Standard	100	100	113	110	115	101	144	129

6

Red-Mill

Weighted Average Percent Constituents in Sink Separates Natural/Sand

ve a	% Discrete Hornblende		% Discrete Biotite		Mixed Dark Particles		Epidote and Other		Wt.Avg. % Sinks Retained
	% F	Wt.%	% F	Wt.%	% F	Wt.%	% F	Wt.%	
#8	4.0		6.0		85.0	0.1	5.0		0.1
16	5.9		8.0		81.3	0.6	4.7		0.6
30	10.7	0.2	51.3	0.9	36.3	0.6	1.7		1.7
50	19.0	0.8	69.0	2.6	11.3	0.4	0.7		3.8
100	16.0	0.5	75.0	2.0	4.0	0.1	5.0	0.1	2.7
	18.0	0.4	67.0	1.2	6.0	0.1	9.0	0.2	1.9
als		1.9		6.7		1.9		0.3	10.8
Total Wt.Avg.	17%		62%		18%		3%		

APPENDIX C, PART IV, C

CORPS OF ENGINEERS, U.S. ARMY OFFICE OF THE DIVISION ENGINEER NORTH PACIFIC DIVISION 500 PITtock BLOCK PORTLAND 5, OREGON NPD TESTING LABORATORY		PETROGRAPHIC REPORT	
DATE 30 Apr 65		EXPL. NO. C-13002	
PETROGRAPHER N. B. Higgs		W/O NO. 64-CPCh-188	
SAMPLED BY SUBMITTED BY (DIST OR AGENCY) Alaska District	PROJECT Snettisham Project Juneau, Alaska		
		SOURCE FREEZE-THAW BEAMS Prepared with Manufactured 3/4"-#4 Coarse Aggregate and Rod-Mill Sand from Granite Ledge Rock	

1. Samples and Tests

One series of freeze-thaw beams was prepared with laboratory manufactured 3/4"-#4 coarse aggregate and rod-mill sand made from gneissoid granite ledge rock obtained from the vicinity of Long Lake Damsite, Snettisham Project, Alaska. The series of beams was tested in accordance with test method CRD C-114. All beams contained 44% fine aggregate, had a water-cement ratio of 0.49, and were retained in the freeze-thaw unit for 300 cycles of alternate freezing and thawing. Other test data are as follows:

<u>Series & Set No.</u>	<u>Slump, Inches</u>	<u>% Air</u>	<u>DFE₃₀₀ Avg.</u>	<u>Beam Nos.</u>	<u>Rel. E of Each Beam at 300 Cycles</u>
C-13002	2.00	6.2	70.2	1	72.3
				2	70.6
				3	67.7
C-13002A	2.25	6.0	66.1	4	65.9
				5	64.2
				6	68.2
C-13002B	2.25	6.4	65.7	7	64.5
				8	68.0
				9	64.5
Grand Average DFE ₃₀₀			67.3		

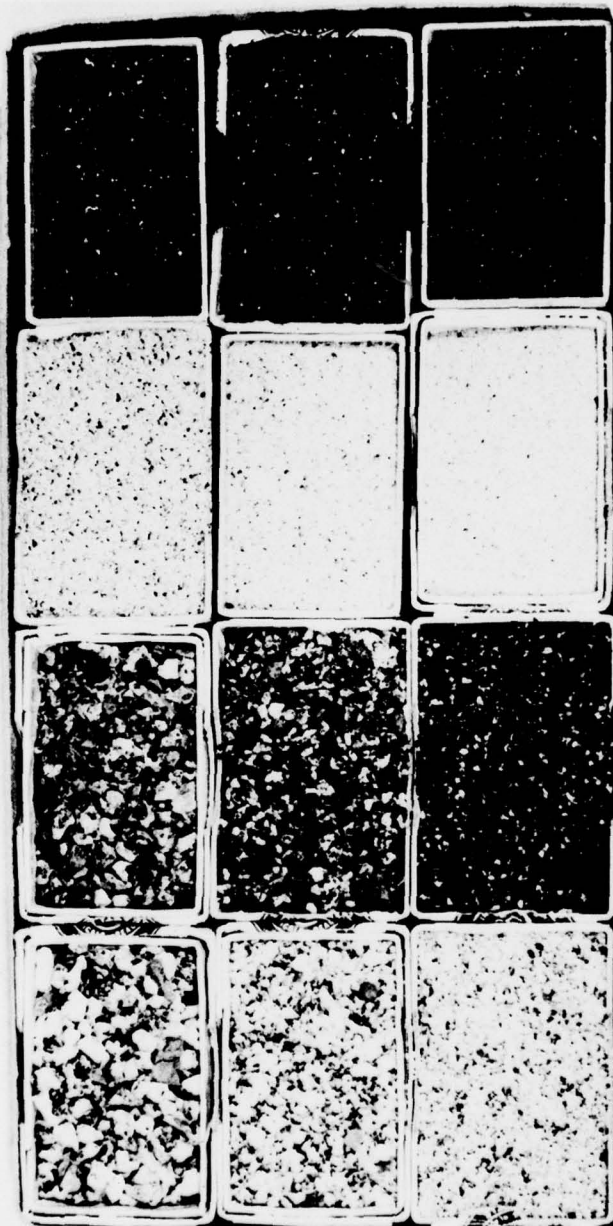
2. Examination of Beams

All beams have well preserved original contours. Beams No. 5 and No. 6 have lost corners to respective depths of one-half and one inch, otherwise the beam surfaces show only minor mortar-coarse aggregate losses. Surface relief of projecting coarse aggregate particles averages less than one-sixteenth inch. The mortar is hard and firm. Surface coarse aggregate particles when pried with a dull dental probe tend to granulate rather than break from the mortar indicating that the mortar-coarse aggregate bond is good and greater than the abrasion resistance of the granite particles. Air voids are normal in sizes, amounts and distribution. There is no evidence of aggregate segregation, or bleeding. The moderate depths of penetration of destructive freeze-thaw action are illustrated by the dark wet edges of the sawed sections shown in the accompanying photograph. The slices were cut one-half inch thick from third points of selected beams, air dried, then placed in water one-fourth inch deep for 15 minutes to allow for capillary rise of moisture through ruptured peripheral mortar, and were then photographed immediately.

NPD FORM 306
22 MAR 64

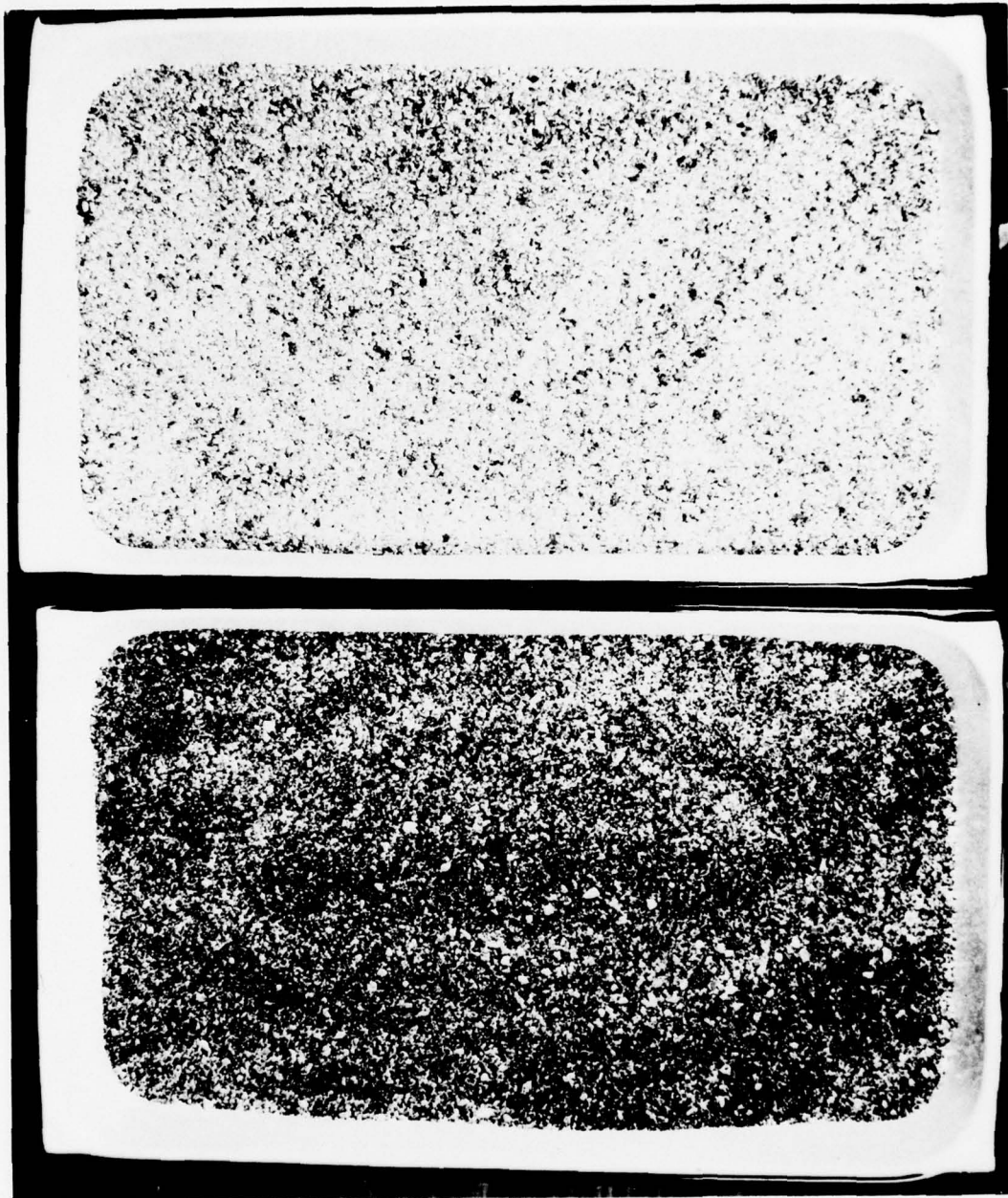
NELSON B. HIGGS
Chief, Petrography Branch

SHEET 1 OF 1



Snettisham Project

Samples of Dark Sink and Light Float Separates from
No. 8, 16, 30, 50, 100 and Pan Sizes of Rod-Mill Sand. 30 Apr. 65



Snettisham Project

30 Apr 65

Left and Right, High and Low Mica Blended Rod-Mill Sands for Mortar Strength Studies.



Snettisham Project

Freeze-Thaw Beams Prepared with Manufactured Coarse Aggregate
and Rod-Mill Sand Made from Granite Ledge Rock Bulk Sample.

30 Apr. 65

NPDEN-GS-L
W. O. 64-CPCh-188

SNETTISHAM PROJECT

30 April 1965

Aggregate Tests

	Nominal Size				
	6"-3"	3"-1½"	1½"-¾"	¾"-No. 4	Sand
1. <u>Specific Gravity</u> (BSSD)	2.68	2.66	2.65	2.61	2.65
2. <u>Absorption, %</u>	0.6	0.7	0.4	1.4	0.5
3. <u>Unit Weight</u> lb/cu.ft.	93.9	94.7	92.0	90.3	102.0
4. <u>Los Angeles Abrasion, (Grading "A")</u>					
% wear @ 100 Rev.			13.1		
% wear @ 500 Rev.			51.6		
5. <u>Percent Minus No. 200</u>					0.6
6. <u>Freeze-Thaw, DFE300</u> 300 cycles					
Round 1					70.2
2					66.1
3					65.7
Average					67.3

7. Mortar Strength

	Ottawa Standard		Rodmill Sand		High Mica Content		Low Mica Content	
	7-Day	28-Day	7-Day	28-Day	7-Day	28-Day	7-Day	28-Day
a. Compressive								
(1) PSI	1525	3050	1860	2950	1810	3000	2475	3725
	1610	2750	1620	3600	1740	2925	2005	3600
	1490	3050	1745	3175	1770	3025	2185	4050
Avg.	1540	2950	1740	3240	1775	2985	2220	3800
(2) % of con- trol	100	100	113	110	115	101	144	129
b. Flexural								
(1) PSI	505	755	325	610				
	470	830	360	650				
	430	-	360	705				
Avg.	470	795	350	655				
(2) % of con- trol	100	100	74	82				

Inc. 1'

NPDEN-GS-L
W. O. 64-CPCh-188

SNETTISHAM PROJECT

29 January 1965

Processing Studies

1. Primary Crushing

- a. Crusher: 15" X 24" jaw crusher
- b. Setting: 6"
- c. Feed: 1173 lb. of quarry run (large pieces only)
- d. Product:

(1) Nominal Size	<u>+6"</u>	<u>6"-3"</u>	<u>3"-1½"</u>	<u>1½"-3/4"</u>	<u>3/4"-No.4</u>	<u>-No.4</u>	<u>Total</u>
(2) Weight, lb.	0	581*	421	74	65	32	1173
(3) Percent	0	49.6	35.9	6.3	5.5	2.7	100

*451 Lb. saved for aggregate stock.

2. Secondary Crushing

- a. Crusher: 15" X 24" jaw crusher
- b. Setting: 3"

c. Feed:	<u>Quarry Run</u>	<u>Primary</u>	<u>Total</u>
Plus 6"	2344		2344
6"-3"		130	130
Total			2474

- d. Product:

(1) Nominal Size	<u>+3"</u>	<u>3"-1½"</u>	<u>1½"-3/4"</u>	<u>3/4"-No.4</u>	<u>-No. 4</u>	<u>Total</u>
(2) Weight, lb.	65	1338**	440	305	196	2344
(3) Percent	2.8	57.0	18.8	13.0	8.4	100

**455 lb. saved for aggregate stock.

3. Tertiary Crushing

a. Crusher: 15" X 24" jaw crusher

b. Setting: 1½" Max.

c. Feed:	<u>Primary</u>	<u>Secondary</u>	<u>Total</u>
Plus 3"		65	65
3"-1½"	<u>421</u>	<u>883</u>	<u>1304</u>
Total	421	948	1369

d. Product:

(1) Nominal Size:	<u>+1½"</u>	<u>1½"-3/4"</u>	<u>3/4"-No.4</u>	<u>-No.4</u>	<u>Total</u>
(2) Weight, lb.	160	720*	330	129	1339
(3) Percent	12.0	53.8	24.6	9.6	100

*300 lb. saved for aggregate stock.

4. Quarterternary Crushing

a. Crusher: 18" gyratory

b. Setting: 3/4" Max.

c. Feed:	<u>Primary</u>	<u>Secondary</u>	<u>Tertiary</u>	<u>Total</u>
3"-1½"			160	160
1½"-3/4"	<u>74</u>	<u>440</u>	<u>420</u>	<u>934</u>
Total	74	440	580	1094

d. Product:

(1) Nominal Size	<u>3/4"-No.4</u>	<u>-No.4</u>	<u>Total</u>
(2) Weight, lb.	780**	285	1065
(3) Percent	73.2	26.8	100

**780 lb. saved for aggregate stock.

NPDEN-GS-L

W. O. 64-CPCh-188

Snettisham Project Processing Studies (Cont'd)

29 January 1965

5. Sand Manufacture:

a. Mill: 16" X 32" rodmill

Rod charge 7 - 2-3/8" dia.
 9 - 2" dia.
 3 - 1-5/8" dia.
 3 - 1-3/8" dia.

Charge weight 600 lb.

b. Classifier: Rake Type

c. Feed:	<u>Primary</u>	<u>Secondary</u>	<u>Tertiary</u>	<u>Quaternary</u>	<u>Total</u>
3/4"-No. 4	65	305	330	-	700
-No. 4	<u>32</u>	<u>196</u>	<u>129</u>	<u>285</u>	<u>642</u>
Total	97	501	459	285	1342

d. Rate of Feed: 1550 lb/hr.

e. Loss in Milling: 13.4 percent

f. Product:

(1) Weight: 1162 lb.

(2) Gradation Size	<u>Percent Passing</u>		<u>Ind. Percent Retained</u>	
	<u>Product</u>	<u>Specs^{1/}</u>	<u>Product</u>	<u>Specs^{2/}</u>
3/8"		100		0
No. 4	100	95-100	0	0-5
No. 8	92	80-90	8	5-20
No. 16	74	55-75	18	10-22
No. 30	47	30-60	27	18-32
No. 50	24	12-30	23	18-32
No. 100	9	2-10	15	10-25
Pan			9	5-10
FM	2.54	2.4-3.1	2.54	2.25-2.85

1/ CE 1401.01, Alternate 1

2/ CE 1401.01, Alternate 2

NPDEN-GS-L

29 January 1965

W. O. 64-CPCh-188

Snettisham Project Processing Studies (Cont'd)

6. Summary of Aggregate Production

	<u>6"-3"</u>	<u>3"-1½"</u>	<u>1½"-3/4"</u>	<u>3/4"-No. 4</u>	<u>Sand</u>
a. Weight produced, lb.	451	455	300	780	1162
b. Percent of Quarry Run:	13.2	13.3	8.8	22.8	34.0

Note: 10.5% (369#) loss in processing.

c. Gradations:

Screen Size	Nominal Size												Ind. % Ret.	
	6"-3"		3"-1½"		1½"-3/4"		3/4"-No. 4		Sand					
	Percent Passing													
	Prod.	Spec.	Prod.	Spec.	Prod.	Spec.	Prod.	Spec.	Prod.	Spec.	Prod.	Spec.		
6"	100	90-100												
5"	89													
4"	62	20-45	100	100										
3"	7	0-15	95	90-100										
2½"			87											
2"			61	20-55		100								
1½"			24	0-10	100	90-100								
1"			4	0-5	54	20-45	100	100						
3/4"			3		6	0-10	96	90-100						
1/2"					3		76							
3/8"					2	0-5	55	30-55		100				
No. 4							11	0-5	100	95-100			0-5	
No. 8									92	80-90	8		5-20	
No. 16									74	55-75	18		10-22	
No. 30									47	30-60	27		18-32	
No. 50									24	12-30	23		18-32	
No. 100									9	2-10	15		10-25	
Pan											9		5-10	
FM									2.54	2.4-3.1	2.54		2.25-2.85	

ADDRESS REPLY TO
DIRECTOR
(NOT TO INDIVIDUALS)

U. S. ARMY ENGINEER DIVISION, NORTH PACIFIC
CORPS OF ENGINEERS
NORTH PACIFIC DIVISION MATERIALS LABORATORY
RT. 2, BOX 12A
TROUTDALE, OREGON

NPDEN-GS-L (64-CPCh-188)

7 May 1965

SUBJECT: Snettisham Project, Report of Laboratory Investigations of
Granite Ledge Rock Sample

TO: District Engineer
U. S. Army Engineer District, Alaska
ATTN: NPAEN-FM

1. Reference is made to:

a. Your letter of 6 January 1965, subject, "Letter of Transmittal to Accompany Concrete Aggregate Materials" applicable to a 3,600 pound sample of rock received at this laboratory 22 December 1964.

b. Reports forwarded under cover of our 29 January 1965 letter, subject as above.

c. Telephone conversations 30 April 1965 between Messrs. Jones, Knoppe, Clarke and the writer wherein it was agreed to reschedule mortar bar expansion tests and to perform one trial concrete mix design using available rock with compressive strength specimens for three test ages (7, 28 and 90 days) using aggregate processed to 1½" top size.

2. Attached completing all work scheduled except that outlined in reference 1c above are: (Incl.1) A one page summary report of aggregate tests, (Incl.2) a five page report of petrographic examinations of the aggregate plus data sheets of referenced similar manufactured aggregate for NFW Dworshak Dam and two summary tabulations and (Incl.3) NPD Form 361 report of the results of CRD C-114 freeze-thaw soundness test with a one page report of petrographic examinations after test and one photo illustration.

3. Reports on remaining scheduled testing referenced in 1c above will, unless otherwise requested, be forwarded when 90 day results become available on or about 16 August 1965.

3 Incl
as (Reproducibles w/2 copies)

J. L. Bargar
J. L. BARGAR
Director

AIR MAIL

MIX DATA						
MATERIAL	% C.A.	SAMPLE NO.	SOLID VOLUME cu ft	WEIGHTS S.S.D. lb	BULK SP. GR. S.S.D.	% ABSORP.
Coarse Agg (A)						
" (B)						
" (C)	45	12937	5.366	887	2.65	0.4
" (D)	55	"	6.562	1069	2.61	1.4
Sand		"	6.768	1119	2.65	0.5
Cement		12895	1.674	114.0	3.15	
Pozzolan		12064	0.717	329.0	2.55	
Water			4.293	267.9		
Air (6.0 on-1/2)			1.620			
Totals			27.000	3786		

MIX CHARACTERISTICS		
WATER- CEMENT RATIO	Gallons per bag, equivalent cement	6.43
	By weight, "	0.57
	By weight, $\frac{\text{water}}{\text{cement} + \text{pozzolan}}$	0.605
Cement factor, bags/cu yd (Eq. Sol. Vol. as P.C.)		5.0
Pozzolan, % replacement by solid volume		30.0
Slump, inches (2 - 2 1/2)		2
Air content (Note 1), % (5.8 - 6.0)		5.9
Unit weight, lbs/cu ft		140.4
Bleeding (Note 2), %		5.5
Sand / aggregate, % by volume		38.0
Temperature of plastic concrete, °F		67

STRENGTH TEST DATA (Compressive)			
Age, days	3	7	28
Strength, p s i (average)	800	1120	2320
Age, days	90		
Strength, p s i (average)	3420		

NOTES:
 1. In that portion of the concrete containing aggregate smaller than the 1/2" - inch sieve.
 2. Percentage of mix water separating from concrete in bleeding test.

REMARKS Mix was proportioned to approximately a 3.25 sk, 6" max. mix with plus 1/2" aggregate removed.

Batch	Air	Slump	Comp. Str. PSI			
			3-Day	7-Day	28-Day	90-Day
1	5.8	2	790	1070	2320	3350
			800	1110	2300	3510
			760	1100	2280	3330
2	6.3	2 1/2	770	1060	2260	3230
			840	1150	2350	3560
3	5.8	2	850	1200	2380	3550
Pre-5.4 2 1/2				810	2140	3100
Soaked				950	1920	3030
Aggregate						
Avg.				880	2030	3070

SCREEN ANALYSES-% RETAINED							
SIEVE SIZE	NOMINAL SIZES				COMBINED		
	(A) 6" to 3"	(B) 3" to 1 1/2"	(C) 1 1/2" to 3/4"	(D) 3/4" to 1/2"	36.2 Sand	Coarse Aggregate	Total Aggregate
7"			28.7	35.1			
6"							
5"							
4"							
3"							
2 1/2"							
2"							
1 1/2"						0	0
1"			66			29.7	19.0
3/4"			28	9		47.3	30.2
1/2"			4	31		66.1	42.2
3/8"				25		79.8	51.0
4			1	30		96.7	62.0
8			1	5	8		65.2
16					18		72.3
30					27		83.0
50					23		92.0
100					15		98.0
PAN					9	100.0	100.0
F.M.					2.54		

PROJECT		SNETTISHAM PROJECT	
DISTRICT		Alaska	
FINE AGGREGATE		Lab-produced granite sand (Rodmill product)	
COARSE AGGREGATE		Lab-crushed granite	
CEMENT		Blended Type II, 1A. Blend of equal parts Ideal (Gold Hill), Oregon (Lake-Oswego) & Permanente (Bellingham)	
POZZOLAN		Idealite (Calcined Shale) Great Western Aggregates Co. Laramie, Wyoming	
A.E.A. Lab Stock NVX (Lab No. 13076)		310 cc/cu.yd.	
OTHER ADMIXTURE		None	
MIX NO. 13114		W/O NO.	
CAST 5/14/65		64-CP-188	
AUG 20 1965 (Date of Report)		D. L. Houghton Chief, Concrete Branch	

NPD RF 359 APRIL 61		REPORT OF CONCRETE MIXTURE DESIGN METHODS CRD-C 3 AND 10 CORPS OF ENGINEERS NORTH PACIFIC DIVISION TESTING LABORATORY	
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Room Dry
Aggregate

Aggregate

INCL 1'

U. S. DEPARTMENT OF COMMERCE
NATIONAL BUREAU OF STANDARDS
WASHINGTON, D.C. 20234

North Pacific Division Laboratory
Attention: Mr. J. L. Karger, Director
Corps of Engineers
Route 2, Box 12A
Troutdale, Oregon

Seattle, Washington 98107
October 29, 1964

NATIONAL BUREAU OF STANDARDS

REPORT OF TEST
PORTLAND CEMENT

Date:
File No.

NBS Test No. MS-0243 Specification SS-C-00192f, (COM-NBS), II, low alkali
Company *Composite Location _____ Bin No. _____
Dates sampled October 29, 1964, sample submitted by Corps Barrels represented _____
of Engineers.

This cement does _____ meet the specification requirements.

SAMPLE NOS.	IGN. LOSS %	INSOL. RES. %	SO ₃ %	M ₂ O %	SiO ₂ %	Al ₂ O ₃ %	Fe ₂ O ₃ %	CaS + CaA %	CaA %	CaS %	C ₃ A %	TOTAL ALKALI AS Na ₂ O %	COMP. STRENGTH LBS./SQ. IN.		AIR ENTR. %	BLAINE SPECIFIC SURFACE cm ² /g
													3-DAY	7-DAY		
NPD 12895	1.2	0.1	1.9	1.7	22.5	4.4	3.5	55	5.7	27	11	0.44	2280	3270	8.4	3590
													<u>*28-Day</u> 4870			

	AUTO. EXPAN. %	TIME OF SET			AUTO. EXPAN. %	TIME OF SET		REMARKS: Heat of hydration 67 cal./gr. NPD 12895 composite sample consisting of: NPD 12765 - Ideal, Gold Hill NPD 12832 - Oregon, Lake Oswego NPD 12894 - Permanente, Bellingham Alkalies CaO Free CaO Na ₂ O K ₂ O 0.23 0.32 64.0 0.87 Refer: Your letter 20 October 1964, NPDEN- GS-L. Charge costs to 96x4902 Revolving Fund, Acct. 441.3.1.3, W. O. 62-CPCh-68. Note: 28 day supplementary report will follow.
		Initial (hr. min.)	Final (hr. min.)			Initial (hr. min.)	Final (hr. min.)	
1-2	-0.01	3:50	6:20	21-22				
3-4				23-24				
5-6				25-26				
7-8				27-28				
9-10				29-30				
11-12				31-32				
13-14				33-34				
15-16				35-36				
17-18				37-38				
19-20				39-40				

The contents of this report are for your information and shall not be used in advertising or sales promotion to indicate either explicitly or implicitly endorsement of this product by the National Bureau of Standards.

cc: Institute for Applied Technology, NBS, Washington, D. C.
*Reported by NBS En 539 B, 18 Nov 64.

For the Director

Frank W. Winblade
Chief, Seattle Laboratory
Inorganic Building Materials Sec.
Building Research Division

INCL 2

STATE: Idaho	INDEX NO: 3 (Supple #1)	AGGREGATE DATA SHEET	TESTED BY: North Pacific Division
LAT: 46	LONG: 116	DATE: 1961	
LAB SYMBOL NO: 61-CPCh-2	TYPE OF MATERIAL: Ledge Rock		
LOCATION: Proposed Granite-Gneiss Quarry, S ₂ , NW ₄ , Sec. 35, T-37-N, R-1-E, BM.			
About $\frac{3}{4}$ mile downstream from dam axis, left bank North Fork Clearwater River			
PRODUCER: Undeveloped			
SAMPLED BY: NPW Personnel			
TESTED FOR: Brucos Eddy Dam on North Fork Clearwater River, Idaho			
PROCESSING BEFORE TESTING: Laboratory processed in jaw and gyratory crushers and rod-mill and rake classifier. (Bulk sample obtained by blasting face of ledge rock).			
GEOLOGICAL FORMATION AND AGE: laramide Orogenic Epoch (Cretaceous)			
GRADING (CRD-C 103) (CUM. % PASSING):		TEST RESULTS	
SIEVE	3-6" 1 1/2-3" 3-1 1/2" #4-2" FINE AGG	3-6" 1 1/2-3" 3-1 1/2" #4-2" FINE AGG	
6 IN.	93		
5 IN.	61		
4 IN.	24		
3 IN.	1		
2 1/2 IN.	81		
2 IN.	55		
1 1/2 IN.	9		
1 IN.			
3/4 IN.			
3/8 IN.			
NO. 4			
NO. 8			
NO. 16			
NO. 30			
NO. 50			
NO. 100			
NO. 200			
- 200 ^(a)			
F.V. ^(b)			
(a) CRD-C 105 (b) CRD-C 104		MORTAR:	
MORT. 1-BAR EXPANSION AT 100F, % (CRD-C 123):		FINE AGGREGATE	
LOW-ALK. CEMENT: 0.45 % Na ₂ O EQUIVALENT:		COARSE AGGREGATE	
HIGH-ALK. CEMENT: 1.10 % Na ₂ O EQUIVALENT:			
SOUNDNESS IN CONCRETE (CRD-C 40, 114):		F & T HW-CD HD-CW	
FINE AGG. Rod-mill		COARSE AGG. Crushed Gneiss	
FINE AGG.		COARSE AGG.	
PETROGRAPHIC DATA (CRD-C 127): Hornblende-biotite-quartz diorite gneiss (commonly called granite gneiss): 85% medium to coarse grained gneiss, 10% fine grained hard schist that does not split easily, and 5% medium to fine grained quartz feldspar rock. Gneiss: 45-55% plagioclase feldspar, 20-30% hornblende and biotite, 15-20% quartz, and 10-15% accessory minerals. Hard schist: 40-50% hornblende and biotite, 30-40% quartz, and 10-20% plagioclase feldspar and minor accessory minerals. Quartz-feldspar rock: 60% plagioclase and 40% quartz, traces of accessories. No alkali-reactive constituents. Angular particles of fresh rock, except for minor hydrothermal alteration. Sand same composition, except 30% discrete mineral grains. Sand 3% soft (NPD and Devil Shaker Test Method)			

STATE: Idaho	INDEX NO: 3(Supl 243, AGGREGATE	TESTED BY: North Pacific Division
LAT: 46	LONG: 116 Rev.)	DATE: 1961-1964
LAB SYMBOL NO: W.O. 61-CPCh-2 & 62-CPCh-68		TYPE OF MATERIAL: Gneiss Ledge Rock
LOCATION: Proposed granite gneiss quarry, left bank North Fork Clearwater River, from stream bank to elev. 2300', test quarry and tunnel QT-1 of quarry area		
PRODUCER: Undeveloped		
SAMPLED BY: NPW Personnel		
TESTED FOR: Dworshak (Bruce Eddy) Dam, North Fork Clearwater River, Idaho		
PROCESSING BEFORE TESTING: Laboratory processed in jaw and gyratory crushers and rod-mill and rake classifier. (Bulk sample obtained by blasting face of ledge rock and 100 ton bulk sample from tunnel excavation, in 1962 & 1963,		
GEOLOGICAL FORMATION AND AGE: Orofino Series, Laramide Orogenic Epoch (Cretaceous)		

GRADING (CRD-C 103)(CUM. % PASSING)						TEST RESULTS					
SIEVE	3-6"	1 1/2-3"	3/4-1 1/2"	3/8-3/4"	FINE AGG.		3-6"	1 1/2-3"	3/4-1 1/2"	3/8-3/4"	FINE AGG.
BULK SP. GR., SAT SURF DRY (CRD-C 107, 108)							2.74	2.76	2.74	2.72	2.72
ABSORPTION, PER CENT (CRD-C 107, 108)							0.3	0.3	0.3	0.8	0.6
ORGANIC IMPURITIES, FIG. NO (CRD-C 121)											1
SOFT PARTICLES, PER CENT (CRD-C 130)											
PER CENT LIGHTER THAN 2.00 (CRD-C 129)											
PER CENT FLAT AND ELONGATED (CRD-C 119, 120)							2.0	3.6	1.4	2.5	5.7
WEIGHTED AV. % LOSS, 5 CYC. Na ₂ SO ₄ (10 1/2", 1", 3/8", 3/4") (CRD-C 115)											2.5
ABRASION LOSS (L.A.), % (CRD-C 117) "A" Grad.											31.8
UNIT WT., LB/CU FT (CRD-C 106)							100	99	97	97	108
CLAY LUMPS, % (CRD-C 118)											
COAL AND LIGNITE, % (CRD-C 122)											
SPECIFIC HEAT, BTU/LB/DEG. F. (CRD-C 123)							2.42	0.183			0.189
REACTIVITY WITH H ₂ SO ₄ (CRD-C 128)											
MORTAR-MAKING PROPERTIES (CRD-C 119) (104) (flexural) (125)											
TYPE II CEMENT, RATIO 7 DAYS 118											111
LINEAR THERMAL EXPANSION X 10 ⁻⁶ /DEG. F. (CRD-C 123, 126)											
ROCK TYPE	PARALLEL	CROSS	ON	AVERAGE							
Quartz feldspar	2.3	3.7		3.0							
Gneiss	2.7	3.4		3.1							
Hard Schist	3.5	3.4	3.8	3.6							
Coarse gneiss	3.5	3.6	3.8	3.6							
Mod. Gneiss	4.2	3.7	3.3	3.8							

MORTAR-BAR EXPANSION AT 100F, % (CRD-C 123)		FINE AGGREGATE				COARSE AGGREGATE			
		3 MO.	6 MO.	9 MO.	12 MO.	3 MO.	6 MO.	9 MO.	12 MO.
LOW-ALK. CEMENT: 0.385% Na ₂ O EQUIVALENT:		0.013	0.017	0.015	0.022	*			
HIGH-ALK. CEMENT: 0.972% Na ₂ O EQUIVALENT:		0.012	0.012	0.007	0.014	*			

SOUNDNESS IN CONCRETE (CRD-C 40, 114):			
FINE AGG.	COARSE AGG.	DFE ₃₀₀	DFE ₃₀₀
FINE AGG. Rod-mill	COARSE AGG. Crushed gneiss	69	

PETROGRAPHIC DATA (CRD-C 127): Hornblende-biotite-quartz diorite gneiss (commonly called granite gneiss): 85% medium to coarse grained gneiss, 10% fine grained hard schist that does not split easily, and 5% medium to fine grained quartz feldspar rock. Minor amounts of garnet rock, marble, and basalt dike material in quarry areas. Typical gneiss: 45-55% plagioclase feldspar, 20-30% hornblende and biotite, 15-20% quartz, and 10-15% accessory minerals. No alkali-reactive constituents. Fresh rock, except for minor hydrothermal alteration and weathering near surface. Sand has 30% discrete mineral grains of which 1% are free mica flakes. Subject to mixer-grinding due to cleavable feldspar and mica.

REMARKS: * Data from W.O. 61-CP-2, river bank quarry site; rest of data from W.O. 62-CPCh-68, tunnel sample. Sand 3% soft by NPD Shaker test.

MIX DATA							SCREEN ANALYSES-% RETAINED							
MATERIAL	% C.A.	SAMPLE NO.	SOLID VOLUME cu ft	WEIGHTS S.S.D. lb	BULK SP.GR. S.S.D.	% ABSORP.	SIEVE SIZE	NOMINAL SIZES				COMBINED		
								(A) 6" to 3"	(B) 3" to 1 1/2"	(C) 1 1/2" to 3/4"	(D) 3/4" to 3/8"	Sand	Coarse Aggregate	Total Aggregate
Coarse Agg (A)	30	11778	5.185	886	2.74	0.3	7"	0					0	0
" (B)	30	"	5.185	893	2.76	0.3	6"	5					1.6	1.3
" (C)	20	"	3.449	590	2.74	0.3	5"	24					8.7	6.9
" (D)	20	"	3.449	585	2.72	0.8	4"	46					22.5	17.9
Sand		"	4.426	751	2.72	0.6	3"	20					28.5	22.7
Cement		12078	1.088	213.8	3.15		2 1/2"	5	33				39.9	31.8
Pozzolan		12064	0.466	74.1	2.55		2"		34				50.1	39.9
Water			2.742	171.1			1 1/2"		28				59.7	47.6
Air (5.0% on 1/2")			1.010				1"		5	50			71.2	56.6
Totals			27.000	4164			3/4"			35	4		79.0	62.9
							1/2"			7	38		88.0	70.0
							3/8"			2	28		94.0	74.8
							4"				26		99.2	79.0
							8"				4	19		83.0
							16"					25		88.3
							30"					16		91.4
							50"					16		95.0
							100"					14		98.0
							PAN					10	100.0	100.0
							F.M.					2.89		

MIX CHARACTERISTICS		
WATER-CEMENT RATIO	Gallons per bag, equivalent cement	6.32
	By weight, " "	0.56
	By weight, $\frac{\text{water}}{\text{cement} + \text{pozzolan}}$	0.594
Cement factor, bags/cu yd (Eq. Sol. Vol. as P.C.)		3.25
Pozzolan, % replacement by solid volume		30.0
Slump, inches (2-2 1/2)		2
Air content (Note 1), % (5.8-6.6)		6.2
Unit weight, lbs/cu ft (Theoretical)		154.0
Bleeding (Note 2), %		3.5
Sand / aggregate, % by volume		21.0
Temperature of plastic concrete, °F		67

STRENGTH TEST DATA (Compressive)			
Age, days	3	7	28
Strength, p.s.i. (average)	840	1050	2270
Age, days	90	180	365
Strength, p.s.i. (average)	3640	4020	4480

NOTES:
 1. In that portion of the concrete containing aggregate smaller than the 1/2" - inch sieve.
 2. Percentage of mix water separating from concrete in bleeding test.

REMARKS Compressive strengths are average of six cylinders: two from each of three batches.

PROJECT

BRUCES EDDY DAM

DISTRICT

Walla Walla (E-278)

FINE AGGREGATE Laboratory produced granite-gneiss sand; 50/50 blend of No. 4 - No. 16

COARSE AGGREGATE Laboratory crushed granite-gneiss

CEMENT Blended Type II cement, 25% each, Ideal, Lehigh, Oregon and Permanente

POZZOLAN Idealite (calcined shale) Great Western Aggregates Company, Laramie, Wyoming

A.E.A. Lab Stock NVR, (#11356)
426 cc/cu.yd.

OTHER ADMIXTURE

None

MIX NO. 12128

W/O NO.

CAST 1/28/63

62-CPCh-68

AUG 20 1965

O. E. BORGE

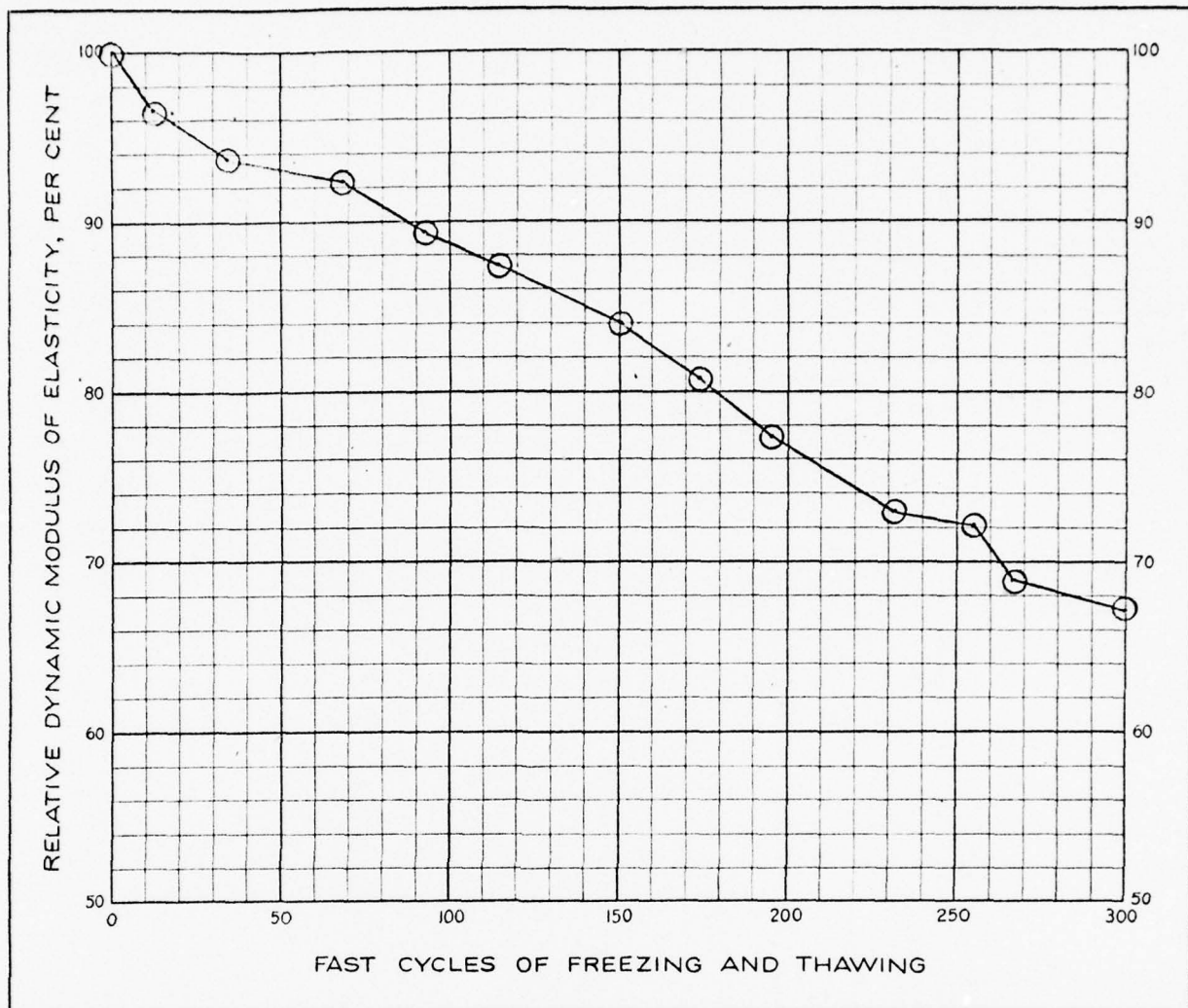
(Date of Report)

Act'g Chief, Concrete Branch

NPD RF 359
APRIL 61REPORT OF CONCRETE MIXTURE DESIGN
METHODS CRD-C 3 AND 10
CORPS OF ENGINEERS NORTH PACIFIC DIVISION TESTING LABORATORY

Mix No. C-2

MIX DATA							SCREEN ANALYSES--% RETAINED							
MATERIAL	% C.A.	SAMPLE NO.	SOLID VOLUME cu ft	WEIGHTS S. S. D. lb	BULK SP. GR. S. S. D.	% ABSORP.	SIEVE SIZE	NOMINAL SIZES				COMBINED		
								(A) 6" to 3"	(B) 3" to 1 1/2"	(C) 1 1/2" to 3/4"	(D) 3/4" to 4"	Sand	Coarse Aggregate	Total Aggregate
Coarse Agg(A)							7"							
" (B)							6"							
" (C)	50	11778	5.744	982	2.74	0.3	5"							
" (D)	50	"	5.744	975	2.72	0.8	4"							
Sand		"	7.283	1236	2.72	0.6	3"							
Cement		12078	1.674	329.0	3.15		2 1/2"							
Pozzolan		12064	0.717	114.1	2.55		2"					0	0	
Water			4.218	263.2			1 1/2"			6		3.0	1.8	
Air (6.0% on-1/2)			1.620				1"			50		23.0	14.1	
Totals			27.000	3899			3/4"			35	4	47.5	29.1	
							1/2"			7	38	70.0	42.9	
							3/8"			2	28	85.0	52.0	
							4"				26	98.0	60.0	
							8"				4	19	67.5	
							16"					25	77.4	
							30"					16	83.6	
							50"					16	89.9	
							100"					14	95.3	
							PAN					10	100.0	100.0
							F. M.					2.89		
MIX CHARACTERISTICS														
WATER-CEMENT RATIO	Gallons per bag, equivalent cement					6.32								
	By weight, " "					0.56								
RATIO	By weight, water					0.594								
	By weight, cement + pozzolan													
Cement factor, bags/cu yd (Eq. Sol. Vol. as P.C.)						5.0								
Pozzolan, % replacement by solid volume						30.0								
Slump, inches						2								
Air content (Note 1), %						5.8								
Unit weight, lbs/cu ft						144.7								
Bleeding (Note 2), %						-								
Sand / aggregate, % by volume						40.0								
Temperature of plastic concrete, °F						66								
STRENGTH TEST DATA (Compressive)														
Age, days		3	7	28										
Strength, p s i (average)		840	1120	2320										
Age, days		90	180	365										
Strength, p s i (average)		3700												
NOTES:														
1. In that portion of the concrete containing aggregate smaller than the 1/2" - inch sieve.														
2. Percentage of mix water separating from concrete in bleeding test.														
REMARKS														
1 1/2" Max. version of Mix A-2														
							PROJECT							
							BRUCES EDDY DAM							
							DISTRICT Walla Walla (E-278)							
							FINE AGGREGATE Laboratory produced granite-gneiss sand; 50/50 blend of No. 4 - No. 16 and minus No. 16							
							COARSE AGGREGATE Laboratory crushed granite-gneiss							
							CEMENT							
							Blended Type II cement, 25% each, Ideal, Lehigh, Oregon and Permanente							
							POZZOLAN							
							Idealite (Calcined Shale) Great Western Aggregates Company, Laramie, Wyoming							
							A.E.A. Lab. stock NVR (#11356) 320 cc/cu.yd.							
							OTHER ADMIXTURE							
							None							
							MIX NO. 12258				W/O NO.			
							CAST 4/28/63				62-CPCh-68			
							AUG 20 1965 (Date of Report)				O. E. BORGE Actg. Chief, Concrete Branch			
NPD RF 359 APRIL 61							REPORT OF CONCRETE MIXTURE DESIGN METHODS CRD-C 3 AND 10 CORPS OF ENGINEERS NORTH PACIFIC DIVISION TESTING LABORATORY							



BEAM NO.	SYMBOL	DFE AT 300 CYCLES	NO. CYCLES REL E = 50%	PROJECT
13002		70.2		SNETTISHAM
13002-A		66.1		DISTRICT Alaska
13002-B		65.7		FINE AGGREGATE
				Rod-mill sand processed from blasted Granite Ledge rock sample.
Grand Avg.	○	67.3		COARSE AGGREGATE
				Crushed gravel processed from blasted Granite Ledge rock sample.
REMARKS				Plotted JP
				Checked OEB
				30 Apr 65 (Date of Report)
				W/O NO. 64-CPCh-188
				D. L. HOUGHTON Chief, Concrete Branch

NPD FORM 361
JUNE 49

RESISTANCE OF CONCRETE BEAMS TO ACCELERATED
FREEZING AND THAWING
CORPS OF ENGINEERS NORTH PACIFIC DIVISION TESTING LABORATORY

100-3

ADDRESS REPLY TO
DIRECTOR
NOT TO INDIVIDUALS

U. S. ARMY ENGINEER DIVISION, NORTH PACIFIC
CORPS OF ENGINEERS

NORTH PACIFIC DIVISION MATERIALS LABORATORY
RT. 2, BOX 12A
TROUTDALE, OREGON

NPDEN-GS-L (64-CPCh-188)

20 August 1965

SUBJECT: Snettisham Project, Report of Laboratory Investigations of
Granite Ledge Rock Sample

TO: District Engineer
U. S. Army Engineer District, Alaska
ATTN: NPAEN-FM

1. Reference is made to:

a. Your letter of 6 January 1965, subject: "Letter of Transmittal to Accompany Concrete Aggregate Materials" applicable to a 3600 pound sample of rock received at this laboratory 22 December 1964.

b. Reports forwarded under cover of our letters of 29 January and 7 May 1965.

2. Attached is report which includes:

a. NPD Form 359, Report of Concrete Mixture Design, with concrete strengths through 90-days age for 1½-inch maximum size aggregate mix NPD Lab. No. 13114.

b. NPD Form 359 for Dworshak Dam 6-inch mass mix No. 12128.

c. NPD Form 359 for Dworshak Dam mix No. 12258, a 1½-inch max version of 6-inch mass mix No. 12128.

d. NBS Report of Test Portland Cement for lab stock blended cement, NPD Lab No. 12895, used in the Snettisham mix.

e. ENG Form 6000R, Report of Tests on Pozzolan for pozzolan NPD Lab No. 12064, Idealite (calcined shale) which was used in the Snettisham and Dworshak mixes.

f. First quarter report of the alkali-aggregate reactivity mortar bar expansion tests.

3. Since laboratory stock of Snettisham aggregate was limited, particularly the plus 1½-inch sizes, a 6-inch mass mix could not be batched for this study. However, a 1½-inch maximum aggregate mix was proportioned on the basis of a 6-inch mix with the plus 1½-inch aggregate excluded. Data

20 August 1965

SUBJECT: Snettisham Project, Report of Laboratory Investigations of
Granite Ledge Rock Sample

for this mix No. 13114 is given on an attached Form 359. To give you additional information on this batching technique we have attached mix design data for two Dworshak Dam mixes, Nos. 12258 and 12128 in which this method was used. Mix No. 12258, a 1½-inch maximum mix was designed to be the equivalent of mix No. 12128, a 6-inch mix, with the plus 1½-inch aggregate excluded. The study was made to see the correlation in strength of the two comparable mixes having different maximum size aggregates. This Dworshak mix data has significance with regard to the Snettisham investigation in that the aggregates of the two projects are similar in physical characteristics. It is interesting to note also that to date strength developing characteristics for these mixes having similar water cement ratios are relatively equivalent.


The Snettisham mix was batched by two methods, the conventional NPD method using air-dried aggregate and a newer pre-soaked aggregate method. In the conventional method, air-dried aggregates having a known moisture content are weighed and batched into the mixer. Net aggregate absorption during mixing and plastic test period is allowed for on the basis of 70 percent of the 24 hour net absorption value. For the pre-soaked mixes, air-dried aggregates having a known moisture content are weighed and soaked in water overnight. Aggregate batches are then drained and accurately weighed prior to charging the mixer. Increased aggregate batch weight represents absorbed and free moisture, and the difference between that and the total prescribed batch water represents water to be added to the mix.

4. Results of the mortar bar expansion tests are inconclusive at this time. Six months results should indicate a definite trend.

5. The second quarter report of the alkali-aggregate reactivity mortar bar expansion tests will be forwarded about 20 October 1965.

4 Incl (in dupe)

1. NPD Fm 359's
2. NBS Rpt Portland Cement
3. ENG Rpt Pozzolan
4. First quarter rpt, MBE


J. L. BARGAR
Director

NPDEN-GS-L (64-CPCh-188)

SNETTISHAM PROJECT

REPORT OF ALKALI-AGGREGATE REACTIVITY MO

<u>Set No.</u>	<u>Cement *</u>	<u>Aggregate</u>	<u>8-Day</u>	<u>1-Mo.</u>	<u>2-Mo.</u>	<u>3-Mo.</u>	<u>4-Mo.</u>	<u>Exps</u> <u>5-Mo</u>
12955-1	High Alk.	Lab-processed	0.005	0.009	0.012	0.012		
	Low Alk.		0.001	-0.002	0	0.005		
12955-2	High Alk.		0.003	0.005	0.008	0.011		
	Low Alk.		-0.001	0	0.002	0.008		
12955-3	High Alk.		0.001	0.005	0.012	0.012		
	Low Alk.		-0.001	0.004	0.007	0.011		

* Note: Alkali content of cement expressed as percent Na_2O : High Alkali 0.972
Low Alkali 0.385

JECT

Aug 20, 1965

MORTAR BAR EXPANSION TESTS

Expansion, Percent

<u>5-Mo.</u>	<u>6-Mo.</u>	<u>7-Mo.</u>	<u>8-Mo.</u>	<u>9-Mo.</u>	<u>10-Mo.</u>	<u>11-Mo.</u>	<u>12-Mo.</u>
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PART 5 CONCRETE AGGREGATE - COST STUDY A, B & C

SNETTISHAM AGGREGATE SOURCES

END FORM 1730 (REV 11-10-2-1902) Translucent
1 JAN 49

GOVERNMENT ESTIMATE DETAIL SUMMARY SHEET

[illegible]

SHEET OF

ADJUSTED

UNIT
COST

AMOUNT

8 9.25

832,500

2 11.40

1,026,000

2 9.00

810,000

COMP. _____
CHKD. _____

CORPS OF ENGINEERS
ALASKA DISTRICT
ANCHORAGE, ALASKA

SHEET NO. _____
FILE _____

SUBJECT SNETTISHAM AGGREGATE SOURCE "A"

MOBILIZATION COSTS:

	MACHINE	WEIGHT (LBS)
1	SHOVEL	55,000
1	PUMP	900
1	DOZER	42,800
1	FRONT END LOADER	23,100
1	TRUCK 2 1/2 TON	7,000
3	CONVEYOR	69,000
1	CONVEYOR	80,000
1	CRUSHER	72,000
1	RESCREENING PLANT	57,000
	GATES, PIPES, BULKHEADS	20,000
1	BARGE	145,000

TOTAL WT. 572,200 LBS

TRANSPORT. COST @ 3.25/100 LBS

$5722 \times 3.25 = 18,597 \$$

LOAD EQUIPM. AT YARD AND HAUL TO BARGE

10 MEN - 2 DAYS @ 4.0 \$/HR

$160 \times 4.00 = 640 \$$
TRANSPORTATION COST 1,500

UNLOAD AND RE-ASSEMBLE EQUIPM.

12 MEN - 30 DAYS @ 5.97 \$/HR.

$96 \times 30 \times 5.97 = 17,194 \$$

COMP & LIAB. INC. @ 13.72% = $17,834 \times .1372 = 2,447 \$$

TOTAL MOBIL. COST = 40,378 \$

DEMOR. COST 75 % = 30,284 \$

TOTAL MOBIL & DEMOBIL. COST = 70,662 \$

SHEET 1 OF

PROJECT

SNETTISHAM

SUBJECT

AGGREGATE SOURCE "1"

QUANTITY	UNIT	PRICE	TOTAL
1	100	100	100
2	100	100	200
3	100	100	300
4	100	100	400
5	100	100	500
6	100	100	600
7	100	100	700
8	100	100	800
9	100	100	900
10	100	100	1000
11	100	100	1100
12	100	100	1200
13	100	100	1300
14	100	100	1400
15	100	100	1500
16	100	100	1600
17	100	100	1700
18	100	100	1800
19	100	100	1900
20	100	100	2000
21	100	100	2100
22	100	100	2200
23	100	100	2300
24	100	100	2400
25	100	100	2500
26	100	100	2600
27	100	100	2700
28	100	100	2800
29	100	100	2900
30	100	100	3000
31	100	100	3100
32	100	100	3200
33	100	100	3300
34	100	100	3400
35	100	100	3500
36	100	100	3600
37	100	100	3700
38	100	100	3800
39	100	100	3900
40	100	100	4000
41	100	100	4100
42	100	100	4200
43	100	100	4300
44	100	100	4400
45	100	100	4500
46	100	100	4600
47	100	100	4700
48	100	100	4800
49	100	100	4900
50	100	100	5000
51	100	100	5100
52	100	100	5200
53	100	100	5300
54	100	100	5400
55	100	100	5500
56	100	100	5600
57	100	100	5700
58	100	100	5800
59	100	100	5900
60	100	100	6000
61	100	100	6100
62	100	100	6200
63	100	100	6300
64	100	100	6400
65	100	100	6500
66	100	100	6600
67	100	100	6700
68	100	100	6800
69	100	100	6900
70	100	100	7000
71	100	100	7100
72	100	100	7200
73	100	100	7300
74	100	100	7400
75	100	100	7500
76	100	100	7600
77	100	100	7700
78	100	100	7800
79	100	100	7900
80	100	100	8000
81	100	100	8100
82	100	100	8200
83	100	100	8300
84	100	100	8400
85	100	100	8500
86	100	100	8600
87	100	100	8700
88	100	100	8800
89	100	100	8900
90	100	100	9000
91	100	100	9100
92	100	100	9200
93	100	100	9300
94	100	100	9400
95	100		

90,000 cY

AVAILABLE OPERATION TIME IN

CALENDAR DAYS

WORKING DAYS

PLAN OF OPERATIONS: IN THIS STUDY ALL AGGREGATES ARE PRODUCED
IN ONE CONSTRUCTION SEASON AND STOCKPILED NEAR GRAVEL
PIT AREA.

ASSUME 30% REJECT AND INCUR AN ADDITIONAL 25%

$$90,000 \times 1.25 = 112,500 \text{ "X"}$$

SAY 6 acres ARE TO BE MOWED AND GRUBBED

@ 500 #/acre	100%	5,200 #
--------------	------	---------

USE 1 C.Y. SHOVEL CAP/room = 180 C.Y.

USE PORTABLE CRUSHING PLANT CAP 200 T. P. H.

USE AT 70% EFF $500 \cdot .7 = 100 \text{ C.Y./HR}$

BASE TIME REQU. ON PRODUCTION RATE OF 100 CY / HR.

$$\text{TIME REQD} = \frac{160.500}{120} = 1.605 \text{ HRS.}$$

AT 16 HRS/DAY TIME REQU = 100 DAYS

DETERMINATION OF CONTROLLING PLANT OR LABOR OUTPUT, AMOUNT OF CONTROLLING PLANT OR LABOR, OPERATIONAL SHIFTS AND SECONDARY PLANT REQUIRED TO ACCOMPLISH WORK IN AVAILABLE WORKING DAYS:

USE 1-200 TON BARGE TO HAUL AGGREGATES TO BATCH PLANT

TIME REQ. FOR ONE TRIP IS 7 HRS

LOADING CAPACITY OF BARGE $\frac{200}{1.5} = 133 \text{ C.Y.}$

FROM COMPUTATION FOR STUDY "B" 124,260 C.Y. ARE TO BE MINED

TRIPS REQUIRED $128,260 \div 133 = 964$

AT 3 TRIPS / DAY $964/3 = 321$ DAYS

TOTAL HRS REQD. $321 \times 12 = 3,860$ HRS

SUMMARY OF OPERATIONAL TIME

SHIFTS PER DAY

HOURS PER SHIFT

PLANT HOURS	
1	1
2	2
3	3
4	4
5	5
6	6
7	7
8	8
9	9
10	10
11	11
12	12
13	13
14	14
15	15
16	16
17	17
18	18
19	19
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72	72
73	73
74	74
75	75
76	76
77	77
78	78
79	79
80	80
81	81
82	82
83	83
84	84
85	85
86	86
87	87
88	88
89	89
90	90
91	91
92	92
93	93
94	94
95	95
96	96
97	97
98	98
99	99
100	100

WORKING HOURS

REMARKS

GOVERNMENT ESTIMATE WORK SHEET						SHEET 2 OF 3	
SUBJECT					QUANTITY		
SNETTISHAM SOURCE "A"							
PLANT							
UNIT OF PLANT		SIZE	NO.	HOURS*	RATE	AMOUNT	
RENTAL OR OWNERSHIP EXPENSE	DOZER		100-150HP	1	1,600	14.10	22,560
	SHOVEL		1 CY	1	1,600	21.70	44,320
	CRUSHING UNIT		200 TPH	1	"	25.00	40,000
	BELT CONVEYOR		18" x 70'	1	"	7.80	12,480
	CONVEYOR		"	1	"	7.80	12,480
	LOADING CONVEYOR		"	1	"	7.80	12,480
	FUEL TRUCK		2 1/2 TON	1	800	5.10	4,080
	CONVEYOR (UNLOADING)		18" x 2000'	1	"	"	75,000
	FRONT END LOADER		2 1/4 CY	1	1,400	12.47	17,458
	PUMP		4"	1	1,600	.91	1,456
	RESCREENING PLANT		5' x 12'	1	1,600	6.10	9,760
	GATES, PIPES, BULKHEADS						5,000
OPERATING SUPPLIES AND FIELD REPAIRS	BARGE				18 M	1,000	18,000
	BOAT AND MOTOR			1	1.00		3,000
	TRUCK 3/4 TON				3,860	2.94	11,348
	" "				6 M	202	1,212
	FRONT END LOADER		2 1/4 CY	1	3,860	12.47	48,134
	" "				6 M	1310	7,860
						SUBTOTAL	346,568
* NOTE: Use calendar hours for plant rental and working hours for plant operating supplies and field repairs.						SMALL TOOLS % OF LABOR	
						TOTAL PLANT COST	
LABOR							
OPERATION	CRAFT	NO.	HOURS**	RATE	AMOUNT		
	DOZER OPERATOR	1	1,600	5.86	9,376		
	SHOVEL "	1	"	6.00	9,600		
	CRUSHER "	1	"	5.97	9,552		
	CRUSHER OILER	1	"	5.34	8,544		
	MECHANIC	1	"	5.97	9,552		
	FOREMAN	1	"	7.00	11,200		
	TRUCK DRIVER	1	800	5.47	4,376		
	LABORER	2	3,200	5.30	16,960		
						SUBTOTAL	79,160
						OVERTIME 8.53 %	6,594
						SUBTOTAL	85,754
						COMP. & LIAB. INS., SOC. SEC. & STATE EMP. INS. 13.72 %	11,765
(** NOTE: USE WORKING HOURS)						TOTAL LABOR COST	97,519

SHEET 3 OF 3

SUBJECT SNETTISHAM AGGREGATE, ZURLE "A"

QUANTITY	UNIT	PRICE	TOTAL
1	100	100	100
2	100	100	200
3	100	100	300
4	100	100	400
5	100	100	500
6	100	100	600
7	100	100	700
8	100	100	800
9	100	100	900
10	100	100	1000
11	100	100	1100
12	100	100	1200
13	100	100	1300
14	100	100	1400
15	100	100	1500
16	100	100	1600
17	100	100	1700
18	100	100	1800
19	100	100	1900
20	100	100	2000
21	100	100	2100
22	100	100	2200
23	100	100	2300
24	100	100	2400
25	100	100	2500
26	100	100	2600
27	100	100	2700
28	100	100	2800
29	100	100	2900
30	100	100	3000
31	100	100	3100
32	100	100	3200
33	100	100	3300
34	100	100	3400
35	100	100	3500
36	100	100	3600
37	100	100	3700
38	100	100	3800
39	100	100	3900
40	100	100	4000
41	100	100	4100
42	100	100	4200
43	100	100	4300
44	100	100	4400
45	100	100	4500
46	100	100	4600
47	100	100	4700
48	100	100	4800
49	100	100	4900
50	100	100	5000
51	100	100	5100
52	100	100	5200
53	100	100	5300
54	100	100	5400
55	100	100	5500
56	100	100	5600
57	100	100	5700
58	100	100	5800
59	100	100	5900
60	100	100	6000
61	100	100	6100
62	100	100	6200
63	100	100	6300
64	100	100	6400
65	100	100	6500
66	100	100	6600
67	100	100	6700
68	100	100	6800
69	100	100	6900
70	100	100	7000
71	100	100	7100
72	100	100	7200
73	100	100	7300
74	100	100	7400
75	100	100	7500
76	100	100	7600
77	100	100	7700
78	100	100	7800
79	100	100	7900
80	100	100	8000
81	100	100	8100
82	100	100	8200
83	100	100	8300
84	100	100	8400
85	100	100	8500
86	100	100	8600
87	100	100	8700
88	100	100	8800
89	100	100	8900
90	100	100	9000
91	100	100	9100
92	100	100	9200
93	100	100	9300
94	100	100	9400
95	100		

MATERIALS

[illegible]

SUPPLIES

[illegible]

SUMMARY FOR TRANSFER TO ENG FORM NO. 1739 OR ENG FORM NO. 1740

PLANT	346,568
LABOR	218,604
MATERIALS	
SUPPLIES	
TOTAL	565,172
REMARKS: *Prices on these items are based on quotations from manufacturers or suppliers.	PREPARED BY G. GROESCHEL CHECKED BY BB

COMP. _____
CHKD. _____

CORPS OF ENGINEERS
ALASKA DISTRICT
ANCHORAGE, ALASKA

SHEET NO. _____
FILE _____

SUBJECT SNETTISHAM AGGREGATE SOURCE "B"

COMPUTATIONS FOR MOBILIZATION

MACHINE	WEIGHT (LBS)
1 DRAGLINE	81900
1 PUMP	900
1 DOZER	42800
1 TRUCK 2 1/2 TON	7000
3 TRUCK 20 C.Y.	102600
1 CRUSHER	72000
1 RESCREENING PLANT	57400
3 CONVEYOR	69000
GATES, PIPES, BULKHEADS	20000
GRADER	26800
	<u>480400</u>

FREIGHT @ 3.25/100 LB

$$4,804 \times 3.25 = \underline{15,613 \$}$$

LOAD EQUIPMENT AT YARD AND HAUL TO BARGE

10 MEN FOR 2 DAYS @ 4 \$/HR

$$160 \times 4 = 640 \$$$

TRANSPORTATION COST - 1500 \$

UNLOAD EQUIPMENT AND RE-ASSEMBLE

10 MEN - 20 DAYS

$$@ \$4/\text{HR} = 1600 \text{ HRS}$$

$$@ \$5.97/\text{HR} = 1600 \times 5.97 = \underline{9,552 \$}$$

$$\text{COMP. \& LIAB. INS. } 10192 \times .1372 = \underline{1,398}$$

$$\text{TOTAL MOBILIZATION COST } \underline{28,703}$$

$$\text{DEMobilIZATION } 75\% \underline{21,527}$$

$$\text{TOTAL MOBIL. \& DEMOBIL. COST } \underline{\underline{50,230 \$}}$$

GOVERNMENT ESTIMATE WORK SHEET

SHEET 1 OF

PROJECT

SNETTISHAM

SUBJECT

AGGREGATE SOURCE "B"

QUANTITY

90,000 CY

AVAILABLE OPERATION TIME IN

CALENDAR DAYS

WORKING DAYS

PLAN OF OPERATIONS: IN THIS STUDY ALL AGGREGATES ARE PRODUCED
IN ONE CONSTRUCTION SEASON AND STOCKPILED NEAR GRAVEL
PIT AREA. 25% EXTRA AGGR. ARE STOCKPILED
FROM COURSE AGGREGATE SOURCE ASSUME 50% WASTE (SAND)

COURSE AGGR REQUIRED $\frac{90,000 \times 1.25}{.75} = 132,930 \text{ TONS}$

FOR FINE AGGREGATE SOURCE ASSUME 50% WASTE
ASSUME SOURCE PRODUCES 50% SAND AND 50% FINE AGGR.

SAND REQUIRED $\frac{90,000 \times .75}{.25} = 53,795 \text{ TONS}$

REDUCE COURSE AGGR $132,930 - 53,795 = 99,135 \text{ TONS}$

COURSE AGGR. $\frac{99,135 \times 1.25}{(1.5)(1.5)} = 165,220 \text{ C.Y.}$

FINE AGGR. $\frac{67,590 \times 1.25}{(1.7)(1.5)} = 20,460 \text{ C.Y.}$

TOTAL AMOUNT TO BE PRODUCED - 245,680 C.Y.

DETERMINATION OF CONTROLLING PLANT OR LABOR OUTPUT, AMOUNT OF CONTROLLING PLANT OR LABOR, OPERATIONAL SHIFTS
AND SECONDARY PLANT REQUIRED TO ACCOMPLISH WORK IN AVAILABLE WORKING DAYS:

CLEARING AND GRUBBING @ 100 CY/HR X 10 DAYS = 10,000 CY

ASSUME GRAVEL CAN BE EXCAVATED TO A DEPTH OF 21'
ASSUME DEPTH OF OVERBORDEN TO BE REMOVED IS 5'

USE PORTABLE CRUSHING PLANT (CAP. 200 TPH)

AT 70% EFFIC. $\frac{200 \times .7}{1.5} = 100 \text{ C.Y./HR}$

TIME REQD. $\frac{245,680}{100} = 2,457 \text{ HRS}$

AT 16 HRS/DAY $\frac{2,457}{16} = 153 \text{ DAYS}$

SUMMARY OF OPERATIONAL TIME

SHIFTS PER DAY

HOURS PER SHIFT

PLANT HOURS

WORKING HOURS

REMARKS

GOVERNMENT ESTIMATE WORK SHEET

SHEET 1 OF

PROJECT S'NETTISHAM

SUBJECT AGGREGATE SOURCE "B"

QUANTITY

AVAILABLE OPERATION TIME IN

CALENDAR DAYS

WORKING DAYS

PLAN OF OPERATIONS: HAUL AGGREGATE IN 20 CY DUMP TRUCKS FROM STOCKPILES TO BATCH PLANT OVER PERIOD OF TWO CONSTRUCTION SEASONS.

HAUL 115 %

COARSE AGGR. $99,135 \times 1.15 = 76,000 \text{ C.Y.}$

FINE AGGR. $62,590 \times 1.15 = 52,260 \text{ C.Y.}$

TOTAL C.Y. TO BE HAULED $128,260 \text{ C.Y.}$

HAULING DISTANCE IS ABOUT 3 MILES
ASSUME 1 TRIP/HR

TOTAL TRIPS $128,260 / 20 = 6413$

AT 10 TRIPS/DAY AND 3 TRUCKS $6413 / 3 = 214 \text{ DAYS}$

DETERMINATION OF CONTROLLING PLANT OR LABOR OUTPUT, AMOUNT OF CONTROLLING PLANT OR LABOR, OPERATIONAL SHIFTS AND SECONDARY PLANT REQUIRED TO ACCOMPLISH WORK IN AVAILABLE WORKING DAYS:

SUMMARY OF OPERATIONAL TIME

SHIFTS PER DAY

HOURS PER SHIFT

PLANT HOURS

WORKING HOURS

REMARKS

SHEET 2 OF 2

SNETTISHAM 16-7-50 "E"

* NOTE: Use calendar hours for plant rental and working hours for plant operating supplies and field repairs.

(* NOTE: USE WORKING HOURS)

GOVERNMENT ESTIMATE WORK SHEET

SHEET 1 OF

PROJECT

SNETTISHAM

SUBJECT

AGGREGATE SOURCES

"C"

QUANTITY

85,215 cu. yd.

AVAILABLE OPERATION TIME IN

CALENDAR DAYS

WORKING DAYS

PLAN OF OPERATIONS:

SOURCE C IS AGGREGATE SOURCE OF NATURAL LEAST
ROCK IMMEDIATELY ADJACENT TO 101 SITE.CAPACITIES DERIVED FROM TEST RUNS GIVE AGE 29.5T
CORE 7.51"

3705 lb.

FROM 70,000 cu. yd. core

 $70,000 \times 2705 = 189,350,000$

ASSUMING 15% PRODUCTION LOSS

 $189,350,000 \times 1.15 = 217,752,500 = 191,734 T$

4500 lb. CRUSHES = 1 cu. yd. IN PLACE AGGREGATE

 $191,734 \div 4500 = 85,215$ cu. yd.DRILL HOURS REQ'D 27V $27(85,215) = 1192$ hrs. 8'x8' PATTERN

64R 64(30)

30' FEET PER HOUR

USE 1200 HRS $\div 16$ HOURS PER DAY = 75 PRODUCTION DAYSUSE 50% PLANTING TIME = $5 \times 1200 = 600$ HRS.

TOTAL CORE TO BE DRILLED 36,000 FT

DETERMINATION OF CONTROLLING PLANT OR LABOR OUTPUT, AMOUNT OF CONTROLLING PLANT OR LABOR, OPERATIONAL SHIFTS
AND SECONDARY PLANT REQUIRED TO ACCOMPLISH WORK IN AVAILABLE WORKING DAYS:USING SWELL FACTOR OF 40% PLANT MUST BE
DESIGNED TO HANDLE $85,215 \times 1.40 = 119,301$ cu. yd.BASE PRODUCTION CAPACITY OF PLANT ON 200 WORKING
DAYS $119,301 \div 200 = 597$ CY/DAYUSE 100 CY/HR CAPACITY CRUSHING PLANT
@ 2 HRS DAY TOTAL OPERATING TIME IS 1600 HRS.MOBILIZATION AND DEMOBILIZATION USE SAME
COST AS FOR STUDY "C" 50,230 \$

SUMMARY OF OPERATIONAL TIME

SHIFTS PER DAY

HOURS PER SHIFT

PLANT HOURS

WORKING HOURS

REMARKS

GOVERNMENT ESTIMATE WORK SHEET						SHEET 2 OF 3	
SUBJECT <i>Accident</i>					QUANTITY <i>75,215 cu yd</i>		
PLANT							
RENTAL OR OWNERSHIP EXPENSE	UNIT OF PLANT	SIZE	NO.	HOURS*	RATE	AMOUNT	
	<i>COMPRESSOR</i>	<i>700 SEM</i>	<i>1</i>	<i>1200</i>	<i>9.55</i>	<i>11,460</i>	
	<i>AIR TANK</i>	<i>2 1/2"</i>	<i>1</i>	<i>1200</i>	<i>7.22</i>	<i>8,664</i>	
	<i>POWER TRUCK</i>	<i>2 TON</i>	<i>1</i>	<i>600</i>	<i>4.07</i>	<i>2442</i>	
OPERATING SUPPLIES AND FIELD REPAIRS	<i>COMPRESSOR</i>			<i>1200</i>	<i>8.22</i>	<i>9864</i>	
	<i>AIR TANK</i>			<i>1200</i>	<i>2.10</i>	<i>2520</i>	
	<i>POWER TRUCK</i>			<i>600</i>	<i>1.82</i>	<i>1092</i>	
* NOTE: Use calendar hours for plant rental and working hours for plant operating supplies and field repairs.					SUBTOTAL		<i>34,548</i>
					SMALL TOOLS % OF LABOR		
					TOTAL PLANT COST		
LABOR							
OPERATION	CRAFT	NO.	HOURS**	RATE	AMOUNT		
	<i>COMPRESSOR OPERATOR</i>	<i>1</i>	<i>1200</i>	<i>5.61</i>	<i>6732</i>		
	<i>DRIVER</i>	<i>1</i>	<i>1200</i>	<i>5.62</i>	<i>6744</i>		
	<i>CHUCK TENDER</i>	<i>1</i>	<i>1200</i>	<i>5.36</i>	<i>6432</i>		
	<i>POWER MAN</i>	<i>1</i>	<i>600</i>	<i>5.76</i>	<i>3456</i>		
	<i>HEWLER</i>	<i>1</i>	<i>600</i>	<i>5.12</i>	<i>3072</i>		
TOTAL				SUBTOTAL		<i>26,456</i>	
				OVERTIME <i>8.32</i> %		<i>2,204</i>	
				SUBTOTAL		<i>28,660</i>	
				COMP. & LIAB. INS., SOC. SEC. & STATE EMP. INS. <i>13.72</i> %		<i>3,932</i>	
(** NOTE: USE WORKING HOURS)				TOTAL LABOR COST		<i>32,592</i>	

GOVERNMENT ESTIMATE WORK SHEET						SHEET 2 OF 3
SUBJECT					QUANTITY	
PLANT						
	UNIT OF PLANT	SIZE	NO.	HOURS*	RATE	AMOUNT
RENTAL OR OWNERSHIP EXPENSE	DOZER	100-150HP	1	1600	14.10	22,560
	SHOVEL	1 CY.	1	"	27.70	44,320
	CRUSHING UNIT	200T	1	"	25.00	40,000
	RESCREENING PLANT	5'x12'	1	"	6.10	9,760
	CONVEYOR	18" x 70'	1	"	7.20	12,480
	CONVEYOR					25,000
	FUEL TRUCK	2 1/2 TON	1	"	5.10	8,160
	TRUCK	3/4 TON	1	"	2.94	4,704
	PUMP	5"	1	"	2.00	3,200
	PRIMARY CRUSHER	24' x 36"	1	"	21.00	33,600
GATES, PIPES, BULKHEADS						5,000
OPERATING SUPPLIES AND FIELD REPAIRS	DOZER		1	6 M	13.70	8,220
	SHOVEL		1	"	2.440	14,640
	CRUSHING UNIT		1	"	3,800	22,800
	RESCREENING PLANT		1	"	1,000	6,000
	CONVEYOR		1	"	7.35	4,410
	FUEL TRUCK		1	"	3.72	2,232
	TRUCK	3/4 TON	1	"	2.02	1,212
	PUMP	5"	1	"	1.10	660
	PRIMARY CRUSHER	24' x 36"	1	"	3.000	1,800
	SUBTOTAL					
SMALL TOOLS						% OF LABOR
TOTAL PLANT COST						
LABOR						
	OPERATION	CRAFT	NO.	HOURS**	RATE	AMOUNT
	DOZER OPERATOR			1600	5.86	9,376
	SHOVEL OPERATOR			"	6.00	9,600
	" OILER			"	5.34	8,544
	CRUSHER OPERATOR		2	1600	5.97	19,104
	CRUSHER OILER		1	1600	5.34	8,544
	CONVEYOR OPERATOR		2	3200	5.34	17,088
	TRUCK DRIVER		1	1600	5.47	8,752
	LABORER		1	"	5.30	8,480
	FOREMAN		1	"	7.00	11,200
TOTAL						
SUBTOTAL						100,688
OVERTIME 8.33 %						8,357
SUBTOTAL						109,045
COMP. & LIAB. INS., SOC. SEC. & STATE EMP. INS. 15.72 %						17,045
TOTAL LABOR COST						126,090

PART 6

LOG RECORDS - TEST PITS

CORPS OF ENGINEERS

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Snettisham		SHEET: 1	
EXPLORATION LOG		LOCATION (Grid or Name): E 95,150		DATE: 28 Jun 68	
PERMANENT TP-1		NAME OF SHOT: Snettisham		NAME OF SHOT: Snettisham	
TEST #1	TYPE OF HOLE	DEPTH	DIAMETER	TIME OF DAY	TIME OF DAY
1	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
2	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
3	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
4	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
5	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
6	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
7	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
<p>FORMATION DESCRIPTION & REMARKS</p> <p>1. Sandy SILT</p> <p>2. Sandy GRAVEL</p> <p>3. Sandy GRAVEL</p> <p>4. Sandy GRAVEL</p> <p>5. Sandy GRAVEL</p> <p>6. Sandy GRAVEL</p> <p>7. Sandy GRAVEL</p> <p>NOTE: 1. Location approximate, no survey control.</p>					

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Snettisham		SHEET: 2	
EXPLORATION LOG		LOCATION (Grid or Name): E 95,150		DATE: 28 Jun 68	
PERMANENT TP-2		NAME OF SHOT: Snettisham		NAME OF SHOT: Snettisham	
TEST #1	TYPE OF HOLE	DEPTH	DIAMETER	TIME OF DAY	TIME OF DAY
1	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
2	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
3	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
4	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
5	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
6	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
7	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
<p>FORMATION DESCRIPTION & REMARKS</p> <p>1. Silty SAND</p> <p>2. Silty SAND</p> <p>3. Silty SAND</p> <p>4. Silty SAND</p> <p>5. Silty SAND</p> <p>6. Silty SAND</p> <p>7. Silty SAND</p> <p>NOTE: 1. Location approximate, no survey control.</p>					

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Snettisham		SHEET: 3	
EXPLORATION LOG		LOCATION (Grid or Name): E 95,150		DATE: 28 Jun 68	
PERMANENT TP-3		NAME OF SHOT: Snettisham		NAME OF SHOT: Snettisham	
TEST #1	TYPE OF HOLE	DEPTH	DIAMETER	TIME OF DAY	TIME OF DAY
1	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
2	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
3	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
4	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
5	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
6	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
7	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
<p>FORMATION DESCRIPTION & REMARKS</p> <p>1. Silty SAND</p> <p>2. Silty SAND</p> <p>3. Silty SAND</p> <p>4. Silty SAND</p> <p>5. Silty SAND</p> <p>6. Silty SAND</p> <p>7. Silty SAND</p> <p>NOTE: 1. Location approximate, no survey control.</p>					

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Snettisham		SHEET: 4	
EXPLORATION LOG		LOCATION (Grid or Name): E 95,150		DATE: 28 Jun 68	
PERMANENT TP-23		NAME OF SHOT: Snettisham		NAME OF SHOT: Snettisham	
TEST #1	TYPE OF HOLE	DEPTH	DIAMETER	TIME OF DAY	TIME OF DAY
1	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
2	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
3	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
4	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
5	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
6	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
7	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
<p>FORMATION DESCRIPTION & REMARKS</p> <p>1. Gravelly clayey SILT</p> <p>2. Sandy GRAVEL</p> <p>3. Sandy GRAVEL</p> <p>4. Sandy GRAVEL</p> <p>5. Sandy GRAVEL</p> <p>6. Sandy GRAVEL</p> <p>7. Sandy GRAVEL</p> <p>NOTE: 1. Location approximate, no survey control. 2. Water level varies with tide.</p>					

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Snettisham		SHEET: 5	
EXPLORATION LOG		LOCATION (Grid or Name): E 95,150		DATE: 28 Jun 68	
PERMANENT TP-24		NAME OF SHOT: Snettisham		NAME OF SHOT: Snettisham	
TEST #1	TYPE OF HOLE	DEPTH	DIAMETER	TIME OF DAY	TIME OF DAY
1	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
2	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
3	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
4	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
5	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
6	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
7	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
<p>FORMATION DESCRIPTION & REMARKS</p> <p>1. Sandy GRAVEL</p> <p>2. Sandy GRAVEL</p> <p>3. Sandy GRAVEL</p> <p>4. Sandy GRAVEL</p> <p>5. Sandy GRAVEL</p> <p>6. Sandy GRAVEL</p> <p>7. Sandy GRAVEL</p> <p>NOTE: 1. Location approximate, no survey control.</p>					

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Snettisham		SHEET: 6	
EXPLORATION LOG		LOCATION (Grid or Name): E 95,150		DATE: 28 Jun 68	
PERMANENT TP-25		NAME OF SHOT: Snettisham		NAME OF SHOT: Snettisham	
TEST #1	TYPE OF HOLE	DEPTH	DIAMETER	TIME OF DAY	TIME OF DAY
1	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
2	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
3	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
4	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
5	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
6	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
7	1.0' 1.0"	1.0'	1.0"	1.0'	1.0"
<p>FORMATION DESCRIPTION & REMARKS</p> <p>1. Gravelly SILT</p> <p>2. Sandy GRAVEL</p> <p>3. Sandy GRAVEL</p> <p>4. Sandy GRAVEL</p> <p>5. Sandy GRAVEL</p> <p>6. Sandy GRAVEL</p> <p>7. Sandy GRAVEL</p> <p>NOTE: 1. Location approximate, no survey control. 2. Water level varies with tide.</p>					

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DESIGNED:	<p>SNETTISHAM PROJECT, ALASKA</p> <p>FIRST STAGE DEVELOPMENT</p> <p>TEST PITS</p> <p>LOG RECORD NO. 1</p>	APPROVED:	<p>FILE NUMBER</p>
<p>Rev. 3 12-1-78</p> <p>DRAWN:</p> <p>Rev. 3 12-1-78</p> <p>CHECKED:</p> <p>D. Hester</p> <p>PREPARED:</p> <p>CWB</p>		<p>ENGINEER</p> <p>DATE</p> <p>DESCRIPTION</p> <p>BY</p>	
<p>SUPERVISED:</p> <p>CWB</p> <p>REVIEWED:</p> <p>RECOMMENDED:</p>		<p>SCALE</p> <p>SHEET</p>	
<p>OF</p>		<p>OF</p>	
<p>OF</p>		<p>OF</p>	

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Snettisham SHEET: 1 OF 1
LOCATION (Latitude & Longitude): N 64 30 E 93 30
NAME OF PROJECT: SNETTISHAM
NAME OF SITE: SNETTISHAM
NAME OF HOLE: TP 30
DATE: 15 OCT 1964

EXPLANATION: LOG

TP 30

TYPE OF HOLE: ☒ AUGER HOLE ☐ CHAIN HOLE ☐ TEST PIT
TYPE OF EQUIPMENT: ☒ POWER DRIVEN ☐ HAND DRIVEN
TYPE OF SAMPLES: ☒ TOP SOIL ☐ BOTTOM SOIL
DATE OF SAMPLES: 15 OCT 1964

TEST PIT

DEPTH OF HOLE: 13.0
TYPE OF SOIL: Silt
CLASSIFICATION: Silt
FORMATION DESCRIPTION & REMARKS: Plastic, high moisture content.

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NOTES: 1. Water table varies with tide.
2. Location approximate, no survey control.
3. Field classification, no laboratory tests at this time.

PROJECT: Snettisham PERMANENT TP 30
HOLE NO.

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Snettisham SHEET: 1 OF 1
LOCATION (Latitude & Longitude): N 64 30 E 93 30
NAME OF PROJECT: SNETTISHAM
NAME OF SITE: SNETTISHAM
NAME OF HOLE: TP 31
DATE: 15 OCT 1964

EXPLANATION: LOG

TP 31

TYPE OF HOLE: ☒ AUGER HOLE ☐ CHAIN HOLE ☐ TEST PIT
TYPE OF EQUIPMENT: ☒ POWER DRIVEN ☐ HAND DRIVEN
TYPE OF SAMPLES: ☒ TOP SOIL ☐ BOTTOM SOIL
DATE OF SAMPLES: 15 OCT 1964

TEST PIT

DEPTH OF HOLE: 13.0
TYPE OF SOIL: Silt
CLASSIFICATION: Silt
FORMATION DESCRIPTION & REMARKS: Organic, plastic, gray brown, high moisture content.

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NOTES: 1. Location approximate, no survey control.
2. Field classification, no laboratory tests at this time.

PROJECT: Snettisham PERMANENT TP 31
HOLE NO.

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Snettisham SHEET: 1 OF 1
LOCATION (Latitude & Longitude): N 64 30 E 93 30
NAME OF PROJECT: SNETTISHAM
NAME OF SITE: SNETTISHAM
NAME OF HOLE: TP 35
DATE: 15 OCT 1964

EXPLANATION: LOG

TP 35

TYPE OF HOLE: ☒ AUGER HOLE ☐ CHAIN HOLE ☐ TEST PIT
TYPE OF EQUIPMENT: ☒ POWER DRIVEN ☐ HAND DRIVEN
TYPE OF SAMPLES: ☒ TOP SOIL ☐ BOTTOM SOIL
DATE OF SAMPLES: 15 OCT 1964

TEST PIT

DEPTH OF HOLE: 10.0
TYPE OF SOIL: Gravelly Silt
CLASSIFICATION: Gravelly Silt
FORMATION DESCRIPTION & REMARKS: Organic plastic, gray brown.

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NOTES: 1. Location approximate, no survey control.
2. Classified in field, no laboratory tests at this time.

PROJECT: Snettisham PERMANENT TP 35
HOLE NO.

U. S. ARMY ENGINEER DISTRICT, ALASKA
CORPS OF ENGINEERS
ANCHORAGE, ALASKA

DESIGNED: *W. J. T. T. T.*
DRAWN: *W. J. T. T. T.*
CHECKED: *D. H. H.*
PREPARED: *M. G. G.*

SNETTISHAM PROJECT, ALASKA
FIRST STAGE DEVELOPMENT
TEST PITS
LOG RECORD NO. 2

APPROVED: *W. J. T. T. T.*
SCALE: *1" = 10'*
DATE: *15 OCT 1964*

FILE NUMBER: *100-100-100*

DESIGNED: *W. J. T. T. T.*
DRAWN: *W. J. T. T. T.*
CHECKED: *D. H. H.*
PREPARED: *M. G. G.*

DESIGNED: *W. J. T. T. T.*
DRAWN: *W. J. T. T. T.*
CHECKED: *D. H. H.*
PREPARED: *M. G. G.*

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CORPS OF ENGINEERS

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA
EXPLORATION LOG

PROJECT: SHELLISHAM SHEET: 1 OF 2
LOCATION (Griding & Survey): N 90,500 E 91,500
DATE: 10/1/50
TIME OF DAY: 10:00
TYPE OF HOLE: 1P 44
DEPTH: 15.0
TEST AT: 10' 15' 20' 25' 30' 35' 40' 45' 50' 55' 60' 65' 70' 75' 80' 85' 90' 95' 100'
TEST AND TIME OF DAY: 10:00
TYPE OF EQUIPMENT: Hyster Backhoe D-7 Cat
TOTAL NO. OF SAMPLES: 1
TIME OF SAMPLES: 10:00
EL. TOP OF HOLE: 10.0
EL. BOTTOM OF HOLE: 0.0

CLASSIFICATION: SILT
FORMATION DESCRIPTION & REMARKS: Organic, Plastic, dark blue

W.T. 10.0'

CLASSIFICATION: Gravelly SAND
FORMATION DESCRIPTION & REMARKS: Some cobbles

PROJECT: SHELLISHAM PERMANENT HOLE NO. TP44

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA
EXPLORATION LOG

PROJECT: SHELLISHAM SHEET: 2 OF 2
LOCATION (Griding & Survey): N 90,500 E 91,500
DATE: 10/1/50
TIME OF DAY: 10:00
TYPE OF HOLE: 1P 44
DEPTH: 15.0
TEST AT: 10' 15' 20' 25' 30' 35' 40' 45' 50' 55' 60' 65' 70' 75' 80' 85' 90' 95' 100'
TEST AND TIME OF DAY: 10:00
TYPE OF EQUIPMENT: Hyster Backhoe D-7 Cat
TOTAL NO. OF SAMPLES: 1
TIME OF SAMPLES: 10:00
EL. TOP OF HOLE: 10.0
EL. BOTTOM OF HOLE: 0.0

CLASSIFICATION: Gravelly SAND
FORMATION DESCRIPTION & REMARKS: Some cobbles

NOTE: 1. Location approximate, no survey control.
2. Classified in field, no laboratory tests at this time.

PROJECT: SHELLISHAM PERMANENT HOLE NO. TP 44

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA
EXPLORATION LOG

PROJECT: SHELLISHAM SHEET: 1 OF 1
LOCATION (Griding & Survey): N 90,180 E 91,020
DATE: 10/1/50
TIME OF DAY: 10:00
TYPE OF HOLE: 1P 45
DEPTH: 6.0
TEST AT: 10' 15' 20' 25' 30' 35' 40' 45' 50' 55' 60' 65' 70' 75' 80' 85' 90' 95' 100'
TEST AND TIME OF DAY: 10:00
TYPE OF EQUIPMENT: Hyster Backhoe D-7 Cat
TOTAL NO. OF SAMPLES: 1
TIME OF SAMPLES: 10:00
EL. TOP OF HOLE: 10.0
EL. BOTTOM OF HOLE: 0.0

CLASSIFICATION: SILT
FORMATION DESCRIPTION & REMARKS: Blue gray, plastic, organic

NOTE: 1. Location approximate, no survey control.

PROJECT: SHELLISHAM PERMANENT HOLE NO. TP 45

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA
EXPLORATION LOG

PROJECT: SHELLISHAM SHEET: 1 OF 1
LOCATION (Griding & Survey): N 90,180 E 91,020
DATE: 10/1/50
TIME OF DAY: 10:00
TYPE OF HOLE: 1P 45
DEPTH: 6.0
TEST AT: 10' 15' 20' 25' 30' 35' 40' 45' 50' 55' 60' 65' 70' 75' 80' 85' 90' 95' 100'
TEST AND TIME OF DAY: 10:00
TYPE OF EQUIPMENT: Hyster Backhoe D-7 Cat
TOTAL NO. OF SAMPLES: 1
TIME OF SAMPLES: 10:00
EL. TOP OF HOLE: 10.0
EL. BOTTOM OF HOLE: 0.0

CLASSIFICATION: SILT
FORMATION DESCRIPTION & REMARKS: Blue gray, plastic, organic

NOTE: 1. Location approximate, no survey control.

PROJECT: SHELLISHAM PERMANENT HOLE NO. TP 45

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA
EXPLORATION LOG

PROJECT: SHELLISHAM SHEET: 1 OF 1
LOCATION (Griding & Survey): N 90,450 E 91,350
DATE: 10/1/50
TIME OF DAY: 10:00
TYPE OF HOLE: 1P 48
DEPTH: 10.0
TEST AT: 10' 15' 20' 25' 30' 35' 40' 45' 50' 55' 60' 65' 70' 75' 80' 85' 90' 95' 100'
TEST AND TIME OF DAY: 10:00
TYPE OF EQUIPMENT: Hyster Backhoe D-7 Cat
TOTAL NO. OF SAMPLES: 1
TIME OF SAMPLES: 10:00
EL. TOP OF HOLE: 10.0
EL. BOTTOM OF HOLE: 0.0

CLASSIFICATION: SILT
FORMATION DESCRIPTION & REMARKS: Bluish gray, plastic, some sand, organic, marine

W.T. 10.0'

CLASSIFICATION: Silty SANDY GRAVEL
FORMATION DESCRIPTION & REMARKS: Cobble common, well rounded chiefly Qz Diorite, some Basalt

NOTE: 1. Location approximate, no survey control.
2. Classified in field, no laboratory tests at this time.

PROJECT: SHELLISHAM PERMANENT HOLE NO. TP48

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA
EXPLORATION LOG

PROJECT: SHELLISHAM SHEET: 1 OF 1
LOCATION (Griding & Survey): N 90,450 E 91,350
DATE: 10/1/50
TIME OF DAY: 10:00
TYPE OF HOLE: 1P 49
DEPTH: 12.0
TEST AT: 10' 15' 20' 25' 30' 35' 40' 45' 50' 55' 60' 65' 70' 75' 80' 85' 90' 95' 100'
TEST AND TIME OF DAY: 10:00
TYPE OF EQUIPMENT: Hyster Backhoe D-7 Cat
TOTAL NO. OF SAMPLES: 1
TIME OF SAMPLES: 10:00
EL. TOP OF HOLE: 10.0
EL. BOTTOM OF HOLE: 0.0

CLASSIFICATION: SILT
FORMATION DESCRIPTION & REMARKS: Plastic, organic, marine, blue gray

W.T. 10.0'

CLASSIFICATION: Silty Gravelly SAND
FORMATION DESCRIPTION & REMARKS: Coarse grained, angular, well rounded gravel, Qz Diorite, some Basalt

W.T. 10.0'

CLASSIFICATION: Silty SANDY GRAVEL
FORMATION DESCRIPTION & REMARKS: Well rounded and graded

NOTE: 1. Location approximate, no survey control.
2. Classified in field, no laboratory tests at this time.

PROJECT: SHELLISHAM PERMANENT HOLE NO. TP 49

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA
EXPLORATION LOG

PROJECT: SHELLISHAM SHEET: 1 OF 1
LOCATION (Griding & Survey): N 90,180 E 90,780
DATE: 10/1/50
TIME OF DAY: 10:00
TYPE OF HOLE: 1P 50
DEPTH: 12.0
TEST AT: 10' 15' 20' 25' 30' 35' 40' 45' 50' 55' 60' 65' 70' 75' 80' 85' 90' 95' 100'
TEST AND TIME OF DAY: 10:00
TYPE OF EQUIPMENT: Hyster Backhoe D-7 Cat
TOTAL NO. OF SAMPLES: 1
TIME OF SAMPLES: 10:00
EL. TOP OF HOLE: 10.0
EL. BOTTOM OF HOLE: 0.0

CLASSIFICATION: SILT
FORMATION DESCRIPTION & REMARKS: Organic plastic, marine, blue gray

W.T. 10.0'

CLASSIFICATION: Silty SANDY GRAVEL
FORMATION DESCRIPTION & REMARKS: Well rounded and graded

NOTE: 1. Location approximate, no survey control.
2. Classified in field, no laboratory tests at this time.

PROJECT: SHELLISHAM PERMANENT HOLE NO. TP 50

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA
EXPLORATION LOG

PROJECT: SHELLISHAM SHEET: 1 OF 1
LOCATION (Griding & Survey): N 90,180 E 90,780
DATE: 10/1/50
TIME OF DAY: 10:00
TYPE OF HOLE: 1P 50
DEPTH: 12.0
TEST AT: 10' 15' 20' 25' 30' 35' 40' 45' 50' 55' 60' 65' 70' 75' 80' 85' 90' 95' 100'
TEST AND TIME OF DAY: 10:00
TYPE OF EQUIPMENT: Hyster Backhoe D-7 Cat
TOTAL NO. OF SAMPLES: 1
TIME OF SAMPLES: 10:00
EL. TOP OF HOLE: 10.0
EL. BOTTOM OF HOLE: 0.0

CLASSIFICATION: SILT
FORMATION DESCRIPTION & REMARKS: Organic plastic, marine, blue gray

W.T. 10.0'

CLASSIFICATION: Silty SANDY GRAVEL
FORMATION DESCRIPTION & REMARKS: Well rounded and graded

NOTE: 1. Location approximate, no survey control.
2. Classified in field, no laboratory tests at this time.

PROJECT: SHELLISHAM PERMANENT HOLE NO. TP 50

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION CIVIL ENGINEER DISTRICT, ALASKA		PROJECT WATKINS DAM AND BRIDGE 1. VI-150 2. 100-150 3. 100-150 4. 100-150 5. 100-150 6. 100-150 7. 100-150 8. 100-150 9. 100-150 10. 100-150 11. 100-150 12. 100-150 13. 100-150 14. 100-150 15. 100-150 16. 100-150 17. 100-150 18. 100-150 19. 100-150 20. 100-150 21. 100-150 22. 100-150 23. 100-150 24. 100-150 25. 100-150 26. 100-150 27. 100-150 28. 100-150 29. 100-150 30. 100-150 31. 100-150 32. 100-150 33. 100-150 34. 100-150 35. 100-150 36. 100-150 37. 100-150 38. 100-150 39. 100-150 40. 100-150 41. 100-150 42. 100-150 43. 100-150 44. 100-150 45. 100-150 46. 100-150 47. 100-150 48. 100-150 49. 100-150 50. 100-150 51. 100-150 52. 100-150 53. 100-150 54. 100-150 55. 100-150 56. 100-150 57. 100-150 58. 100-150 59. 100-150 60. 100-150 61. 100-150 62. 100-150 63. 100-150 64. 100-150 65. 100-150 66. 100-150 67. 100-150 68. 100-150 69. 100-150 70. 100-150 71. 100-150 72. 100-150 73. 100-150 74. 100-150 75. 100-150 76. 100-150 77. 100-150 78. 100-150 79. 100-150 80. 100-150 81. 100-150 82. 100-150 83. 100-150 84. 100-150 85. 100-150 86. 100-150 87. 100-150 88. 100-150 89. 100-150 90. 100-150 91. 100-150 92. 100-150 93. 100-150 94. 100-150 95. 100-150 96. 100-150 97. 100-150 98. 100-150 99. 100-150 100. 100-150		SHEET NO. 1 OF 1	
EXPLORATION LOG DATE 10/10/50		NAME OF TOWNSHIP 100-150		NAME OF COUNTY 100-150	
TIME OF DAY 10:00 AM		TYPE OF SOIL 100-150		TYPE OF ROCK 100-150	
NUMBER OF SAMPLES 100-150		TYPE OF SAMPLES 100-150		TYPE OF SAMPLES 100-150	
LOCATION 100-150		CLASSIFICATION 100-150		FORMATION DESCRIPTION & REMARKS 100-150	
PROJECT 100-150		SHEET NO. 100-150		PERMANENT 100-150	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: <u>Classification</u> LOCATION: <u>Location in Field</u> NO. <u>YOUNG</u> NAME OF AGENT: <u> </u> NAME OF TRAILER: <u> </u>		DATE: <u> </u> TIME: <u> </u> NAME OF TRAILER: <u> </u>
EXPLANATION: <u>LOC</u>				
FIELD: <u>TP 47</u>	PERMANENT: <u>TP 47</u>	TYPE OF SOIL: <u> </u>		
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TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>
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TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>
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TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>
TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>
TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	TYPE OF SOIL: <u> </u>	

APPENDIX C

REPORT OF THE ARMY PACIFIC DIVISION CORPORAL JOSEPH A. ALDER		PROJECT <u>Classification</u>		DATE: <u>10/1/50</u>	
LOCATION <u>WGS</u>		U.S. 10-120 10-120 10-120		E 10-120	
REMARKS TP 59		10-120		10-120	
TYPE OF SOIL		TYPE OF SOIL		TYPE OF SOIL	
SILTY SAND		SILTY SAND		SILTY SAND	
CLASSIFICATION		CLASSIFICATION		CLASSIFICATION	
PSAT		PSAT		PSAT	
COBBLES SAND		COBBLES SAND		COBBLES SAND	
SILTY SAND		SILTY SAND		SILTY SAND	
Dark brown, fine to med grain and sand, no clay seen to 1.6 feet. Few cobbles and boulders much organic material		Dark brown, fine to med grain and sand, no clay seen to 1.6 feet. Few cobbles and boulders much organic material		Dark brown, fine to med grain and sand, no clay seen to 1.6 feet. Few cobbles and boulders much organic material	
NOTE: 1. Location approximate, no survey control. 2. Classified in field, no laboratory tests at this time.		NOTE: 1. Location approximate, no survey control. 2. Classified in field, no laboratory tests at this time.		NOTE: 1. Location approximate, no survey control. 2. Classified in field, no laboratory tests at this time.	
PROJECT <u>Classification</u>		PROJECT <u>Classification</u>		PROJECT <u>Classification</u>	
PERMANENT		PERMANENT		PERMANENT	

OFFICE		DATE		DISPOSITION		BY	
<p align="center">U. S. ARMY ENGINEER DISTRICT. ALASKA CORPS OF ENGINEERS ANCHORAGE ALASKA</p>							
DESIGNED: W. B. TRENTHAM DRAWN: W. B. TRENTHAM CHECKED: D. HANSEN PREPARED: CHD		SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT TEST PITS LOG RECORD NO. 5					
SUPERVISED: CHD SUBMITTED: CHD RECOMMENDED: CHD		APPROVED: (NAME, GRADE, BRANCH) SCALE DATE FILE NUMBER SHEET OF					

[illegible]

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA				PROJECT SWETHSHAN EXPLOSIVE Contaminant in Soil		DATE 1-7 E 94-080 EXPLOSIVE NO.	
EXPANSION LOG				DATE OF SAMPLE None		WEATHER Cloudy	
FIELD TP-61		PERMANENT TP-61		Slope		TEMPERATURE	
TYPE OF HOLE		TYPE OF HOLE		DEPTH		TEMPERATURE	
1. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
2. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
3. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
4. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
5. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
6. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
7. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
8. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
9. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
10. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
11. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
12. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
13. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
14. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
15. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
16. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
17. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
18. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
19. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
20. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
21. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
22. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
23. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
24. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
25. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
26. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
27. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
28. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
29. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
30. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
31. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
32. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
33. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
34. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
35. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
36. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
37. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
38. 1" x 1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE		1" (2) 1/2" HOLE	
39. 1" x 1" (2) 1/2" HOLE		1" (2					

DEPARTMENT OF THE ARMY						SHEET 1 OF 1	
NORTH PACIFIC DIVISION							
U.S. ARMY ENGINEER DISTRICT, ALASKA							
EXPLORATION LOG							
P.M.F.	TP-62	Personnel	TP-62	Morgan		Railay	
1950	DATE	TYPE OF HILL	<input type="checkbox"/> CONE HILL	<input checked="" type="checkbox"/> MOUND	TIME	START OF DAY	END OF DAY
1	YD BACKHOLE BURIAL	<input type="checkbox"/> YES	<input type="checkbox"/> NO	TYPE OF WEATHERING	HYDRIC BACKHOLE	D-7 CAT	
2	NO. OF STATIONS	TYPE OF CORRELATION	<input type="checkbox"/> DISTRIBUTED	<input type="checkbox"/> CONTIGUOUS	WIND DIRECTION	SPEED	TEMPERATURE
3	E. W. OF HILL NUMBER	J. M. KAGE	Background Source	Field Description & Remarks Section			
DEPTH FT.	Cross-section No. or Station	CLASSIFICATION	SPR	SYNOPSIS DESCRIPTION & REMARKS			
		Bouldery GRAVEL		Subrounded boulders to 40", and coarse grained sand, small amounts of silt.			
1							
2							
3							
4							
5							
6				Digging somewhat easier below 6.0', but still many boulders.			
7							
8							
9							
10				Refuse?	Due to boulders		
NOTE: 1. Location approximate, no survey control.							
2. Field classification, no laboratory tests at this time.							
PREPARED BY (NAME) DATE PROJECT SKEETISSAN HOLDING TP-62 PERMANENT HOLDING TP-62							

DEPOSIT	
NAME U.S. ARMY	
NO.	
POLY	
Borrow	
TOTAL AMOUNT	
DATE	
TOTAL NO OF PAGES	
DATE	PAGE
1	
2	
3	
4	
5	
6	
7	
8	
9	
10	
11	
12	
13	
14	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA				PROJECT (SECOND NUMBERING NUMBER) N 94,690 (PLANT NUMBER)		SHEET 1 OF 1 E 95,310 (TOTAL NUMBER OF SHEETS)	
EXPLOSION LOG							
FIELD NO. TP-69		PLACEMENT TP-65		NAME OF SOIL/ICE (1) SOILS		NUMBER Sunny Warm	
TYPE OF HOLE		TYPE OF SOIL/ICE		TYPE OF EQUIPMENT		TYPE OF HOLE	
PICK AND SHOVE (Pick and shovel)		CHAIN HOIST (Chain hoist)		PICK AND SHOVE (Pick and shovel)		PICK AND SHOVE (Pick and shovel)	
TYPE OF SAMPLE		TYPE OF SAMPLE		TYPE OF SAMPLE		TYPE OF SAMPLE	
None		None		None		None	
LOCATION OF HOLE		LOCATION OF HOLE		LOCATION OF HOLE		LOCATION OF HOLE	
NORTH (NORTH ARROW)		SAMPLE NO.		CLASSIFICATION		FORMATION DESCRIPTION & REMARKS	
1.0		PEAT		Brown to Black, Slightly Plastic, Improvised			
2.0							
3.0							
4.0		Sandy PEAT		Black to Brown, Fine Grained Sand			
		Peaty SAND		Fine Grained, Rust Stained Weathered Sand			
5.0							
		SAND		Fine Grained, Tan, Small Amounts of Black Mineral. Well Drained.			
6.0							
NOTES: (1) Water Table not reached, penetration water hole approx 10 ft. Max depth, 2 ft deep - no water (q test pit). (2) Location approx, no survey control.							
PROJECT SMITHMAN - LOWER CAMP SITE				PERMANENT FIELD NO. TP-69			

DEPARTMENT OF THE ARMY				PROJECT <u>SNETTISHAM</u>		POST: <u>1</u>	
NORTH PACIFIC DIVISION				LOCATION (Grid or Name):		Elev. <u>95,370</u>	
U.S. ARMY ENGINEER DIST. ALASKA				M. N. <u>4,860</u>		E.S. CAMP. <u>1</u>	
EXPLOSION LOG				M. N. <u>4,860</u>		E.S. CAMP. <u>1</u>	
EXPLOSION LOG				M. N. <u>4,860</u>		E.S. CAMP. <u>1</u>	
PELO		IP-70		PERMANENT		IP-70	
TYPE OF HOLE		TYPE OF HOLE		TYPE OF HOLE		TYPE OF HOLE	
TEST #1		TEST #2		TEST #3		TEST #4	
TEST #5		TEST #6		TEST #7		TEST #8	
TEST #9		TEST #10		TEST #11		TEST #12	
TEST #13		TEST #14		TEST #15		TEST #16	
TEST #17		TEST #18		TEST #19		TEST #20	
TEST #21		TEST #22		TEST #23		TEST #24	
TEST #25		TEST #26		TEST #27		TEST #28	
TEST #29		TEST #30		TEST #31		TEST #32	
TEST #33		TEST #34		TEST #35		TEST #36	
TEST #37		TEST #38		TEST #39		TEST #40	
TEST #41		TEST #42		TEST #43		TEST #44	
TEST #45		TEST #46		TEST #47		TEST #48	
TEST #49		TEST #50		TEST #51		TEST #52	
TEST #53		TEST #54		TEST #55		TEST #56	
TEST #57		TEST #58		TEST #59		TEST #60	
TEST #61		TEST #62		TEST #63		TEST #64	
TEST #65		TEST #66		TEST #67		TEST #68	
TEST #69		TEST #70		TEST #71		TEST #72	
TEST #73		TEST #74		TEST #75		TEST #76	
TEST #77		TEST #78		TEST #79		TEST #80	
TEST #81		TEST #82		TEST #83		TEST #84	
TEST #85		TEST #86		TEST #87		TEST #88	
TEST #89		TEST #90		TEST #91		TEST #92	
TEST #93		TEST #94		TEST #95		TEST #96	
TEST #97		TEST #98		TEST #99		TEST #100	
TEST #101		TEST #102		TEST #103		TEST #104	
TEST #105		TEST #106		TEST #107		TEST #108	
TEST #109		TEST #110		TEST #111		TEST #112	
TEST #113		TEST #114		TEST #115		TEST #116	
TEST #117		TEST #118		TEST #119		TEST #120	
TEST #121		TEST #122		TEST #123		TEST #124	
TEST #125		TEST #126		TEST #127		TEST #128	
TEST #129		TEST #130		TEST #131		TEST #132	
TEST #133		TEST #134		TEST #135		TEST #136	
TEST #137		TEST #138		TEST #139		TEST #140	
TEST #141		TEST #142		TEST #143		TEST #144	
TEST #145		TEST #146		TEST #147		TEST #148	
TEST #149		TEST #150		TEST #151		TEST #152	
TEST #153		TEST #154		TEST #155		TEST #156	
TEST #157		TEST #158		TEST #159		TEST #160	
TEST #161		TEST #162		TEST #163		TEST #164	
TEST #165		TEST #166		TEST #167		TEST #168	
TEST #169		TEST #170		TEST #171		TEST #172	
TEST #173		TEST #174		TEST #175		TEST #176	
TEST #177		TEST #178		TEST #179		TEST #180	
TEST #181		TEST #182		TEST #183		TEST #184	
TEST #185		TEST #186		TEST #187		TEST #188	
TEST #189		TEST #190		TEST #191		TEST #192	
TEST #193		TEST #194		TEST #195		TEST #196	
TEST #197		TEST #198		TEST #199		TEST #200	
TEST #201		TEST #202		TEST #203		TEST #204	
TEST #205		TEST #206		TEST #207		TEST #208	
TEST #209		TEST #210		TEST #211		TEST #212	
TEST #213		TEST #214		TEST #215		TEST #216	
TEST #217		TEST #218		TEST #219		TEST #220	
TEST #221		TEST #222		TEST #223		TEST #224	
TEST #225		TEST #226		TEST #227		TEST #228	
TEST #229							

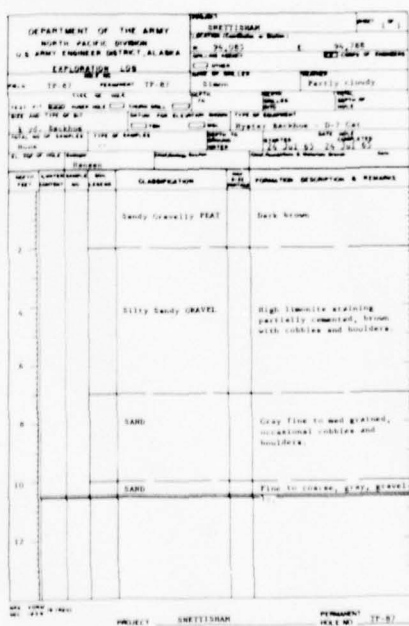
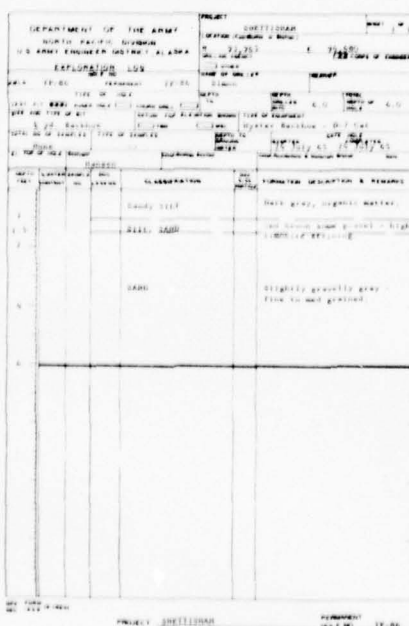
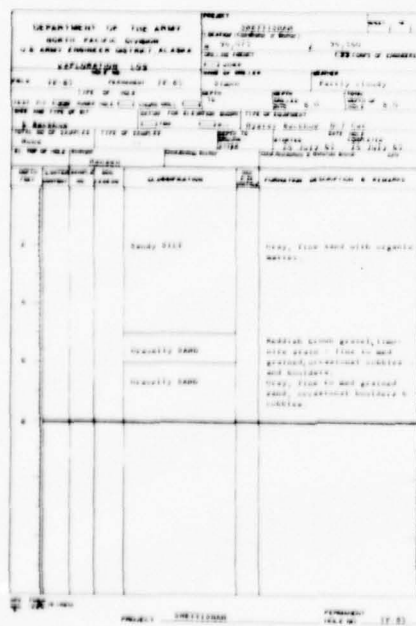
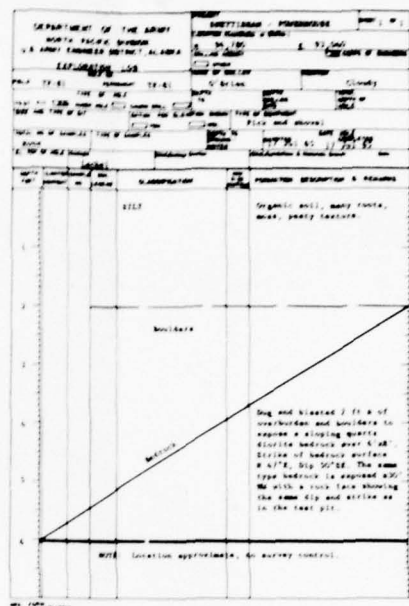
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DEPARTMENT OF THE ARMY U. S. ARMY ENGINEERING CENTER EXPENDITURE REPORT	
PROJECT NO.	FF-72
TEST NAME	Hand Dig
DATE AND TIME OF TEST	TOTAL NO. OF SAMPLES
NOTES	NO. OF HRS. TESTED
DEPTH	WATER SAMPLED
	FOOT
1.0	
2.0	
3.0	
4.0	
5.0	
NOTE:	

DATE: FEBRUARY 19, 1962
 BY: 1535

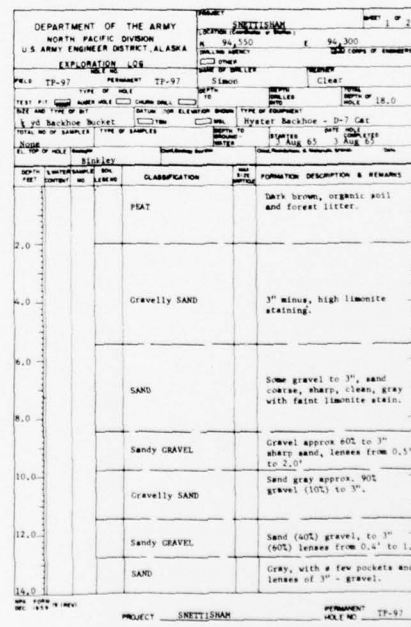
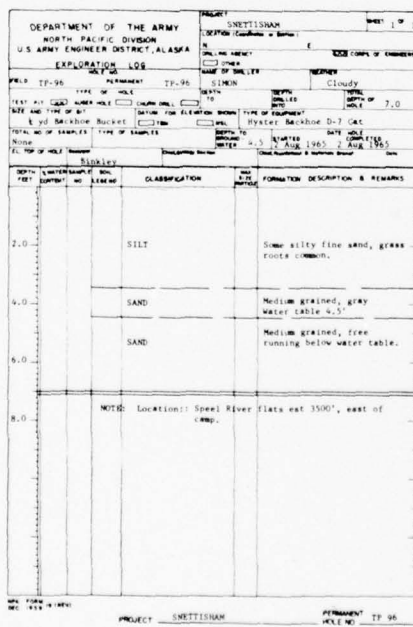
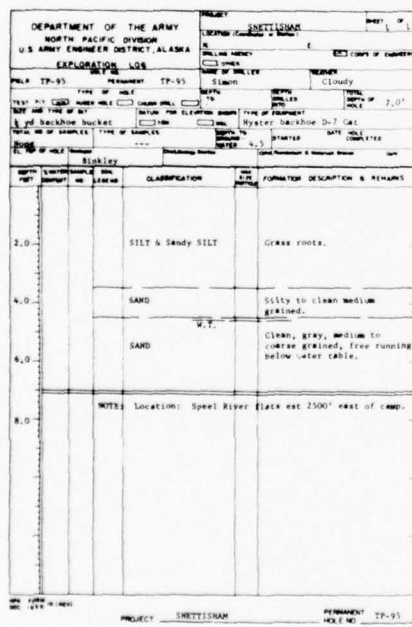
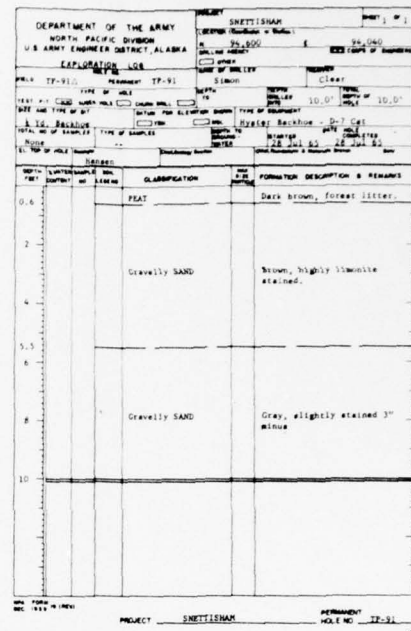
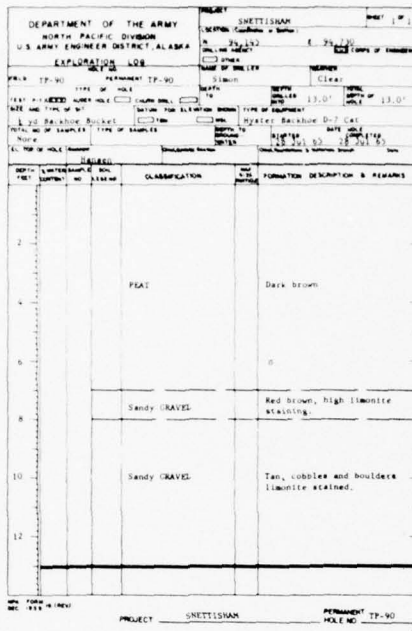
[illegible][illegible]

DESIGNED: <i>W. S. TERTREY</i>		SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT TEST PITS LOG RECORDS NO. 6	APPROVED: <i>Base & Sailing</i>
DRAWN: <i>W. S. TERTREY</i>			COLOUR, OR SURFACE FINISHES
CHECKED: <i>D. Hanson</i>			SCALE
PREPARED: 			DATE <i>20 Apr 65</i>
DATE SUPERVISED <i>M. E. Tilton</i>			FILE NUMBER
SURVEYED: 		METHOD	
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
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SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			
PREPARED 			
DATE SUPERVISED <i>M. E. Tilton</i>			
SURVEYED: 			
DRAWN <i>W. S. TERTREY</i>			
CHECKED <i>D. Hanson</i>			



[illegible]

DESIGNED: <i>Exp. B. T. Smith</i>		SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT TEST PITS LOG RECORDS NO. 7	APPROVED: <i>Case J. Taylor</i>
DRAWN: <i>Exp. B. T. Smith</i>			COUNTRY, OR PROJECT NUMBER
CHECKED: <i>D. Hovson</i>			SCALE
REPAIRED:			DATE <i>20 Apr 66</i>
DATE			FILE NUMBER
SUPERVISED: <i>W. D. Brown</i>		MATERIALS BRANCH SCALE SHEET OF	
SUBMITTED: <i>W. D. Brown</i>			
RECOMMENDED: <i>W. D. Brown</i>			



DEPARTMENT OF THE ARMY		SPEC. 1	
NORTH PACIFIC DIVISION		SPEC. 2	
LABOR ENGINEER DISTRICT ALASKA		SPEC. 3	
EXPIRATION DATE		SPEC. 4	
TP-93	PERMANENT	TP-93	
DATE	AMOUNT	DATE	AMOUNT
1944	100.00	1944	100.00
1945	100.00	1945	100.00
1946	100.00	1946	100.00
1947	100.00	1947	100.00
1948	100.00	1948	100.00
1949	100.00	1949	100.00
1950	100.00	1950	100.00
1951	100.00	1951	100.00
1952	100.00	1952	100.00
1953	100.00	1953	100.00
1954	100.00	1954	100.00
1955	100.00	1955	100.00
1956	100.00	1956	100.00
1957	100.00	1957	100.00
1958	100.00	1958	100.00
1959	100.00	1959	100.00
1960	100.00	1960	100.00
1961	100.00	1961	100.00
1962	100.00	1962	100.00
1963	100.00	1963	100.00
1964	100.00	1964	100.00
1965	100.00	1965	100.00
1966	100.00	1966	100.00
1967	100.00	1967	100.00
1968	100.00	1968	100.00
1969	100.00	1969	100.00
1970	100.00	1970	100.00
1971	100.00	1971	100.00
1972	100.00	1972	100.00
1973	100.00	1973	100.00
1974	100.00	1974	100.00
1975	100.00	1975	100.00
1976	100.00	1976	100.00
1977	100.00	1977	100.00
1978	100.00	1978	100.00
1979	100.00	1979	100.00
1980	100.00	1980	100.00
1981	100.00	1981	100.00
1982	100.00	1982	100.00
1983	100.00	1983	100.00
1984	100.00	1984	100.00
1985	100.00	1985	100.00
1986	100.00	1986	100.00
1987	100.00	1987	100.00
1988	100.00	1988	100.00
1989	100.00	1989	100.00
1990	100.00	1990	100.00
1991	100.00	1991	100.00
1992	100.00	1992	100.00
1993	100.00	1993	100.00
1994	100.00	1994	100.00
1995	100.00	1995	100.00
1996	100.00	1996	100.00
1997	100.00	1997	100.00
1998	100.00	1998	100.00
1999	100.00	1999	100.00
2000	100.00	2000	100.00
2001	100.00	2001	100.00
2002	100.00	2002	100.00
2003	100.00	2003	100.00
2004	100.00	2004	100.00
2005	100.00	2005	100.00
2006	100.00	2006	100.00
2007	100.00	2007	100.00
2008	100.00	2008	100.00
2009	100.00	2009	100.00
2010	100.00	2010	100.00
2011	100.00	2011	100.00
2012	100.00	2012	100.00
2013	100.00	2013	100.00
2014	100.00	2014	100.00
2015	100.00	2015	100.00
2016	100.00	2016	100.00
2017	100.00	2017	100.00
2018	100.00	2018	100.00
2019	100.00	2019	100.00

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT ALASKA		PROJECT <u>NETTISHAN</u> (REQUIRED: Containing a title)		SHEET <u>1</u> OF <u>1</u>
EXPLORATION LOG (REQUIRED)		(OPTIONAL) <input type="checkbox"/> <u>PROBES</u> NAME OF HOLEY <u> </u>		(OPTIONAL) <input type="checkbox"/> <u>TESTS OF MATERIALS</u>
DATE <u>TP-10</u>	TIME OF DAY <u> </u>	LOCATION <u>TP-10</u>	TYPE OF HOLE <u> </u>	DEPTH <u> </u>
(1) <input type="checkbox"/> <u>DRILL</u> (2) <input type="checkbox"/> <u>SHOULDER</u> (3) <input type="checkbox"/> <u>SHOULDER</u> (4) <input type="checkbox"/> <u>SHOULDER</u>	(5) <input type="checkbox"/> <u>SHOULDER</u> (6) <input type="checkbox"/> <u>SHOULDER</u> (7) <input type="checkbox"/> <u>SHOULDER</u>	(8) <input type="checkbox"/> <u>SHOULDER</u> (9) <input type="checkbox"/> <u>SHOULDER</u> (10) <input type="checkbox"/> <u>SHOULDER</u>	(11) <input type="checkbox"/> <u>SHOULDER</u> (12) <input type="checkbox"/> <u>SHOULDER</u> (13) <input type="checkbox"/> <u>SHOULDER</u>	(14) <input type="checkbox"/> <u>SHOULDER</u> (15) <input type="checkbox"/> <u>SHOULDER</u> (16) <input type="checkbox"/> <u>SHOULDER</u>
(17) <input type="checkbox"/> <u>SHOULDER</u> (18) <input type="checkbox"/> <u>SHOULDER</u> (19) <input type="checkbox"/> <u>SHOULDER</u> (20) <input type="checkbox"/> <u>SHOULDER</u>		(21) <input type="checkbox"/> <u>SHOULDER</u> (22) <input type="checkbox"/> <u>SHOULDER</u> (23) <input type="checkbox"/> <u>SHOULDER</u> (24) <input type="checkbox"/> <u>SHOULDER</u>		(25) <input type="checkbox"/> <u>SHOULDER</u> (26) <input type="checkbox"/> <u>SHOULDER</u> (27) <input type="checkbox"/> <u>SHOULDER</u> (28) <input type="checkbox"/> <u>SHOULDER</u>
(29) <input type="checkbox"/> <u>SHOULDER</u> (30) <input type="checkbox"/> <u>SHOULDER</u> (31) <input type="checkbox"/> <u>SHOULDER</u> (32) <input type="checkbox"/> <u>SHOULDER</u>		(33) <input type="checkbox"/> <u>SHOULDER</u> (34) <input type="checkbox"/> <u>SHOULDER</u> (35) <input type="checkbox"/> <u>SHOULDER</u> (36) <input type="checkbox"/> <u>SHOULDER</u>		(37) <input type="checkbox"/> <u>SHOULDER</u> (38) <input type="checkbox"/> <u>SHOULDER</u> (39) <input type="checkbox"/> <u>SHOULDER</u> (40) <input type="checkbox"/> <u>SHOULDER</u>
(41) <input type="checkbox"/> <u>SHOULDER</u> (42) <input type="checkbox"/> <u>SHOULDER</u> (43) <input type="checkbox"/> <u>SHOULDER</u> (44) <input type="checkbox"/> <u>SHOULDER</u>		(45) <input type="checkbox"/> <u>SHOULDER</u> (46) <input type="checkbox"/> <u>SHOULDER</u> (47) <input type="checkbox"/> <u>SHOULDER</u> (48) <input type="checkbox"/> <u>SHOULDER</u>		(49) <input type="checkbox"/> <u>SHOULDER</u> (50) <input type="checkbox"/> <u>SHOULDER</u> (51) <input type="checkbox"/> <u>SHOULDER</u> (52) <input type="checkbox"/> <u>SHOULDER</u>
(53) <input type="checkbox"/> <u>SHOULDER</u> (54) <input type="checkbox"/> <u>SHOULDER</u> (55) <input type="checkbox"/> <u>SHOULDER</u> (56) <input type="checkbox"/> <u>SHOULDER</u>		(57) <input type="checkbox"/> <u>SHOULDER</u> (58) <input type="checkbox"/> <u>SHOULDER</u> (59) <input type="checkbox"/> <u>SHOULDER</u> (60) <input type="checkbox"/> <u>SHOULDER</u>		(61) <input type="checkbox"/> <u>SHOULDER</u> (62) <input type="checkbox"/> <u>SHOULDER</u> (63) <input type="checkbox"/> <u>SHOULDER</u> (64) <input type="checkbox"/> <u>SHOULDER</u>
(65) <input type="checkbox"/> <u>SHOULDER</u> (66) <input type="checkbox"/> <u>SHOULDER</u> (67) <input type="checkbox"/> <u>SHOULDER</u> (68) <input type="checkbox"/> <u>SHOULDER</u>		(69) <input type="checkbox"/> <u>SHOULDER</u> (70) <input type="checkbox"/> <u>SHOULDER</u> (71) <input type="checkbox"/> <u>SHOULDER</u> (72) <input type="checkbox"/> <u>SHOULDER</u>		(73) <input type="checkbox"/> <u>SHOULDER</u> (74) <input type="checkbox"/> <u>SHOULDER</u> (75) <input type="checkbox"/> <u>SHOULDER</u> (76) <input type="checkbox"/> <u>SHOULDER</u>
(77) <input type="checkbox"/> <u>SHOULDER</u> (78) <input type="checkbox"/> <u>SHOULDER</u> (79) <input type="checkbox"/> <u>SHOULDER</u> (80) <input type="checkbox"/> <u>SHOULDER</u>		(81) <input type="checkbox"/> <u>SHOULDER</u> (82) <input type="checkbox"/> <u>SHOULDER</u> (83) <input type="checkbox"/> <u>SHOULDER</u> (84) <input type="checkbox"/> <u>SHOULDER</u>		(85) <input type="checkbox"/> <u>SHOULDER</u> (86) <input type="checkbox"/> <u>SHOULDER</u> (87) <input type="checkbox"/> <u>SHOULDER</u> (88) <input type="checkbox"/> <u>SHOULDER</u>
(89) <input type="checkbox"/> <u>SHOULDER</u> (90) <input type="checkbox"/> <u>SHOULDER</u> (91) <input type="checkbox"/> <u>SHOULDER</u> (92) <input type="checkbox"/> <u>SHOULDER</u>		(93) <input type="checkbox"/> <u>SHOULDER</u> (94) <input type="checkbox"/> <u>SHOULDER</u> (95) <input type="checkbox"/> <u>SHOULDER</u> (96) <input type="checkbox"/> <u>SHOULDER</u>		(97) <input type="checkbox"/> <u>SHOULDER</u> (98) <input type="checkbox"/> <u>SHOULDER</u> (99) <input type="checkbox"/> <u>SHOULDER</u> (100) <input type="checkbox"/> <u>SHOULDER</u>
(101) <input type="checkbox"/> <u>SHOULDER</u> (102) <input type="checkbox"/> <u>SHOULDER</u> (103) <input type="checkbox"/> <u>SHOULDER</u> (104) <input type="checkbox"/> <u>SHOULDER</u>		(105) <input type="checkbox"/> <u>SHOULDER</u> (106) <input type="checkbox"/> <u>SHOULDER</u> (107) <input type="checkbox"/> <u>SHOULDER</u> (108) <input type="checkbox"/> <u>SHOULDER</u>		(109) <input type="checkbox"/> <u>SHOULDER</u> (110) <input type="checkbox"/> <u>SHOULDER</u> (111) <input type="checkbox"/> <u>SHOULDER</u> (112) <input type="checkbox"/> <u>SHOULDER</u>
(113) <input type="checkbox"/> <u>SHOULDER</u> (114) <input type="checkbox"/> <u>SHOULDER</u> (115) <input type="checkbox"/> <u>SHOULDER</u> (116) <input type="checkbox"/> <u>SHOULDER</u>		(117) <input type="checkbox"/> <u>SHOULDER</u> (118) <input type="checkbox"/> <u>SHOULDER</u> (119) <input type="checkbox"/> <u>SHOULDER</u> (120) <input type="checkbox"/> <u>SHOULDER</u>		(121) <input type="checkbox"/> <u>SHOULDER</u> (122) <input type="checkbox"/> <u>SHOULDER</u> (123) <input type="checkbox"/> <u>SHOULDER</u> (124) <input type="checkbox"/> <u>SHOULDER</u>
(125) <input type="checkbox"/> <u>SHOULDER</u> (126) <input type="checkbox"/> <u>SHOULDER</u> (127) <input type="checkbox"/> <u>SHOULDER</u> (128) <input type="checkbox"/> <u>SHOULDER</u>		(129) <input type="checkbox"/> <u>SHOULDER</u> (130) <input type="checkbox"/> <u>SHOULDER</u> (131) <input type="checkbox"/> <u>SHOULDER</u> (132) <input type="checkbox"/> <u>SHOULDER</u>		(133) <input type="checkbox"/> <u>SHOULDER</u> (134) <input type="checkbox"/> <u>SHOULDER</u> (135) <input type="checkbox"/> <u>SHOULDER</u> (136) <input type="checkbox"/> <u>SHOULDER</u>
(137) <input type="checkbox"/> <u>SHOULDER</u> (138) <input type="checkbox"/> <u>SHOULDER</u> (139) <input type="checkbox"/> <u>SHOULDER</u> (140) <input type="checkbox"/> <u>SHOULDER</u>		(141) <input type="checkbox"/> <u>SHOULDER</u> (142) <input type="checkbox"/> <u>SHOULDER</u> (143) <input type="checkbox"/> <u>SHOULDER</u> (144) <input type="checkbox"/> <u>SHOULDER</u>		(145) <input type="checkbox"/> <u>SHOULDER</u> (146) <input type="checkbox"/> <u>SHOULDER</u> (147) <input type="checkbox"/> <u>SHOULDER</u> (148) <input type="checkbox"/> <u>SHOULDER</u>
(149) <input type="checkbox"/> <u>SHOULDER</u> (150) <input type="checkbox"/> <u>SHOULDER</u> (151) <input type="checkbox"/> <u>SHOULDER</u> (152) <input type="checkbox"/> <u>SHOULDER</u>		(153) <input type="checkbox"/> <u>SHOULDER</u> (154) <input type="checkbox"/> <u>SHOULDER</u> (155) <input type="checkbox"/> <u>SHOULDER</u> (156) <input type="checkbox"/> <u>SHOULDER</u>		

[illegible]

U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: <i>Wm. G. Tenthum</i> DRAWN: <i>Wm. G. Tenthum</i> CHECKED: <i>D. Henica</i> PREPARED: DATE: SUPERVISOR: <i>W. G. Tenthum</i> DATE: <i>10-1-54</i> SUBMITTED: <i>Wm. G. Tenthum</i> RECOMMENDED: <i>Wm. G. Tenthum</i>	SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT TEST PITS LOG RECORDS NO. 8 APPROVED: <i>Wm. G. Tenthum</i> SCALE FILE NUMBER SHEET OF

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: SNETTISHAM
HOLE NO. 1

DATE: 1958
TIME: 17:00
WEATHER: Partly Cloudy
WIND: 22'

TYPE OF HOLE: ☐ SHALLOW ☐ DEEP
METHOD OF BORING: ☐ HAND ☐ POWER
EQUIPMENT: ☐ 5 yd. Backhoe Bucket ☐ Hydrant Backhoe - D-7 Cat
STARTER: ☐ F Aug 1958 ☐ F Aug 1958

DEPTH (feet)	CLASSIFICATION	FORMATION DESCRIPTION & REMARKS
0.0	PEAT	Brown
4.0	Gravelly SAND	Brown limonite stained.
10.0	Gravelly SAND	Gray, fine to medium grained sand, 2" minus less than 10%.
12.0	Gravelly SAND	Gray, medium to coarse sand, 2" minus. Gravel 20 - 25%

FORM 100-1 (REV. 1-54)
PROJECT: SNETTISHAM
PERMANENT HOLE NO. TP-98

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: SNETTISHAM
HOLE NO. 2

DATE: 1958
TIME: 17:00
WEATHER: Partly Cloudy
WIND: 22'

TYPE OF HOLE: ☐ SHALLOW ☐ DEEP
METHOD OF BORING: ☐ HAND ☐ POWER
EQUIPMENT: ☐ 5 yd. Backhoe Bucket ☐ Hydrant Backhoe - D-7 Cat
STARTER: ☐ F Aug 1958 ☐ F Aug 1958

DEPTH (feet)	CLASSIFICATION	FORMATION DESCRIPTION & REMARKS
16.0	Gravelly SAND	
18.0		
20.0		
22.0		
24.0		
26.0		
28.0		

FORM 100-1 (REV. 1-54)
PROJECT: SNETTISHAM
PERMANENT HOLE NO. TP-98

DESIGNED: R. B. TRENKLE		SNETTISHAM PROJECT, ALASKA	
DRAWN: R. B. TRENKLE		FIRST STAGE DEVELOPMENT	
CHECKED: D. Hansen		TEST PITS	
PREPARED:		LOG RECORDS NO. 9	
SUPERVISOR: M. B. DeLeon		APPROVED: <i>[Signature]</i>	
SUBMITTED: R. B. TRENKLE		SCALE: _____ DATE: 20 Oct 65	
RECOMMENDED: T. B. TRENKLE		SHEET _____ OF _____	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT Shettisham		SHEET 1 OF 1	
EXPLORATION LOG					
FIELD LLTP 1		PERMANENT TP 5		NAME OF TUNNEL Nielsen and Caskey	
TEST PT. 1		TYPE OF HOLE Auger hole		DEPTH OF HOLE 2.5	
TEST AND TIME OF DAY		DATE 10 Sep		TIME 10:00	
TOTAL NO. OF SAMPLES		TYPE OF SAMPLES		DATE 10 Sep	
EL. TOP OF HOLE		DISTURBED		STARTED 10 Sep	
SIL. 1		S. Sperry		DATE 10 Sep	
NORTH LATITUDE		EAST LONGITUDE		ELEVATION	
CLASSIFICATION		FORMATION DESCRIPTION & REMARKS			
1.0		SAND	Water table varies with lake level		
2.0		SP-SM Gravelly SAND	Some organic material such as twigs and branches in sample.		
Note: 1. Location approximate, no survey control.					
Temporary designation		LLTP 1			
PROJECT		Shettisham		PERMANENT TP 5	
HOLE NO.					

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT Shettisham		SHEET 1 OF 1	
EXPLORATION LOG					
FIELD LLTP 2		PERMANENT TP 6		NAME OF TUNNEL Nielsen and Caskey	
TEST PT. 1		TYPE OF HOLE Auger hole		DEPTH OF HOLE 2.0	
TEST AND TIME OF DAY		DATE 10 Sep		TIME 10:00	
TOTAL NO. OF SAMPLES		TYPE OF SAMPLES		DATE 10 Sep	
EL. TOP OF HOLE		DISTURBED		STARTED 10 Sep	
SIL. 2		S. Sperry		DATE 10 Sep	
NORTH LATITUDE		EAST LONGITUDE		ELEVATION	
CLASSIFICATION		FORMATION DESCRIPTION & REMARKS			
1.0		SAND	Water table		
2.0			Some pebbles		
Note: 1. Location approximate, no survey control.					
Temporary designation		LLTP 2			
PROJECT		Shettisham		PERMANENT TP 6	
HOLE NO.					

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT Shettisham		SHEET 1 OF 1	
EXPLORATION LOG					
FIELD LLTP 3		PERMANENT TP 7		NAME OF TUNNEL Nielsen and Caskey	
TEST PT. 1		TYPE OF HOLE Auger hole		DEPTH OF HOLE 2.0	
TEST AND TIME OF DAY		DATE 10 Sep		TIME 10:00	
TOTAL NO. OF SAMPLES		TYPE OF SAMPLES		DATE 10 Sep	
EL. TOP OF HOLE		DISTURBED		STARTED 10 Sep	
SIL. 3		S. Sperry		DATE 10 Sep	
NORTH LATITUDE		EAST LONGITUDE		ELEVATION	
CLASSIFICATION		FORMATION DESCRIPTION & REMARKS			
1.0		SAND	Water table		
2.0		SANDY GRAVEL	2"		
Note: 1. Location approximate, no survey control.					
Temporary designation		LLTP 3			
PROJECT		Shettisham		PERMANENT TP 7	
HOLE NO.					

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT Shettisham		SHEET 1 OF 1	
EXPLORATION LOG					
FIELD LLTP 6		PERMANENT TP 10		NAME OF TUNNEL Caskey and Nielsen	
TEST PT. 1		TYPE OF HOLE Auger hole		DEPTH OF HOLE 1.8	
TEST AND TIME OF DAY		DATE 10 Sep		TIME 10:00	
TOTAL NO. OF SAMPLES		TYPE OF SAMPLES		DATE 10 Sep	
EL. TOP OF HOLE		DISTURBED		STARTED 10 Sep	
SIL. 6		S. Sperry		DATE 10 Sep	
NORTH LATITUDE		EAST LONGITUDE		ELEVATION	
CLASSIFICATION		FORMATION DESCRIPTION & REMARKS			
1.0		GF	SANDY GRAVEL 3"		
2.0					
Note: 1. Location approximate, no survey control.					
Temporary designation		LLTP 6			
PROJECT		Shettisham		PERMANENT TP 10	
HOLE NO.					

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT Shettisham		SHEET 1 OF 1	
EXPLORATION LOG					
FIELD LLTP 7		PERMANENT TP 11		NAME OF TUNNEL Caskey and Nielsen	
TEST PT. 1		TYPE OF HOLE Auger hole		DEPTH OF HOLE 2.2	
TEST AND TIME OF DAY		DATE 10 Sep		TIME 10:00	
TOTAL NO. OF SAMPLES		TYPE OF SAMPLES		DATE 10 Sep	
EL. TOP OF HOLE		DISTURBED		STARTED 10 Sep	
SIL. 7		S. Sperry		DATE 10 Sep	
NORTH LATITUDE		EAST LONGITUDE		ELEVATION	
CLASSIFICATION		FORMATION DESCRIPTION & REMARKS			
1.0		Silty SAND	5"		
2.0		SANDY GRAVEL	Water table		
Note: 1. Location approximate, no survey control.					
Temporary designation		LLTP 7			
PROJECT		Shettisham		PERMANENT TP 11	
HOLE NO.					

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT Shettisham		SHEET 1 OF 1	
EXPLORATION LOG					
FIELD LLTP 8		PERMANENT TP 12		NAME OF TUNNEL Caskey and Nielsen	
TEST PT. 1		TYPE OF HOLE Auger hole		DEPTH OF HOLE 2.3	
TEST AND TIME OF DAY		DATE 10 Sep		TIME 10:00	
TOTAL NO. OF SAMPLES		TYPE OF SAMPLES		DATE 10 Sep	
EL. TOP OF HOLE		DISTURBED		STARTED 10 Sep	
SIL. 8		S. Sperry		DATE 10 Sep	
NORTH LATITUDE		EAST LONGITUDE		ELEVATION	
CLASSIFICATION		FORMATION DESCRIPTION & REMARKS			
1.0		Silty SAND	5"		
2.0		SANDY GRAVEL	2"		
Note: 1. Location approximate, no survey control.					
Temporary designation		LLTP 8			
PROJECT		Shettisham		PERMANENT TP 12	
HOLE NO.					

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Sleetlikham SHEET: #1
LOCATION (Gridline & Station):
EXPLOSION LOG
PERM. LLTP 10 PERMANENT TP 10
TEST AT: 1.0 TYPE OF HOLE: 1.0 DEPTH: 1.0
SIZE AND TYPE OF BT: 1.0
TOTAL NO. OF SAMPLES: 1.0
DATE: 1.0
TIME: 1.0
CLASSIFICATION: 1.0
FORMATION DESCRIPTION & REMARKS: 1.0
Note: 1. Location approximate, no survey control.

Temporary designation LLTP 10

PROJECT: Sleetlikham PERMANENT TP 10
HOLE NO. 10

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Sleetlikham SHEET: #1
LOCATION (Gridline & Station):
EXPLOSION LOG
PERM. LLTP 11 PERMANENT TP 11
TEST AT: 1.0 TYPE OF HOLE: 1.0 DEPTH: 1.0
SIZE AND TYPE OF BT: 1.0
TOTAL NO. OF SAMPLES: 1.0
DATE: 1.0
TIME: 1.0
CLASSIFICATION: 1.0
FORMATION DESCRIPTION & REMARKS: 1.0
Note: 1. Location approximate, no survey control.

Temporary designation LLTP 11

PROJECT: Sleetlikham PERMANENT TP 11
HOLE NO. 11

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Sleetlikham SHEET: #1
LOCATION (Gridline & Station):
EXPLOSION LOG
PERM. LLTP 12 PERMANENT TP 12
TEST AT: 1.0 TYPE OF HOLE: 1.0 DEPTH: 1.0
SIZE AND TYPE OF BT: 1.0
TOTAL NO. OF SAMPLES: 1.0
DATE: 1.0
TIME: 1.0
CLASSIFICATION: 1.0
FORMATION DESCRIPTION & REMARKS: 1.0
Note: 1. Location approximate, no survey control.

Temporary designation LLTP 12

PROJECT: Sleetlikham PERMANENT TP 12
HOLE NO. 12

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Sleetlikham SHEET: #2
LOCATION (Gridline & Station):
EXPLOSION LOG
PERM. LLTP 13 PERMANENT TP 13
TEST AT: 1.0 TYPE OF HOLE: 1.0 DEPTH: 1.0
SIZE AND TYPE OF BT: 1.0
TOTAL NO. OF SAMPLES: 1.0
DATE: 1.0
TIME: 1.0
CLASSIFICATION: 1.0
FORMATION DESCRIPTION & REMARKS: 1.0
Note: 1. Location approximate, no survey control.

Temporary designation LLTP 13

PROJECT: Sleetlikham PERMANENT TP 13
HOLE NO. 13

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Sleetlikham SHEET: #2
LOCATION (Gridline & Station):
EXPLOSION LOG
PERM. LLTP 14 PERMANENT TP 14
TEST AT: 1.0 TYPE OF HOLE: 1.0 DEPTH: 1.0
SIZE AND TYPE OF BT: 1.0
TOTAL NO. OF SAMPLES: 1.0
DATE: 1.0
TIME: 1.0
CLASSIFICATION: 1.0
FORMATION DESCRIPTION & REMARKS: 1.0
Note: 1. Location approximate, no survey control.

Temporary designation LLTP 14

PROJECT: Sleetlikham PERMANENT TP 14
HOLE NO. 14

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Sleetlikham SHEET: #2
LOCATION (Gridline & Station):
EXPLOSION LOG
PERM. LLTP 15 PERMANENT TP 15
TEST AT: 1.0 TYPE OF HOLE: 1.0 DEPTH: 1.0
SIZE AND TYPE OF BT: 1.0
TOTAL NO. OF SAMPLES: 1.0
DATE: 1.0
TIME: 1.0
CLASSIFICATION: 1.0
FORMATION DESCRIPTION & REMARKS: 1.0
Note: 1. Location approximate, no survey control.

Temporary designation LLTP 15

PROJECT: Sleetlikham PERMANENT TP 15
HOLE NO. 15

[illegible][illegible][illegible]

U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: <i>W. B. TROTTER</i> DRAWN: <i>W. B. TROTTER</i> CHECKED: <i>D. HANNA</i> PREPARED: SUPERVISED: <i>W. B. TROTTER</i> SCALE: <i>AS SHOWN</i> SUBMITTED BY: <i>W. B. TROTTER</i> RECOMMENDED: <i>W. B. TROTTER</i>	SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT TEST PITS LOG RECORD NO. 11 APPROVED: <i>W. B. TROTTER</i> COLONEL, U. S. ARMY SCALE: _____ DATE: <i>20 Oct 61</i> SHEET _____ OF _____ FILE NUMBER _____

[illegible]

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA				SPECIMEN		FORM 1	
EXPEDITION USE				EXPEDITION (Number or Name)		PAGE 1	
NAME		TITLE		NAME OF OFFICER		GRADE	
TT-7a		TT-7b		O'Brien		Clear	
TYPE OF SPECIMEN		SOURCE		DATE		TIME	
1947		1948		1949		1950	
1951		1952		1953		1954	
1955		1956		1957		1958	
1959		1960		1961		1962	
1963		1964		1965		1966	
1967		1968		1969		1970	
1971		1972		1973		1974	
1975		1976		1977		1978	
1979		1980		1981		1982	
1983		1984		1985		1986	
1987		1988		1989		1990	
1991		1992		1993		1994	
1995		1996		1997		1998	
1999		2000		2001		2002	
2003		2004		2005		2006	
2007		2008		2009		2010	
2011		2012		2013		2014	
2015		2016		2017		2018	
2019		2020		2021		2022	
2023		2024		2025		2026	
2027		2028		2029		2030	
2031		2032		2033		2034	
2035		2036		2037		2038	
2039		2040		2041		2042	
2043		2044		2045		2046	
2047		2048		2049		2050	
2051		2052		2053		2054	
2055		2056		2057		2058	
2059		2060		2061		2062	
2063		2064		2065		2066	
2067		2068		2069		2070	
2071		2072		2073		2074	
2075		2076		2077		2078	
2079		2080		2081		2082	
2083		2084		2085		2086	
2087		2088		2089		2090	
2091		2092		2093		2094	
2095		2096		2097		2098	
2099		2100		2101		2102	
2103		2104		2105		2106	
2107		2108		2109		2110	
2111		2112		2113		2114	
2115		2116		2117		2118	
2119		2120		2121		2122	
2123		2124		2125		2126	
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2131		2132		2133		2134	
2135		2136		2137		2138	
2139		2140		2141		2142	
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2147		2148		2149		2150	
2151		2152		2153		2154	
2155		2156		2157		2158	
2159		2160		2161		2162	
2163		2164		2165		2166	
2167		2168		2169		2170	
2171		2172		2173		2174	
2175		2176		2177		2178	
2179		2180		2181		2182	
2183		2184		2185		2186	
2187		2188		2189		2190	
2191		2192		2193		2194	
2195		2196		2197		2198	
2199		2200		2201		2202	
2203		2204		2205			

[illegible][illegible]

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION ENGINEER DISTRICT ALASKA		PROJECT SHETTISHAM NORTH PACIFIC DIVISION ENGINEER DISTRICT ALASKA	
EXPLOSION LOG		EXPLOSION LOG	
DATE: 1947		DATE: 1947	
TIME: 10:00		TIME: 10:00	
LOCATION: Head of Long Lake		LOCATION: Head of Long Lake	
CLASSIFICATION: Slightly Silty Sand		CLASSIFICATION: Slightly Silty Sand	
FORMER DESCRIPTION & REMARKS: Fine grained sand and silt. Some organic material. Sample weight approx. 100 lbs.		FORMER DESCRIPTION & REMARKS: Fine grained sand and silt. Some organic material. Sample weight approx. 100 lbs.	
NOTE: Location - Head of Long Lake - test pit near line 100 and 110.		NOTE: Location - Head of Long Lake - test pit near line 100 and 110.	

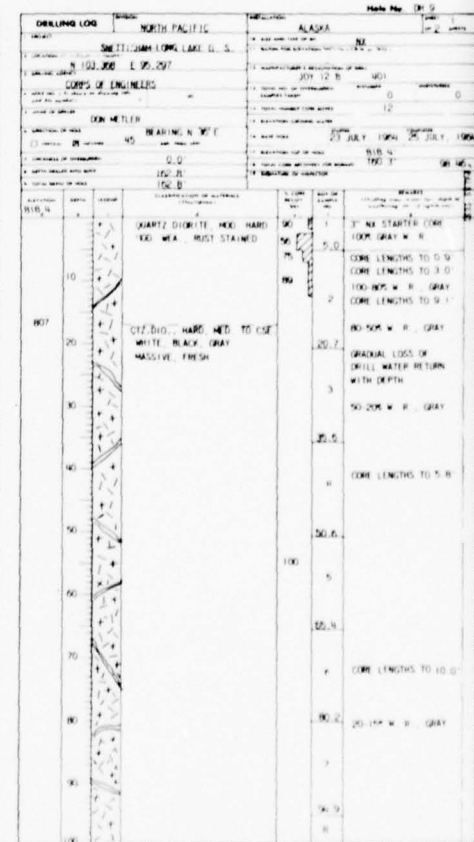
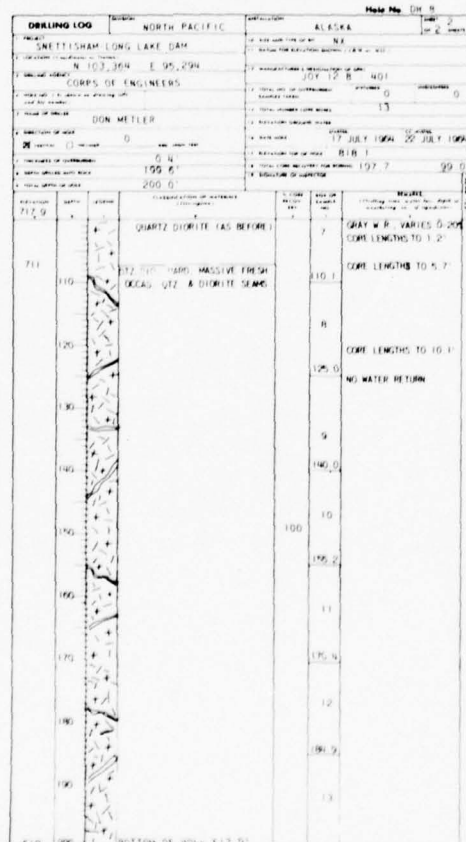
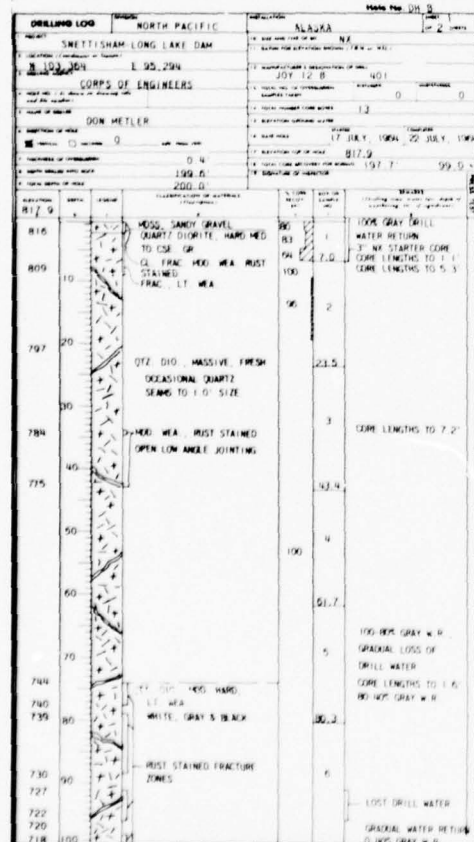
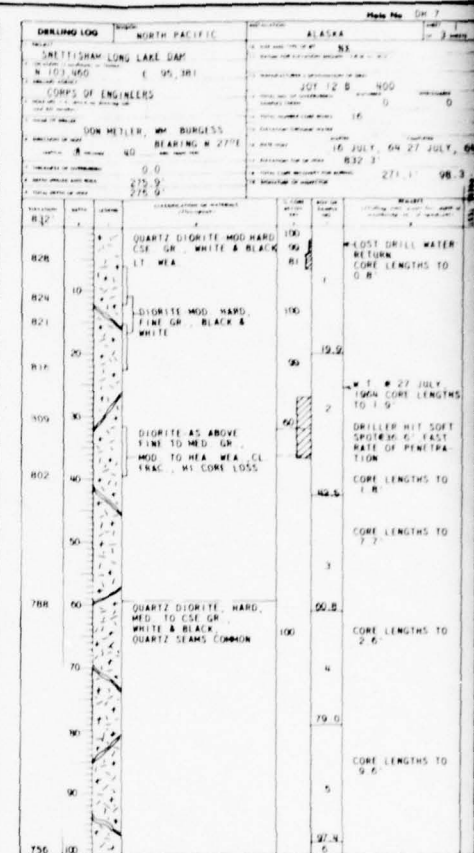
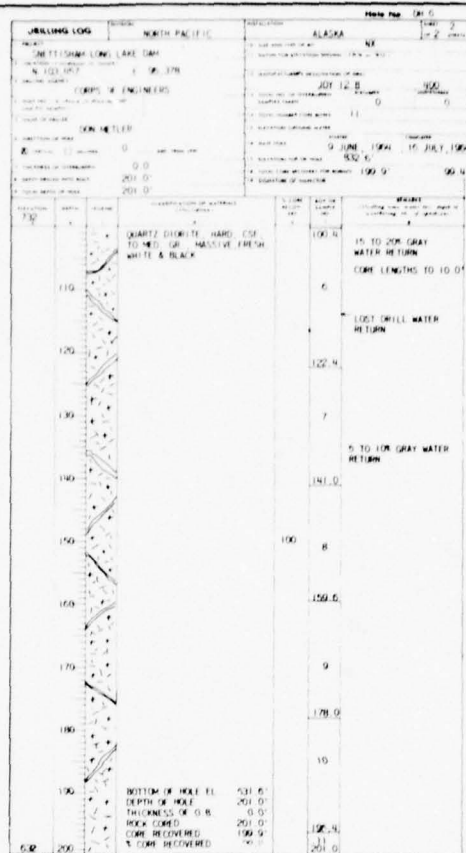
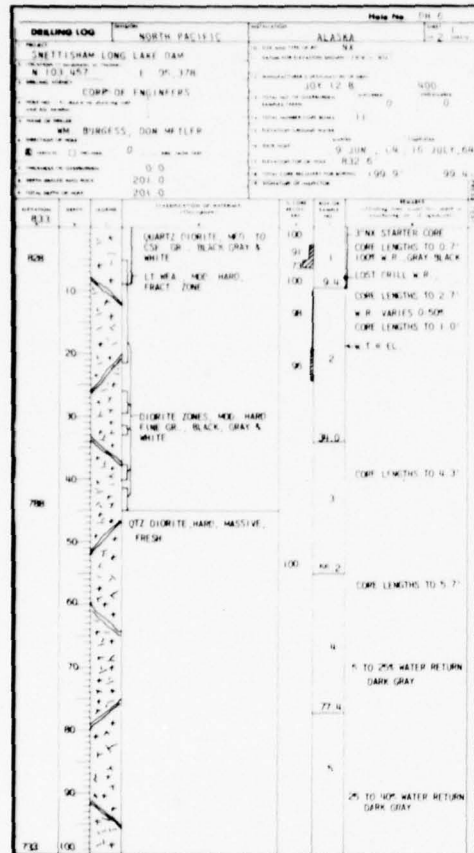
DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION ENGINEER DISTRICT ALASKA		PROJECT SHETTISHAM NORTH PACIFIC DIVISION ENGINEER DISTRICT ALASKA	
EXPLOSION LOG		EXPLOSION LOG	
DATE: 1947		DATE: 1947	
TIME: 10:00		TIME: 10:00	
LOCATION: Head of Long Lake		LOCATION: Head of Long Lake	
CLASSIFICATION: Slightly Silty Sand		CLASSIFICATION: Slightly Silty Sand	
FORMER DESCRIPTION & REMARKS: Fine grained sand and silt. Some organic material. Sample weight approx. 100 lbs.		FORMER DESCRIPTION & REMARKS: Fine grained sand and silt. Some organic material. Sample weight approx. 100 lbs.	
NOTE: Location - Head of Long Lake - test pit near line 100 and 110.		NOTE: Location - Head of Long Lake - test pit near line 100 and 110.	

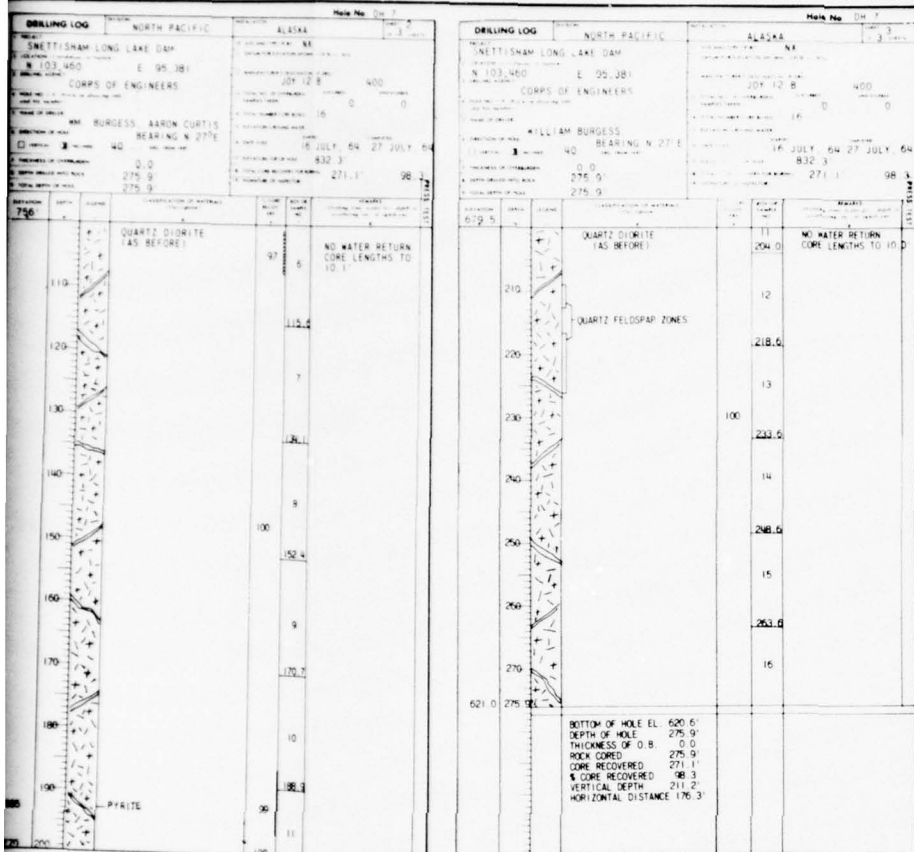
DESIGNED: Wm. B. Tenthren		DRAWN: Wm. B. Tenthren	
CHECKED: D. Hansen		PREPARED: D. Hansen	
SUPERVISOR: Wm. B. Tenthren		APPROVED: Wm. B. Tenthren	
ENGINEER: Wm. B. Tenthren		SCALE: DATE: 20 AUG 61	
RECOMMENDED: Wm. B. Tenthren		FILE NUMBER: SHEET OF	

PART 7

LOG RECORDS — CORE HOLES

CORPS OF ENGINEERS





5 "K" VALUES SHOWN ON LOGS ARE OBTAINED BY MATHEMATICAL COMBINATIONS OF FLOW AND HEADS FOR EACH TEST ZONE. THEY ARE EXPRESSED IN TERMS OF FT./MIN. X 10⁴.

DESIGNED	U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DRAWN	SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT	
CHECKED	LOG RECORD NO. 1	
PREPARED		
SUPERVISED	SECTION	APPROVED
SUBMITTED	SCALE	DATE
RECOMMENDED	FILE NUMBER	
SHEET OF		

DRILLING LOG		LOCATION		DATE		TIME	
PROJECT		NORTH PACIFIC		ALASKA		Hole No. 2	
1. LOCATION		SNETTISHAM LONG LK O. S.		2. DATE AND TIME OF DAY		23 JULY 1964	
3. LOCATION		N 63.300 E 95.297		4. DATE AND TIME OF DAY		25 JULY 1964	
5. COMPANY		CORPS OF ENGINEERS		6. DATE AND TIME OF DAY		25 JULY 1964	
7. NAME OF DRILLER		KIMMELT		8. DATE AND TIME OF DAY		25 JULY 1964	
9. NAME OF DRILLER		KIMMELT		10. DATE AND TIME OF DAY		25 JULY 1964	
11. NAME OF DRILLER		KIMMELT		12. DATE AND TIME OF DAY		25 JULY 1964	
13. NAME OF DRILLER		KIMMELT		14. DATE AND TIME OF DAY		25 JULY 1964	
15. NAME OF DRILLER		KIMMELT		16. DATE AND TIME OF DAY		25 JULY 1964	
17. NAME OF DRILLER		KIMMELT		18. DATE AND TIME OF DAY		25 JULY 1964	
19. NAME OF DRILLER		KIMMELT		20. DATE AND TIME OF DAY		25 JULY 1964	
21. NAME OF DRILLER		KIMMELT		22. DATE AND TIME OF DAY		25 JULY 1964	
23. NAME OF DRILLER		KIMMELT		24. DATE AND TIME OF DAY		25 JULY 1964	
25. NAME OF DRILLER		KIMMELT		26. DATE AND TIME OF DAY		25 JULY 1964	
27. NAME OF DRILLER		KIMMELT		28. DATE AND TIME OF DAY		25 JULY 1964	
29. NAME OF DRILLER		KIMMELT		30. DATE AND TIME OF DAY		25 JULY 1964	
31. NAME OF DRILLER		KIMMELT		32. DATE AND TIME OF DAY		25 JULY 1964	
33. NAME OF DRILLER		KIMMELT		34. DATE AND TIME OF DAY		25 JULY 1964	
35. NAME OF DRILLER		KIMMELT		36. DATE AND TIME OF DAY		25 JULY 1964	
37. NAME OF DRILLER		KIMMELT		38. DATE AND TIME OF DAY		25 JULY 1964	
39. NAME OF DRILLER		KIMMELT		40. DATE AND TIME OF DAY		25 JULY 1964	
41. NAME OF DRILLER		KIMMELT		42. DATE AND TIME OF DAY		25 JULY 1964	
43. NAME OF DRILLER		KIMMELT		44. DATE AND TIME OF DAY		25 JULY 1964	
45. NAME OF DRILLER		KIMMELT		46. DATE AND TIME OF DAY		25 JULY 1964	
47. NAME OF DRILLER		KIMMELT		48. DATE AND TIME OF DAY		25 JULY 1964	
49. NAME OF DRILLER		KIMMELT		50. DATE AND TIME OF DAY		25 JULY 1964	
51. NAME OF DRILLER		KIMMELT		52. DATE AND TIME OF DAY		25 JULY 1964	
53. NAME OF DRILLER		KIMMELT		54. DATE AND TIME OF DAY		25 JULY 1964	
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57. NAME OF DRILLER		KIMMELT		58. DATE AND TIME OF DAY		25 JULY 1964	
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61. NAME OF DRILLER		KIMMELT		62. DATE AND TIME OF DAY		25 JULY 1964	
63. NAME OF DRILLER		KIMMELT		64. DATE AND TIME OF DAY		25 JULY 1964	
65. NAME OF DRILLER		KIMMELT		66. DATE AND TIME OF DAY		25 JULY 1964	
67. NAME OF DRILLER		KIMMELT		68. DATE AND TIME OF DAY		25 JULY 1964	
69. NAME OF DRILLER		KIMMELT		70. DATE AND TIME OF DAY		25 JULY 1964	
71. NAME OF DRILLER		KIMMELT		72. DATE AND TIME OF DAY		25 JULY 1964	
73. NAME OF DRILLER		KIMMELT		74. DATE AND TIME OF DAY		25 JULY 1964	
75. NAME OF DRILLER		KIMMELT		76. DATE AND TIME OF DAY		25 JULY 1964	
77. NAME OF DRILLER		KIMMELT		78. DATE AND TIME OF DAY		25 JULY 1964	
79. NAME OF DRILLER		KIMMELT		80. DATE AND TIME OF DAY		25 JULY 1964	
81. NAME OF DRILLER		KIMMELT		82. DATE AND TIME OF DAY		25 JULY 1964	
83. NAME OF DRILLER		KIMMELT		84. DATE AND TIME OF DAY		25 JULY 1964	
85. NAME OF DRILLER		KIMMELT		86. DATE AND TIME OF DAY		25 JULY 1964	
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89. NAME OF DRILLER		KIMMELT		90. DATE AND TIME OF DAY		25 JULY 1964	
91. NAME OF DRILLER		KIMMELT		92. DATE AND TIME OF DAY		25 JULY 1964	
93. NAME OF DRILLER		KIMMELT		94. DATE AND TIME OF DAY		25 JULY 1964	
95. NAME OF DRILLER		KIMMELT		96. DATE AND TIME OF DAY		25 JULY 1964	
97. NAME OF DRILLER		KIMMELT		98. DATE AND TIME OF DAY		25 JULY 1964	
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101. NAME OF DRILLER		KIMMELT		102. DATE AND TIME OF DAY		25 JULY 1964	
103. NAME OF DRILLER		KIMMELT		104. DATE AND TIME OF DAY		25 JULY 1964	
105. NAME OF DRILLER		KIMMELT		106. DATE AND TIME OF DAY		25 JULY 1964	
107. NAME OF DRILLER		KIMMELT		108. DATE AND TIME OF DAY		25 JULY 1964	
109. NAME OF DRILLER							

[illegible]

DRILLING LOG NORTH PACIFIC

DATE: 27 JULY, 1954
 LOCATION: LONG LAKE, D. S.
 WELL NO.: 102-162
 DEPTH: 102.377
 CORPUS OF ENGINEERS

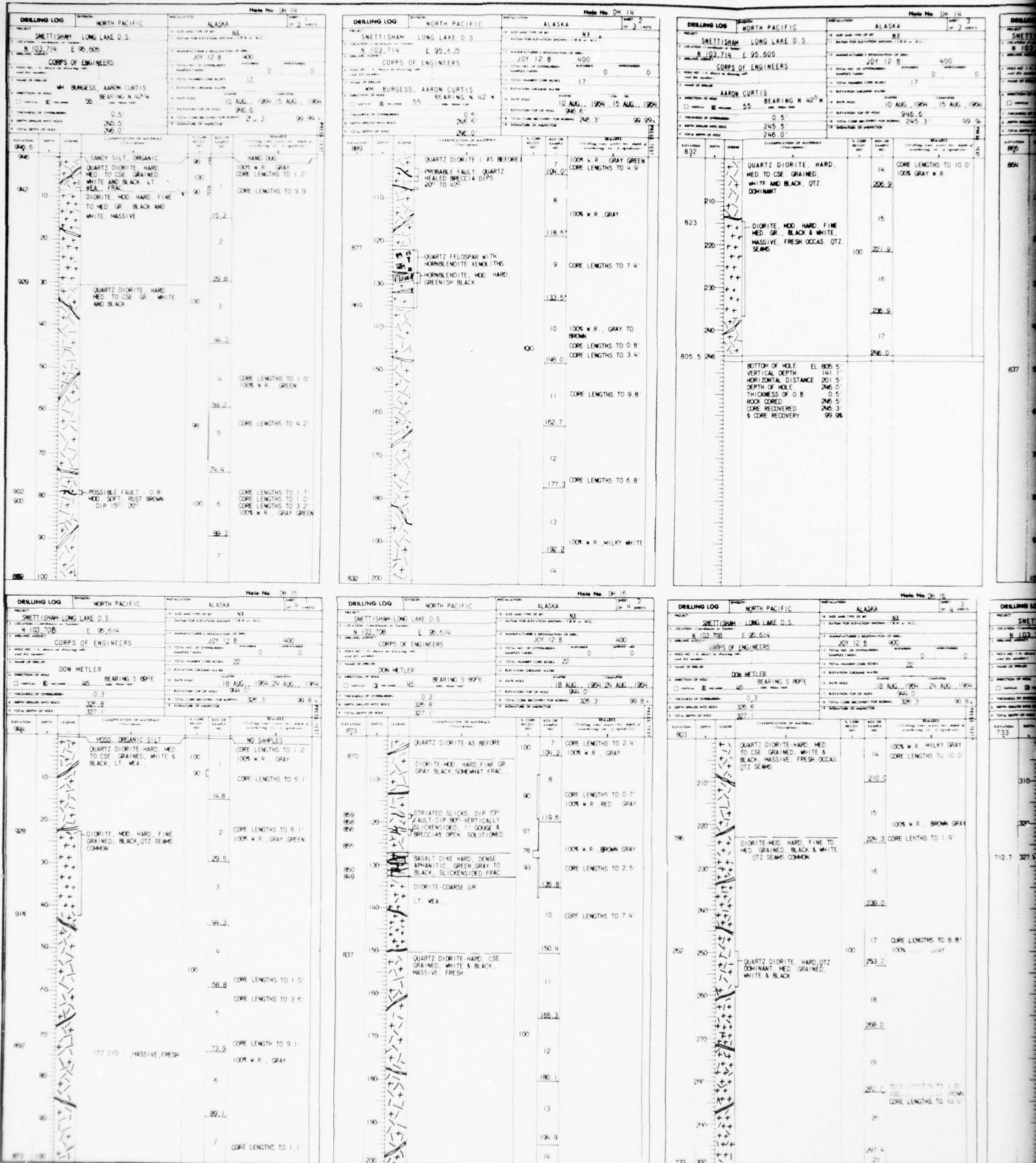
DRILLER: ALAN CURTIS
 SUPERVISOR: 60 BEARING W. 13" W
 DIRECTION OF WIND: 0.8
 DIRECTION OF WAVE: 110.0
 DIRECTION OF CURRENT: 110.0
 DIRECTION OF TIDE: 110.0

REMARKS: 1. CORE LENGTHS TO 5.0
 2. CORE LENGTHS TO 1.0
 3. CORE LENGTHS TO 1.0
 4. CORE LENGTHS TO 1.0
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 99. CORE LENGTHS TO 1.0
 100. CORE LENGTHS TO 1.0

DEBILUNG LOG		NORTH PACIFIC		ALASKA		Map No. 04 12	
PROJECT		SHEPHERD LANE LAKES P.S.		NAME		DATE	
1. LOCATION (Longitude & Latitude)		N 100.000 E 95.000		2. DATE FOR DEBILUNG RECORD		7.0 N. 4.0 E.	
3. DEBILUNG CODE		CORPS OF ENGINEERS		4. NAME OF DEBILUNG		5. NAME OF DEBILUNG	
6. NAME OF DEBILUNG		DON METLER		7. NAME OF DEBILUNG		8. NAME OF DEBILUNG	
9. NAME OF DEBILUNG		DON METLER		10. NAME OF DEBILUNG		11. NAME OF DEBILUNG	
12. NAME OF DEBILUNG		DON METLER		13. NAME OF DEBILUNG		14. NAME OF DEBILUNG	
15. NAME OF DEBILUNG		DON METLER		16. NAME OF DEBILUNG		17. NAME OF DEBILUNG	
18. NAME OF DEBILUNG		DON METLER		19. NAME OF DEBILUNG		20. NAME OF DEBILUNG	
21. NAME OF DEBILUNG		DON METLER		22. NAME OF DEBILUNG		23. NAME OF DEBILUNG	
24. NAME OF DEBILUNG		DON METLER		25. NAME OF DEBILUNG		26. NAME OF DEBILUNG	
27. NAME OF DEBILUNG		DON METLER		28. NAME OF DEBILUNG		29. NAME OF DEBILUNG	
30. NAME OF DEBILUNG		DON METLER		31. NAME OF DEBILUNG		32. NAME OF DEBILUNG	
33. NAME OF DEBILUNG		DON METLER		34. NAME OF DEBILUNG		35. NAME OF DEBILUNG	
36. NAME OF DEBILUNG		DON METLER		37. NAME OF DEBILUNG		38. NAME OF DEBILUNG	
39. NAME OF DEBILUNG		DON METLER		40. NAME OF DEBILUNG		41. NAME OF DEBILUNG	
42. NAME OF DEBILUNG		DON METLER		43. NAME OF DEBILUNG		44. NAME OF DEBILUNG	
45. NAME OF DEBILUNG		DON METLER		46. NAME OF DEBILUNG		47. NAME OF DEBILUNG	
48. NAME OF DEBILUNG		DON METLER		49. NAME OF DEBILUNG		50. NAME OF DEBILUNG	
51. NAME OF DEBILUNG		DON METLER		52. NAME OF DEBILUNG		53. NAME OF DEBILUNG	
54. NAME OF DEBILUNG		DON METLER		55. NAME OF DEBILUNG		56. NAME OF DEBILUNG	
57. NAME OF DEBILUNG		DON METLER		58. NAME OF DEBILUNG		59. NAME OF DEBILUNG	
60. NAME OF DEBILUNG		DON METLER		61. NAME OF DEBILUNG		62. NAME OF DEBILUNG	
63. NAME OF DEBILUNG		DON METLER		64. NAME OF DEBILUNG		65. NAME OF DEBILUNG	
66. NAME OF DEBILUNG		DON METLER		67. NAME OF DEBILUNG		68. NAME OF DEBILUNG	
69. NAME OF DEBILUNG		DON METLER		70. NAME OF DEBILUNG		71. NAME OF DEBILUNG	
72. NAME OF DEBILUNG		DON METLER		73. NAME OF DEBILUNG		74. NAME OF DEBILUNG	
75. NAME OF DEBILUNG		DON METLER		76. NAME OF DEBILUNG		77. NAME OF DEBILUNG	
78. NAME OF DEBILUNG		DON METLER		79. NAME OF DEBILUNG		80. NAME OF DEBILUNG	
81. NAME OF DEBILUNG		DON METLER		82. NAME OF DEBILUNG		83. NAME OF DEBILUNG	
84. NAME OF DEBILUNG		DON METLER		85. NAME OF DEBILUNG		86. NAME OF DEBILUNG	
87. NAME OF DEBILUNG		DON METLER		88. NAME OF DEBILUNG		89. NAME OF DEBILUNG	
90. NAME OF DEBILUNG		DON METLER		91. NAME OF DEBILUNG		92. NAME OF DEBILUNG	
93. NAME OF DEBILUNG		DON METLER		94. NAME OF DEBILUNG		95. NAME OF DEBILUNG	
96. NAME OF DEBILUNG		DON METLER		97. NAME OF DEBILUNG		98. NAME OF DEBILUNG	
99. NAME OF DEBILUNG		DON METLER		100. NAME OF DEBILUNG		101. NAME OF DEBILUNG	
102. NAME OF DEBILUNG		DON METLER		103. NAME OF DEBILUNG		104. NAME OF DEBILUNG	
105. NAME OF DEBILUNG		DON METLER		106. NAME OF DEBILUNG		107. NAME OF DEBILUNG	
108. NAME OF DEBILUNG		DON METLER		109. NAME OF DEBILUNG		110. NAME OF DEBILUNG	
111. NAME OF DEBILUNG		DON METLER		112. NAME OF DEBILUNG		113. NAME OF DEBILUNG	
114. NAME OF DEBILUNG		DON METLER		115. NAME OF DEBILUNG		116. NAME OF DEBILUNG	
117. NAME OF DEBILUNG		DON METLER		118. NAME OF DEBILUNG		119. NAME OF DEBILUNG	
120. NAME OF DEBILUNG		DON METLER		121. NAME OF DEBILUNG		122. NAME OF DEBILUNG	
123. NAME OF DEBILUNG		DON METLER		124. NAME OF DEBILUNG		125. NAME OF DEBILUNG	
126. NAME OF DEBILUNG		DON METLER		127. NAME OF DEBILUNG		128. NAME OF DEBILUNG	
129. NAME OF DEBILUNG		DON METLER		130. NAME OF DEBILUNG		131. NAME OF DEBILUNG	
132. NAME OF DEBILUNG		DON METLER		133. NAME OF DEBILUNG		134. NAME OF DEBILUNG	
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138. NAME OF DEBILUNG		DON METLER		139. NAME OF DEBILUNG		140. NAME OF DEBILUNG	
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144. NAME OF DEBILUNG		DON METLER		145. NAME OF DEBILUNG		146. NAME OF DEBILUNG	
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156. NAME OF DEBILUNG		DON METLER		157. NAME OF DEBILUNG		158. NAME OF DEBILUNG	

[illegible][illegible]

CORPS OF ENGINEERS



BLIND LOG		NORTH PACIFIC		ALASKA		Plate No. 15	
SNETTISHAM LONG LAKE D.S.		E 95,250		N 103,000		CORPS OF ENGINEERS	
DON METLER		BEARING S 10° W		12 AUG. 1964		20 AUG. 1964	
55		2.4		190.9		90.4	
192.0		194.0					
SANDY SILT, ORGANIC, BROWN		QUARTZ DIOXIDE, HARD		MEDIUM TO COARSE GRAINED		BLACK & WHITE, LT. GRAY	
10		17.3		31.8		LOST DRILL WATER RETURN	
20		47.6		CORE LENGTHS TO 0.4		CORE LENGTHS TO 7.1	
30		52.7		CORE LENGTHS TO 2.9			
40		77.6		CORE LENGTHS TO 2.9			
50		91.2		CORE LENGTHS TO 2.9			
60							
70							
80							
90							
100							
110							
120							
130							
140							
150							
160							
170							
180							
190							
200							
TRUE VERTICAL DEPTH 111.5'		TRUE HORIZONTAL DEPTH 169.2'					

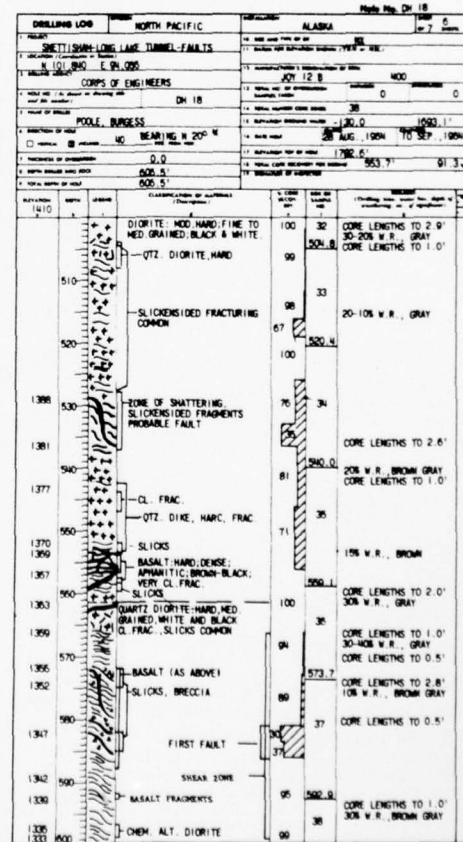
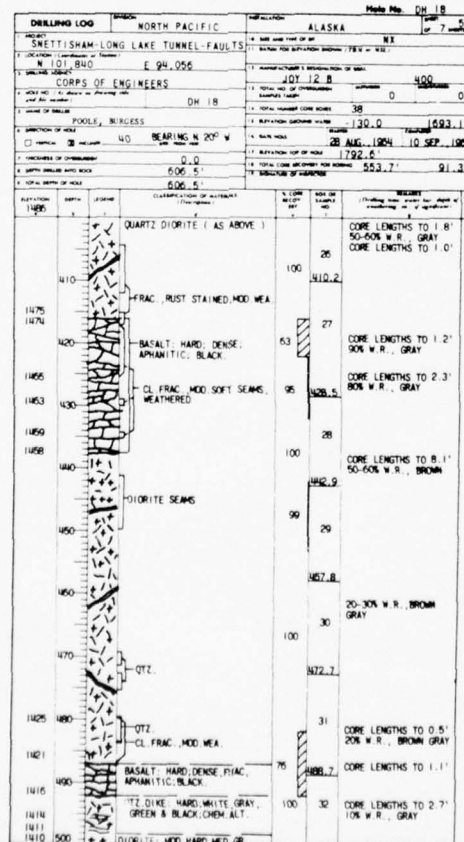
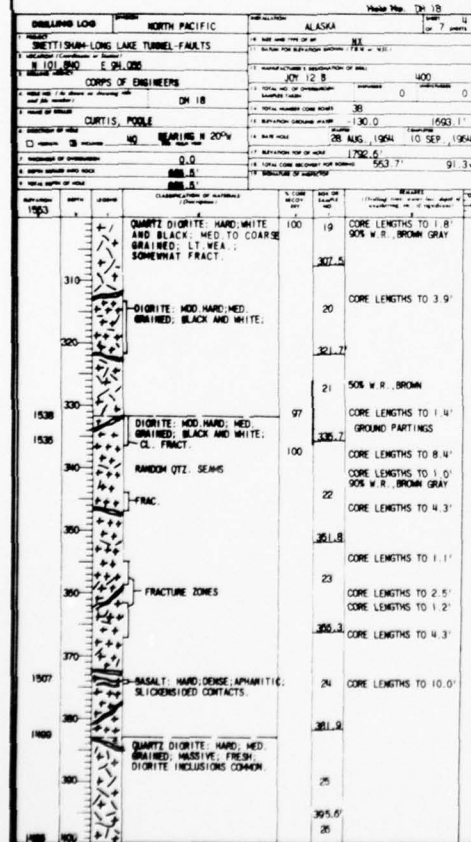
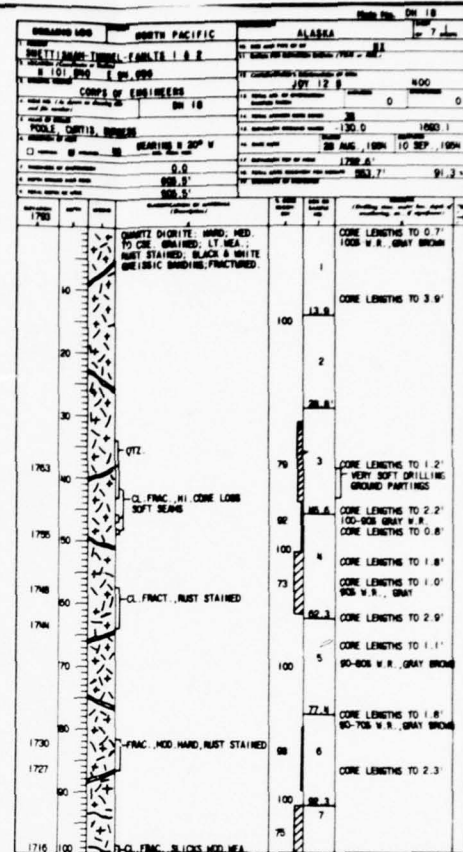
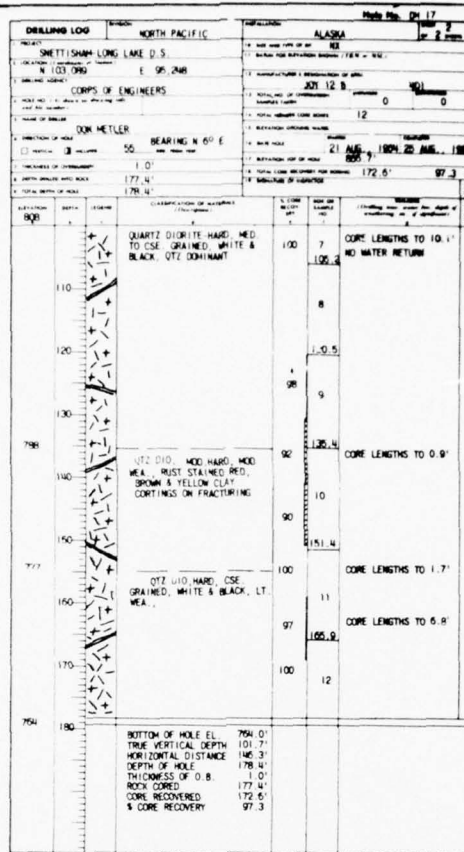
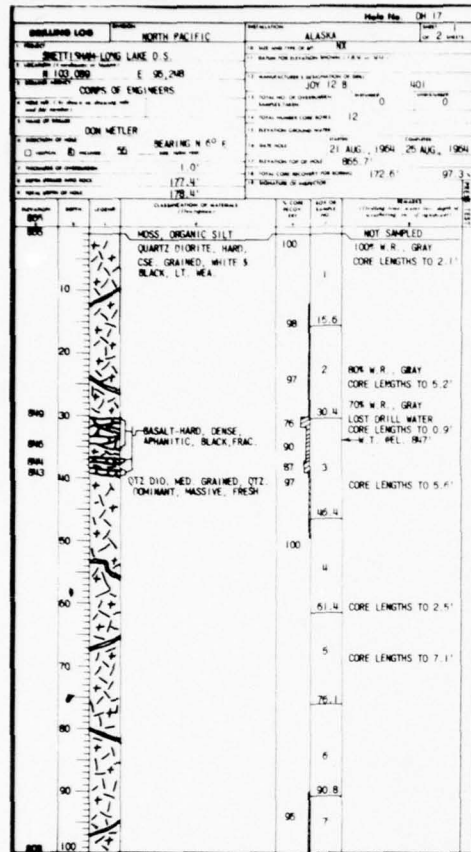
DRILLING LOG		NORTH PACIFIC		ALASKA		Plate No. 15	
SNETTISHAM LONG LAKE D.S.		E 95,250		N 103,000		CORPS OF ENGINEERS	
DON METLER		BEARING S 10° W		12 AUG. 1964		20 AUG. 1964	
55		2.4		190.9		90.4	
192.0		194.0					
SANDY SILT, ORGANIC, BROWN		QUARTZ DIOXIDE, HARD		MEDIUM TO COARSE GRAINED		BLACK & WHITE, LT. GRAY	
10		17.3		31.8		LOST DRILL WATER RETURN	
20		47.6		CORE LENGTHS TO 0.4		CORE LENGTHS TO 7.1	
30		52.7		CORE LENGTHS TO 2.9			
40		77.6		CORE LENGTHS TO 2.9			
50		91.2		CORE LENGTHS TO 2.9			
60							
70							
80							
90							
100							
110							
120							
130							
140							
150							
160							
170							
180							
190							
200							
TRUE VERTICAL DEPTH 111.5'		TRUE HORIZONTAL DEPTH 169.2'					

BLIND LOG		NORTH PACIFIC		ALASKA		Plate No. 15	
SNETTISHAM LONG LAKE D.S.		E 95,614		N 103,000		CORPS OF ENGINEERS	
DON METLER		BEARING S 90° E		18 AUG. 1964		24 AUG. 1964	
55		0.3		225.1		99.8	
192.0		194.0					
SANDY SILT, ORGANIC, BROWN		QUARTZ DIOXIDE, HARD		MEDIUM TO COARSE GRAINED		BLACK & WHITE, LT. GRAY	
10		17.3		31.8		LOST DRILL WATER RETURN	
20		47.6		CORE LENGTHS TO 0.4		CORE LENGTHS TO 7.1	
30		52.7		CORE LENGTHS TO 2.9			
40		77.6		CORE LENGTHS TO 2.9			
50		91.2		CORE LENGTHS TO 2.9			
60							
70							
80							
90							
100							
110							
120							
130							
140							
150							
160							
170							
180							
190							
200							
TRUE VERTICAL DEPTH 111.5'		TRUE HORIZONTAL DEPTH 169.2'					

BLIND LOG		NORTH PACIFIC		ALASKA		Plate No. 15	
SNETTISHAM LONG LAKE D.S.		E 95,614		N 103,000		CORPS OF ENGINEERS	
DON METLER		BEARING S 90° E		18 AUG. 1964		24 AUG. 1964	
55		0.3		225.1		99.8	
192.0		194.0					
SANDY SILT, ORGANIC, BROWN		QUARTZ DIOXIDE, HARD		MEDIUM TO COARSE GRAINED		BLACK & WHITE, LT. GRAY	
10		17.3		31.8		LOST DRILL WATER RETURN	
20		47.6		CORE LENGTHS TO 0.4		CORE LENGTHS TO 7.1	
30		52.7		CORE LENGTHS TO 2.9			
40		77.6		CORE LENGTHS TO 2.9			
50		91.2		CORE LENGTHS TO 2.9			
60							
70							
80							
90							
100							
110							
120							
130							
140							
150							
160							
170							
180							
190							
200							
TRUE VERTICAL DEPTH 111.5'		TRUE HORIZONTAL DEPTH 169.2'					

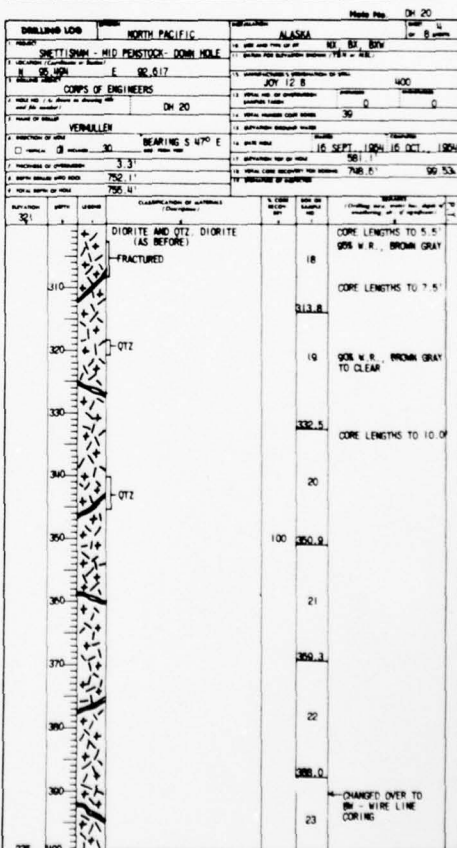
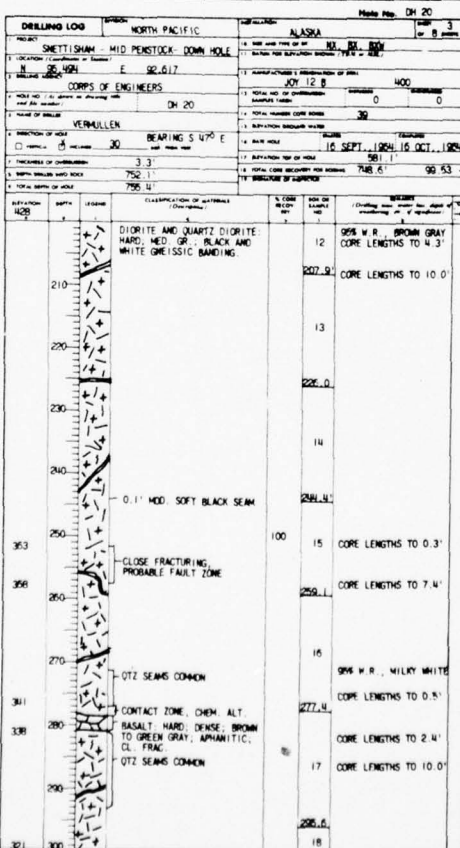
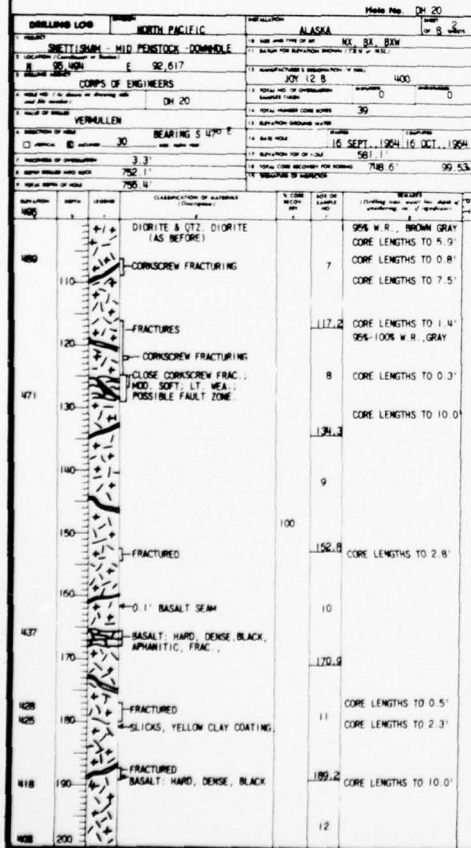
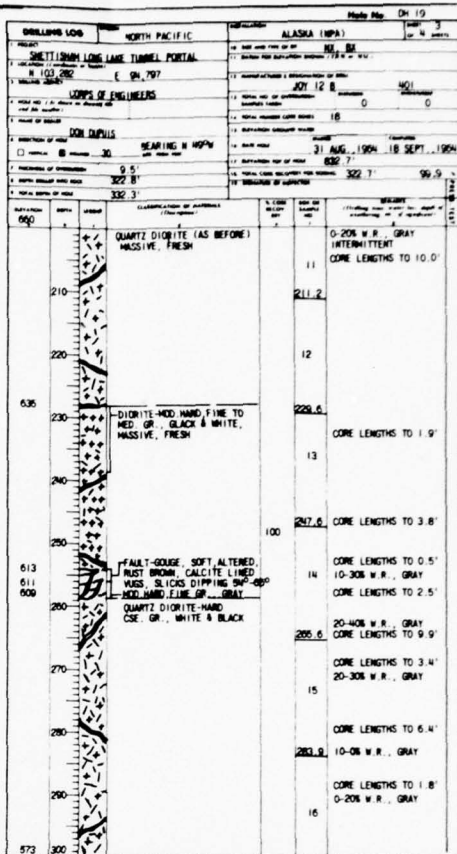
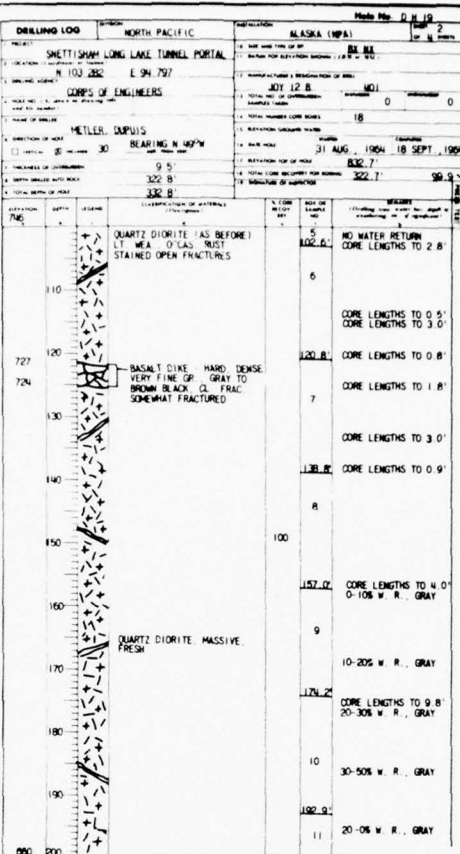
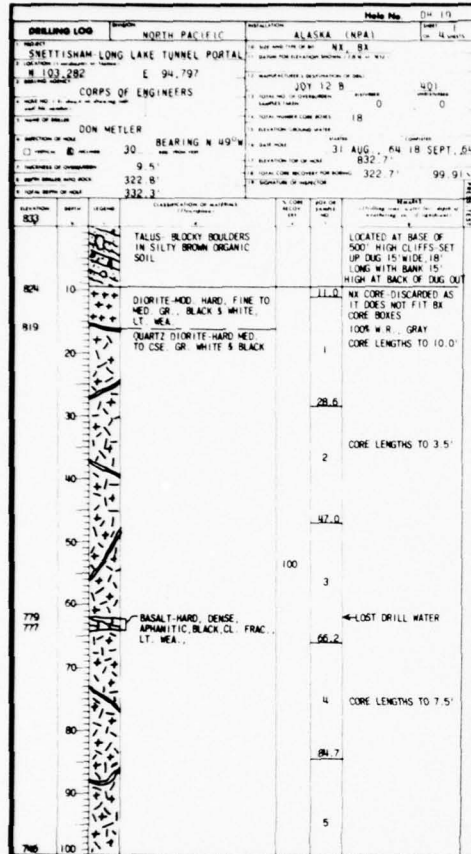
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P. J. B. B. B.		P. J. B. B. B.		P. J. B. B. B.		P. J. B. B. B.	
SUPERVISED		APPROVED		SCALE		DATE	
P. J. B. B. B.		P. J. B. B. B.		P. J. B. B. B.		P. J. B. B. B.	
SUBMITTED		FILE NUMBER		SHEET		OF	
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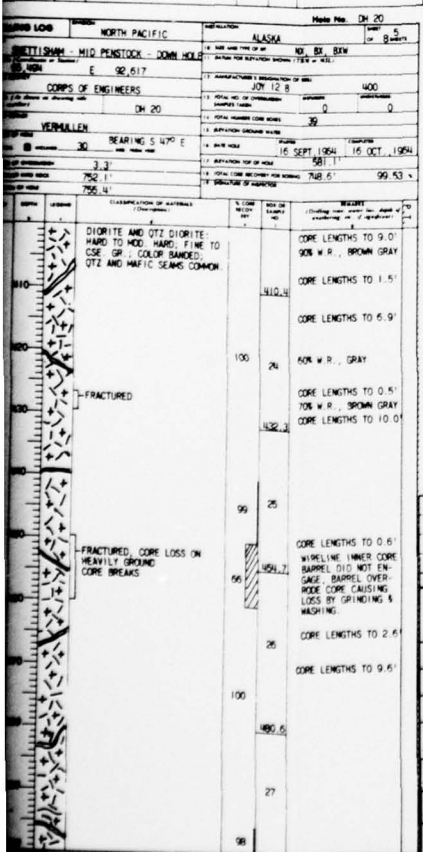
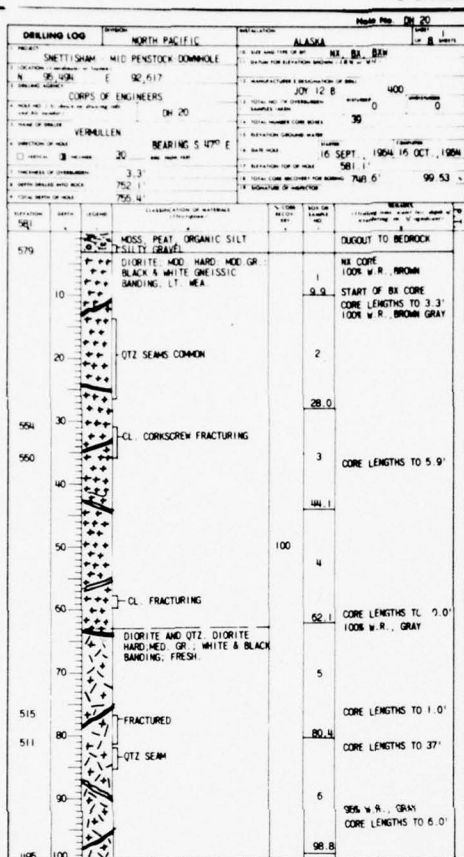
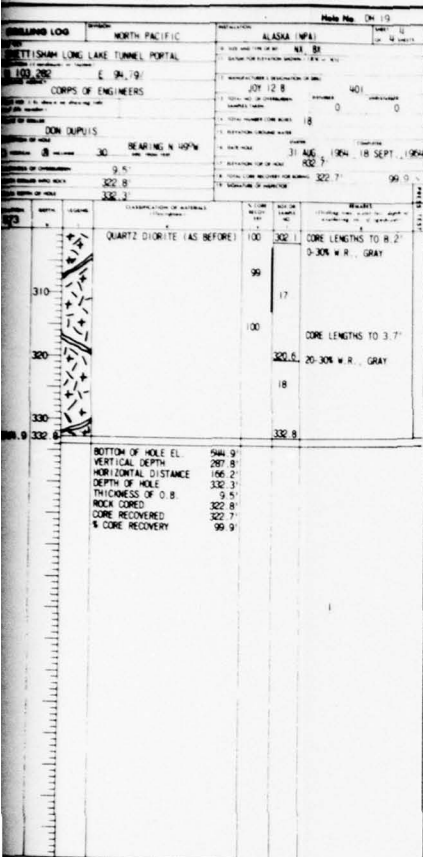
U. S. ARMY ENGINEER DISTRICT, ALASKA	
CORPS OF ENGINEERS	
ANCHORAGE, ALASKA	
LOG RECORD NO. 3	



[illegible]

CORPS OF ENGINEERS





NOTE: SEE LOG RECORDS NUMBER 1 FOR NOTES AND LEGEND

U. S. ARMY ENGINEER DISTRICT, ALASKA
CORPS OF ENGINEERS
ANCHORAGE, ALASKA

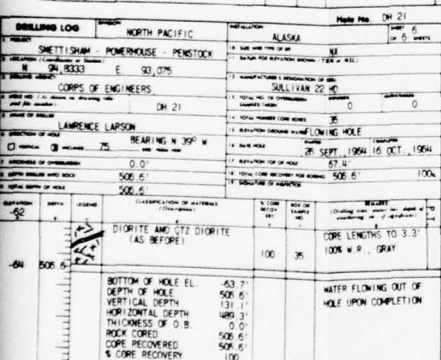
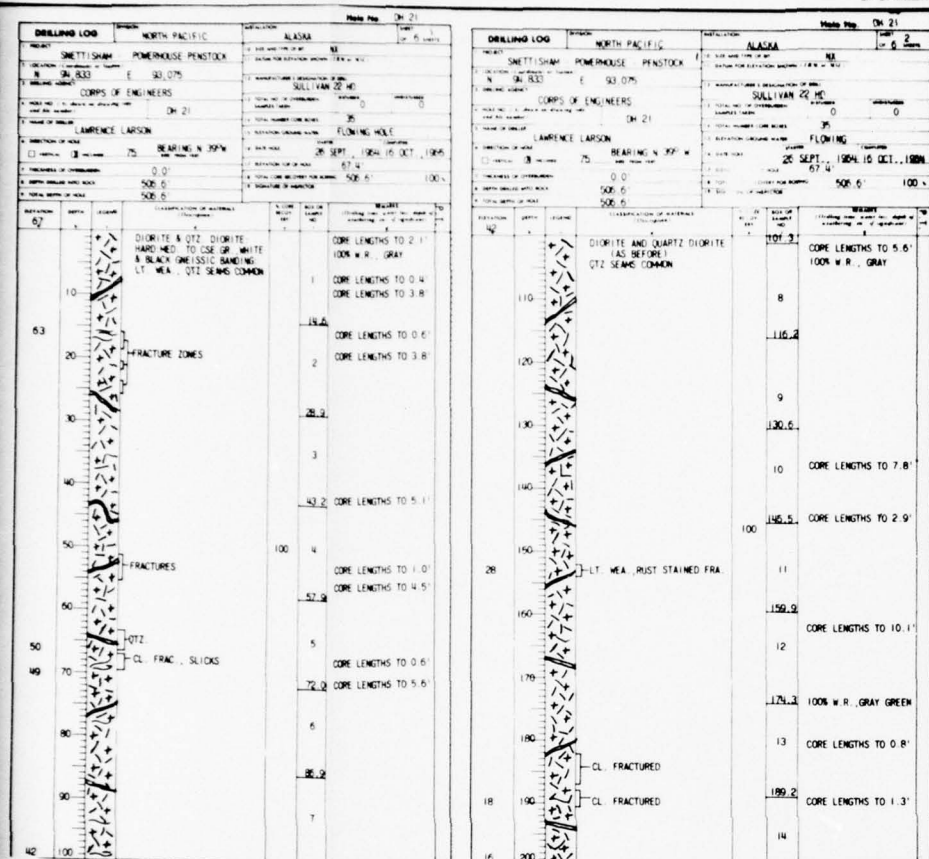
DESIGNED: *W. D. T. T. T.*
DRAWN: *W. D. T. T. T.*
CHECKED: *W. D. T. T. T.*
PREPARED: *W. D. T. T. T.*

SNETTISHAM PROJECT, ALASKA
FIRST STAGE DEVELOPMENT

LOG RECORD NO. 5

SUPERVISOR: *W. D. T. T. T.*
SUBMITTER: *W. D. T. T. T.*
RECOMMENDED: *W. D. T. T. T.*

APPROVED: *W. D. T. T. T.*
SCALE: *1:1*
DATE: *10/12/94*
FILE NUMBER: *10/12/94*



NOTE:
SEE LOG RECORDS NUMBER 1 FOR NOTES AND LEGEND

U. S. ARMY ENGINEER DISTRICT, ALASKA
CORPS OF ENGINEERS
ANCHORAGE, ALASKA

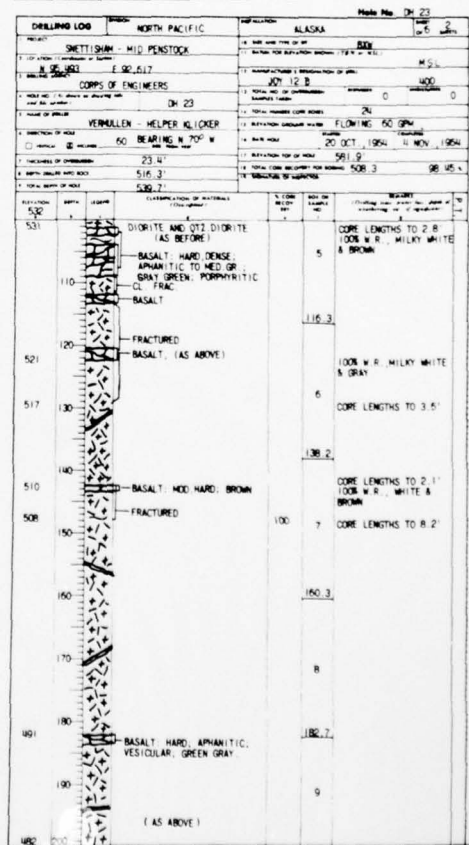
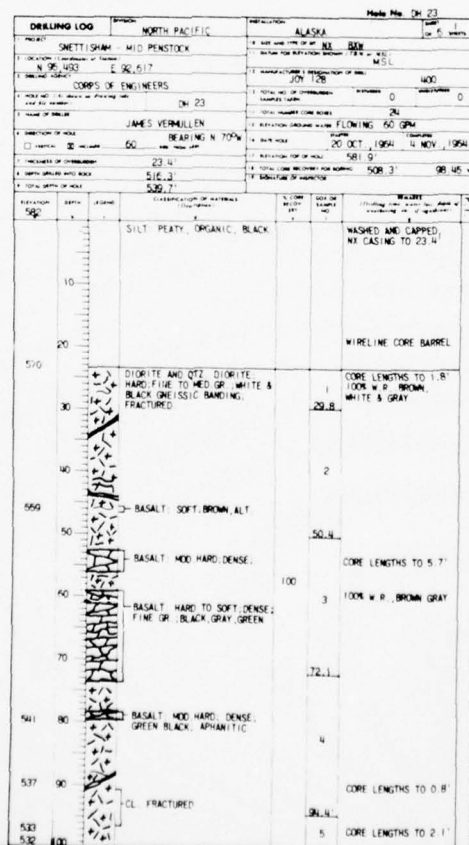
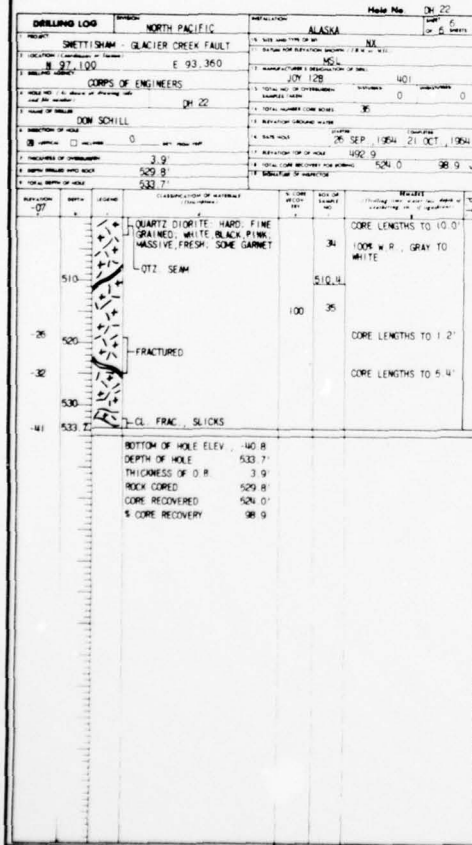
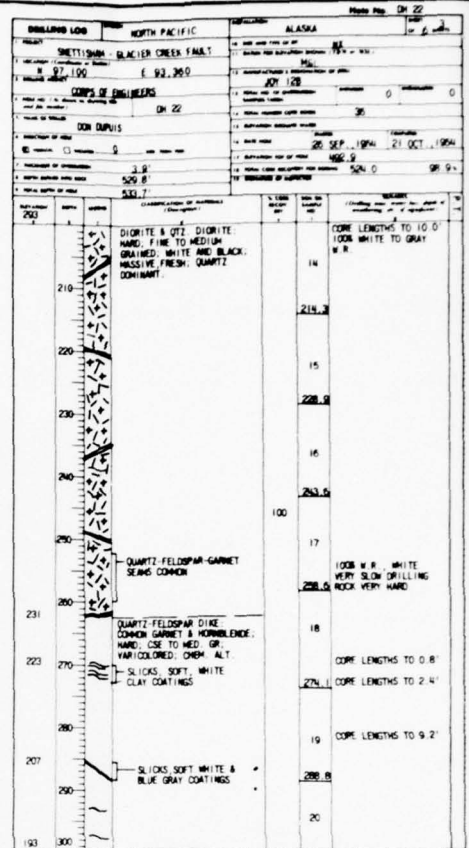
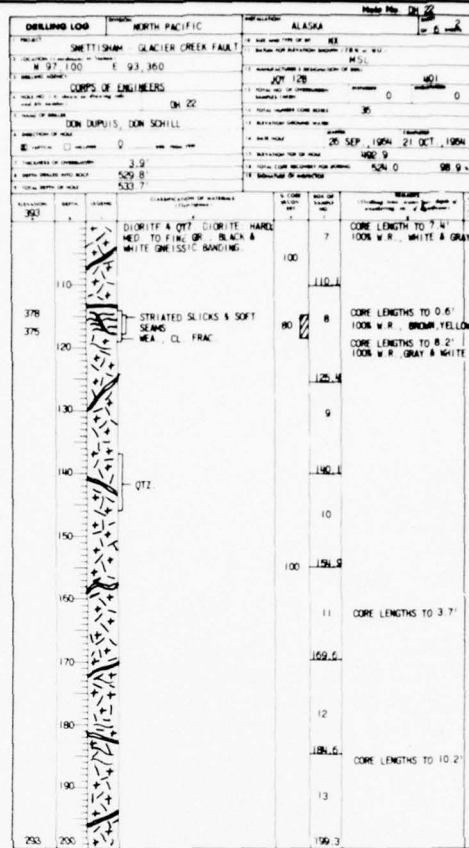
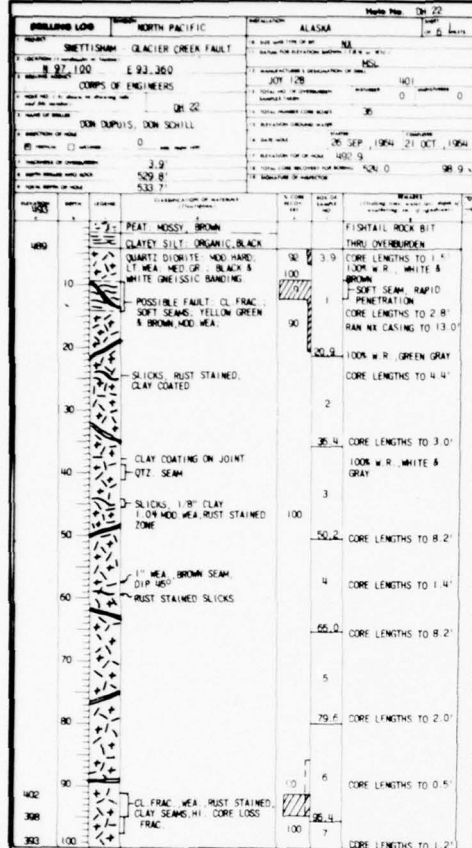
DESIGNED: *W. B. TROTTER*
DRAWN: *W. B. TROTTER*
CHECKED: *W. B. TROTTER*
PREPARED: *W. B. TROTTER*

LOG RECORD NO. 6

SUPERVISOR: *W. B. TROTTER*
SUBMITTED: *W. B. TROTTER*
RECOMMENDED: *W. B. TROTTER*

APPROVED: *W. B. TROTTER*
SCALE: *1" = 10'*
DATE: *20 Oct 1954*
SHEET: *OF*

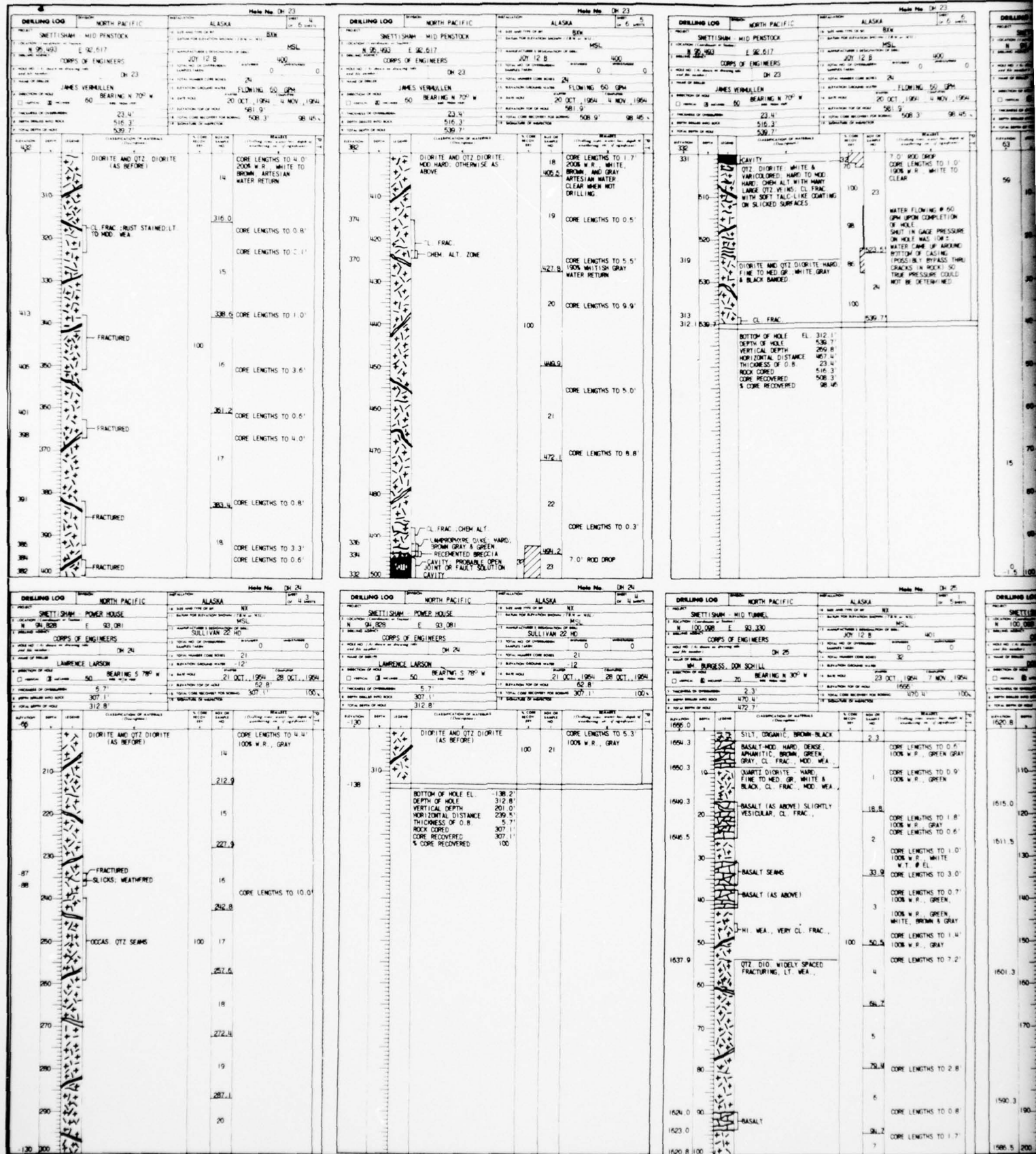
CORPS OF ENGINEERS

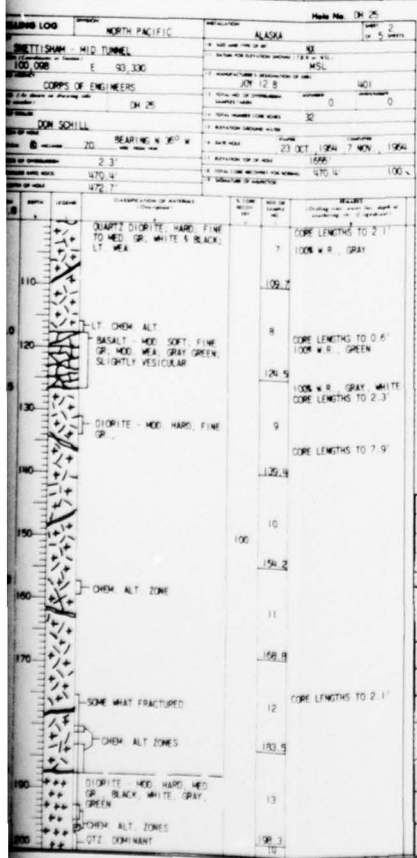
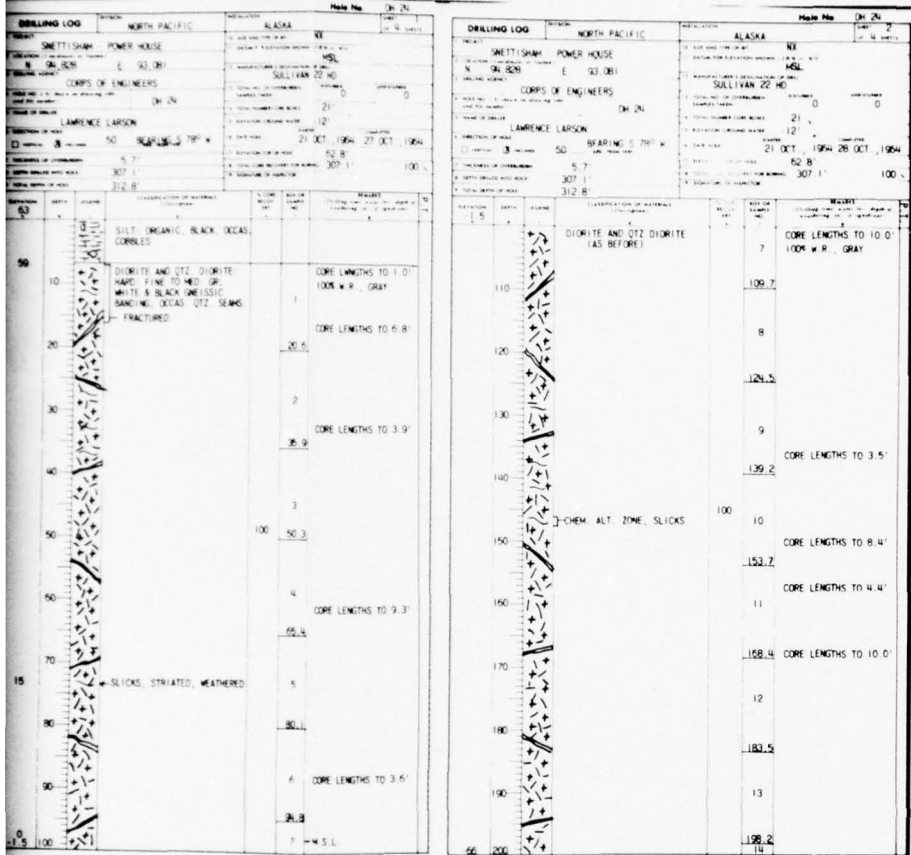


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NOTE SEE LOG RECORDS NUMBER 1 FOR NOTES AND LEGEND							
U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA							
DESIGNED DRAWN CHECKED IN CHARGE SPECIALIST	<div style="text-align: center; font-size: 1.2em; font-weight: bold;"> SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT </div> <div style="text-align: center; font-size: 1.5em; font-weight: bold; margin-top: 20px;"> LOG RECORD NO. 7 </div>						
REVISION BY DATE FOUNDATION COMMAND	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td colspan="2" style="padding: 5px;"> APPROVED <i>Chas. J. Dwyer</i> COLONEL, DISTRICT ENGINEER </td> </tr> <tr> <td style="width: 50%; padding: 5px;"> SCALE </td> <td style="width: 50%; padding: 5px;"> DATE <i>20 Oct 60</i> </td> </tr> <tr> <td colspan="2" style="padding: 5px;"> SHEET <i>OF</i> </td> </tr> </table>	APPROVED <i>Chas. J. Dwyer</i> COLONEL, DISTRICT ENGINEER		SCALE	DATE <i>20 Oct 60</i>	SHEET <i>OF</i>	
APPROVED <i>Chas. J. Dwyer</i> COLONEL, DISTRICT ENGINEER							
SCALE	DATE <i>20 Oct 60</i>						
SHEET <i>OF</i>							

CORPS OF ENGINEERS





NOTE:
SEE LOG RECORDS NUMBER 1 FOR NOTES AND LEGEND

U. S. ARMY ENGINEER DISTRICT, ALASKA
CORPS OF ENGINEERS
ANCHORAGE, ALASKA

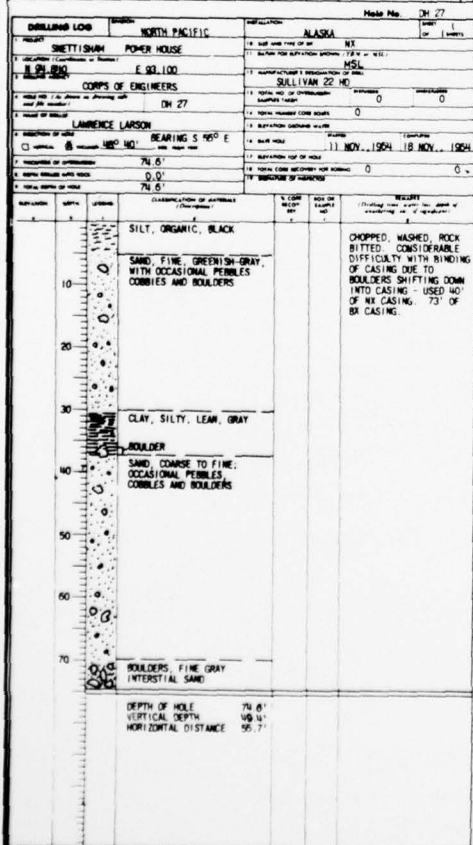
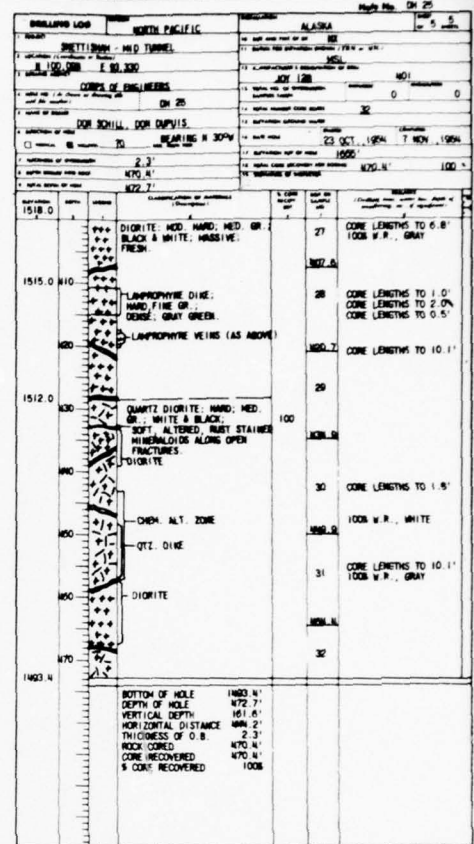
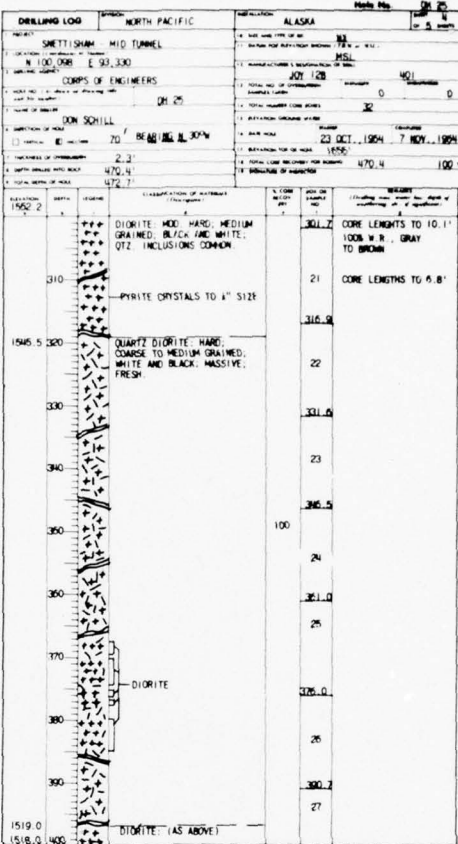
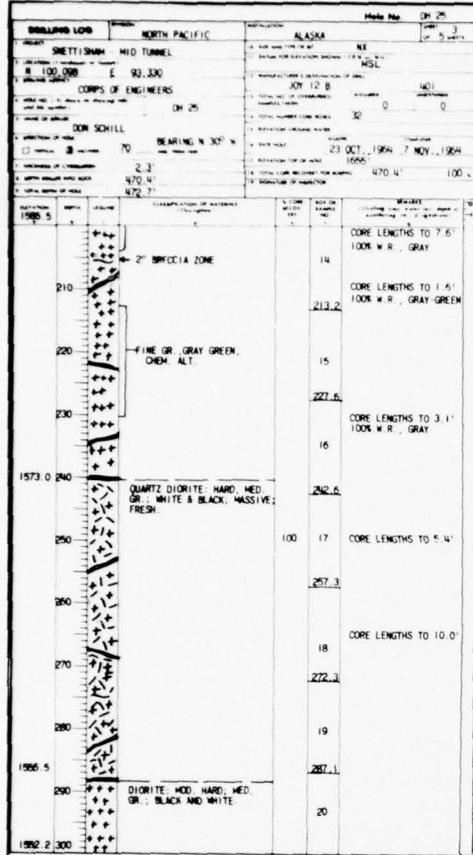
DESIGNED: *W. D. Kesteven*
DRAWN: *W. D. Kesteven*
CHECKED: *D. Hansen*
PREPARED: *D. Hansen*

LOG RECORD NO. 8

SUPervised: *W. D. Kesteven*
SUBMITTED: *W. D. Kesteven*
RECOMMENDED: *W. D. Kesteven*

APPROVED: *W. D. Kesteven*
SCALE: *1 inch = 10 feet*
DATE: *20 Oct 1964*
FILE NUMBER: *100-100-100*

CORPS OF ENGINEERS



DRILLING LOG		NORTH PACIFIC		ALASKA		Plate No. 21	
PROJECT: SNETTISHAM POWER HOUSE		E 93.000		SULLIVAN 22 HD		DATE: 31 OCT 1964	
CORPS OF ENGINEERS		DH 26		LAWRENCE LARSON		DATE: 31 OCT 1964	
1. NAME OF SITE		LAWRENCE LARSON		2. LOCATION OF SITE		3. DATE OF LOG	
4. NAME OF PERSON		LAWRENCE LARSON		5. LOCATION OF SITE		6. DATE OF LOG	
7. NAME OF PERSON		LAWRENCE LARSON		8. LOCATION OF SITE		9. DATE OF LOG	
10. NAME OF PERSON		LAWRENCE LARSON		11. LOCATION OF SITE		12. DATE OF LOG	
13. NAME OF PERSON		LAWRENCE LARSON		14. LOCATION OF SITE		15. DATE OF LOG	
16. NAME OF PERSON		LAWRENCE LARSON		17. LOCATION OF SITE		18. DATE OF LOG	
19. NAME OF PERSON		LAWRENCE LARSON		20. LOCATION OF SITE		21. DATE OF LOG	
22. NAME OF PERSON		LAWRENCE LARSON		23. LOCATION OF SITE		24. DATE OF LOG	
25. NAME OF PERSON		LAWRENCE LARSON		26. LOCATION OF SITE		27. DATE OF LOG	
28. NAME OF PERSON		LAWRENCE LARSON		29. LOCATION OF SITE		30. DATE OF LOG	
31. NAME OF PERSON		LAWRENCE LARSON		32. LOCATION OF SITE		33. DATE OF LOG	
34. NAME OF PERSON		LAWRENCE LARSON		35. LOCATION OF SITE		36. DATE OF LOG	
37. NAME OF PERSON		LAWRENCE LARSON		38. LOCATION OF SITE		39. DATE OF LOG	
40. NAME OF PERSON		LAWRENCE LARSON		41. LOCATION OF SITE		42. DATE OF LOG	
43. NAME OF PERSON		LAWRENCE LARSON		44. LOCATION OF SITE		45. DATE OF LOG	
46. NAME OF PERSON		LAWRENCE LARSON		47. LOCATION OF SITE		48. DATE OF LOG	
49. NAME OF PERSON		LAWRENCE LARSON		50. LOCATION OF SITE		51. DATE OF LOG	
52. NAME OF PERSON		LAWRENCE LARSON		53. LOCATION OF SITE		54. DATE OF LOG	
55. NAME OF PERSON		LAWRENCE LARSON		56. LOCATION OF SITE		57. DATE OF LOG	
58. NAME OF PERSON		LAWRENCE LARSON		59. LOCATION OF SITE		60. DATE OF LOG	
61. NAME OF PERSON		LAWRENCE LARSON		62. LOCATION OF SITE		63. DATE OF LOG	
64. NAME OF PERSON		LAWRENCE LARSON		65. LOCATION OF SITE		66. DATE OF LOG	
67. NAME OF PERSON		LAWRENCE LARSON		68. LOCATION OF SITE		69. DATE OF LOG	
70. NAME OF PERSON		LAWRENCE LARSON		71. LOCATION OF SITE		72. DATE OF LOG	
73. NAME OF PERSON		LAWRENCE LARSON		74. LOCATION OF SITE		75. DATE OF LOG	
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79. NAME OF PERSON		LAWRENCE LARSON		80. LOCATION OF SITE		81. DATE OF LOG	
82. NAME OF PERSON		LAWRENCE LARSON		83. LOCATION OF SITE		84. DATE OF LOG	
85. NAME OF PERSON		LAWRENCE LARSON		86. LOCATION OF SITE		87. DATE OF LOG	
88. NAME OF PERSON		LAWRENCE LARSON		89. LOCATION OF SITE		90. DATE OF LOG	
91. NAME OF PERSON		LAWRENCE LARSON		92. LOCATION OF SITE		93. DATE OF LOG	
94. NAME OF PERSON		LAWRENCE LARSON		95. LOCATION OF SITE		96. DATE OF LOG	
97. NAME OF PERSON		LAWRENCE LARSON		98. LOCATION OF SITE		99. DATE OF LOG	
100. NAME OF PERSON		LAWRENCE LARSON		101. LOCATION OF SITE		102. DATE OF LOG	

NOTE:
SEE LOG RECORDS NUMBER 1 FOR NOTES AND LEGEND

DESIGNED: <i>Wm. B. T. Smith</i>		DRAWN: <i>Wm. B. T. Smith</i>		CHECKED: <i>D. Hunter</i>		PREPARED: <i>D. Hunter</i>	
SUPERVISOR: <i>Wm. B. T. Smith</i>		SUBMITTED: <i>Wm. B. T. Smith</i>		RECOMMENDED: <i>Wm. B. T. Smith</i>		APPROVED: <i>Wm. B. T. Smith</i>	
CORPS OF ENGINEERS		MATERIALS BRANCH		SCALE: <i>1" = 10'</i>		DATE: <i>31 OCT 1964</i>	
SHEET: <i>1</i>		OF: <i>1</i>		FILE NUMBER: <i>100-100000-100</i>		PROJECT: <i>100-100000-100</i>	

AD-A067 895

CORPS OF ENGINEERS ANCHORAGE AK ALASKA DISTRICT
SNETTISHAM PROJECT ALASKA, FIRST STAGE DEVELOPMENT. DESIGN MEMO--ETC(U)
OCT 65

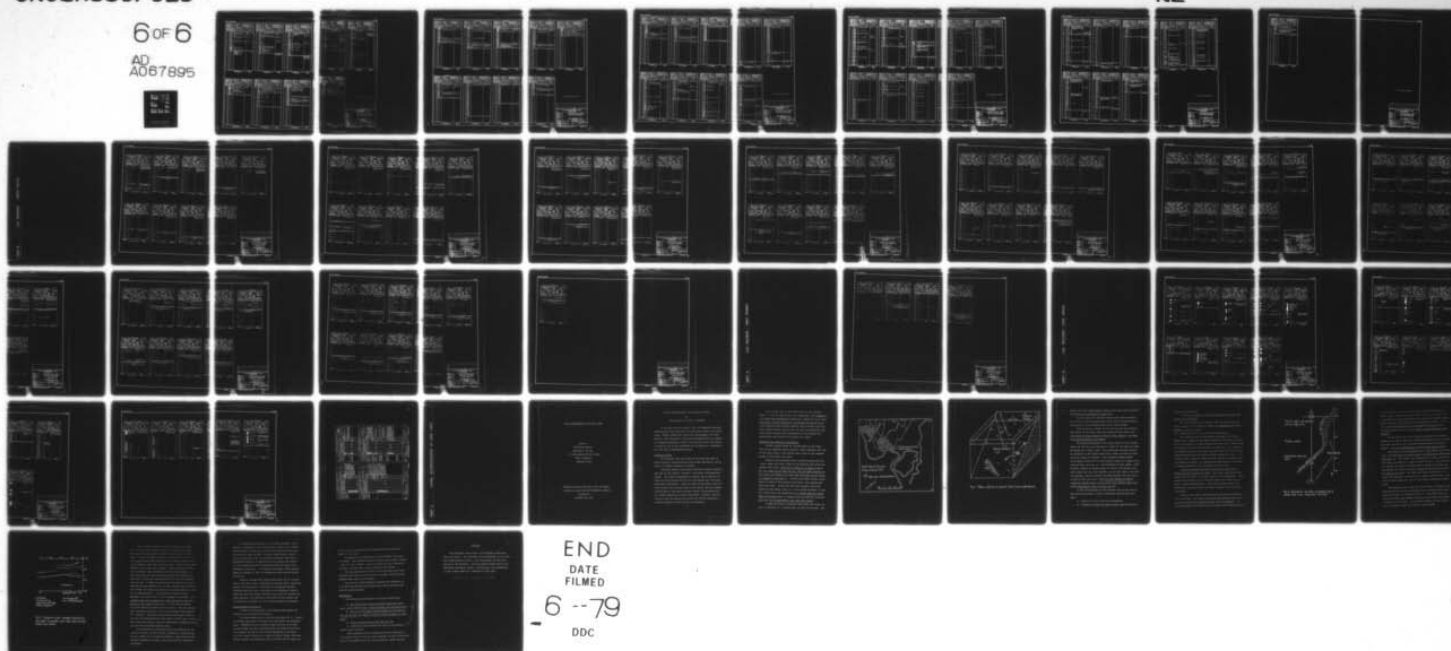
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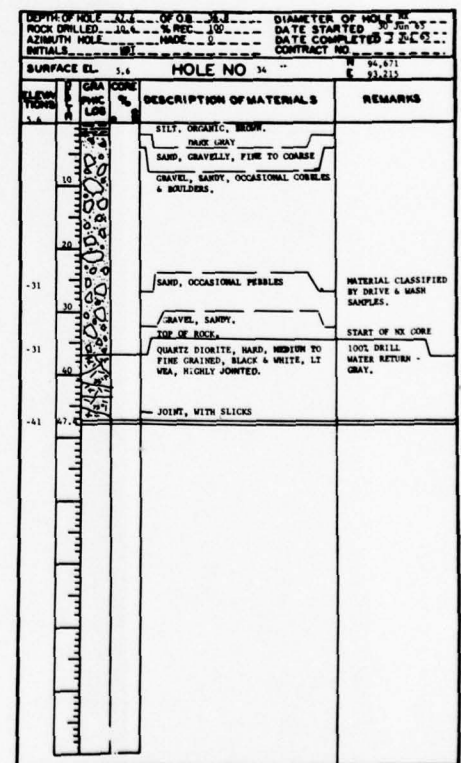
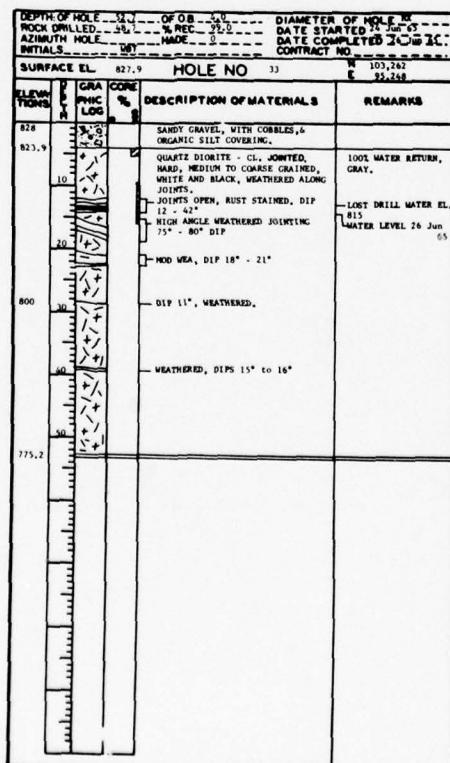
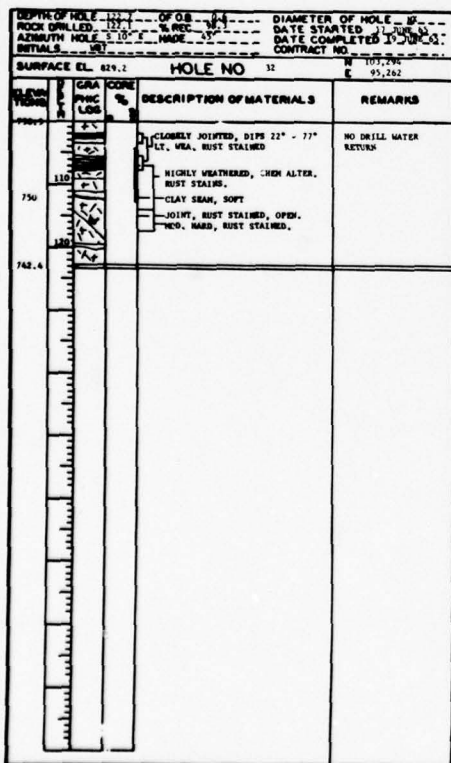
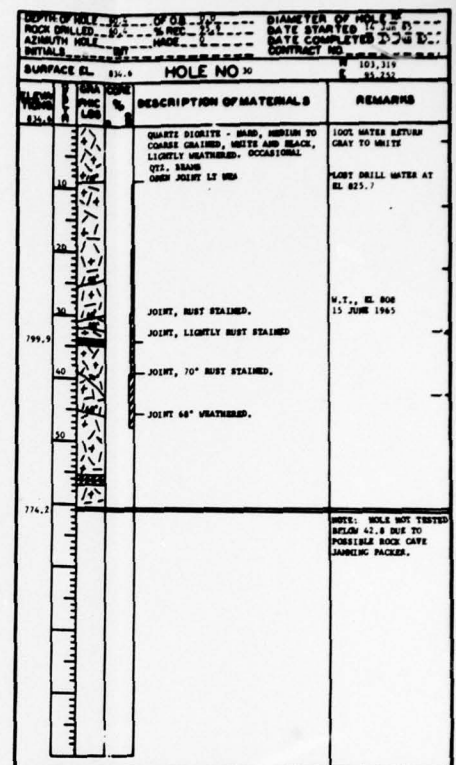
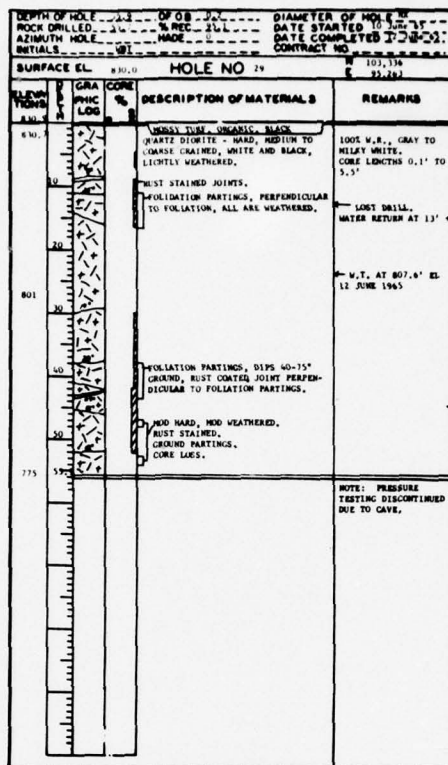
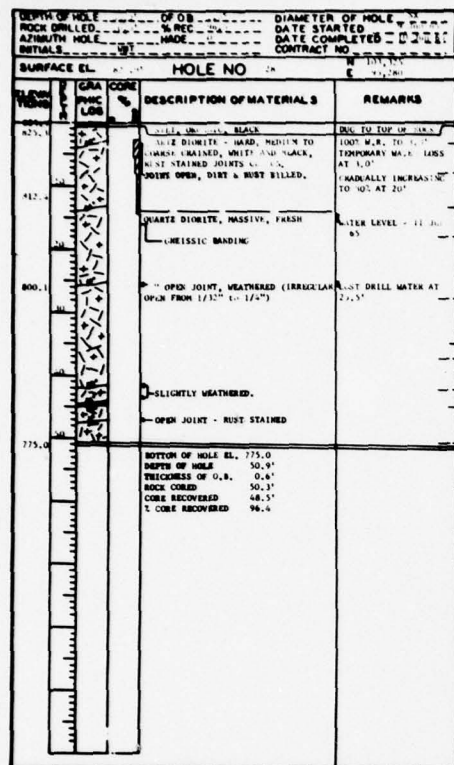
NL

6 OF 6

AD
A067895



CORPS OF ENGINEERS



2

CORPS OF ENGINEERS

DEPTH OF HOLE ROCK DRILLED		OF OB % REC		DIAMETER OF HOLE	
AZIMUTH HOLE		MADE		DATE STARTED	
INITIALS				DATE COMPLETED	
SURFACE EL.		HOLE NO.		CONTRACT NO.	
ELEVATION	GRA PNC LBS	DESCRIPTION OF MATERIALS	REMARKS		
10		SILT, DARK GRAY	MATERIAL DETERMINED BY DRIVE & WASH SAMPLES.		
20		SANDY GRAVEL, COBBLES OCCASIONAL TO COMMON, OCCASIONAL Boulders.			
23.2		TOP OF ROCK	START NO CORING		
23.4		QUARTZ DIORITE - HARD, MEDIUM GRAINED, WHITE & BLACK.	100% DRILL WATER RETURN - GRAY		
23.6		DIORITE - HARD, HARD GRAY BLACK.			

--- SHEETISHAN --- DR-16
SITE AND PROJECT HOLE NO.

DEPTH OF HOLE ROCK DRILLED		OF OB % REC		DIAMETER OF HOLE	
AZIMUTH HOLE		MADE		DATE STARTED	
INITIALS				DATE COMPLETED	
SURFACE EL.		HOLE NO.		CONTRACT NO.	
ELEVATION	GRA PNC LBS	DESCRIPTION OF MATERIALS	REMARKS		
10		SILT, DARK GRAY, SOMEWHAT ORGANIC	MATERIAL DETERMINED BY DRIVE & WASH SAMPLES IN OVERBINDER.		
20		SANDY GRAVEL, SAND COARSE TO FINE, GRAVEL TO COBBLE SIZE, CHIEFLY FINE.			
30		GRAVELLY SAND, OCCASIONAL COBBLE.			
37.9		SANDY GRAVEL	TOP OF ROCK.		
38.5		COBBLE GRAVEL	100% DRILL WATER RETURN, GRAY, BROWN GRAY BELOW 47.5' DEPTH.		
38.5		DIORITE - HARD, MED GRAIN, BLACK & WHITE, LIGHTLY WEATHERED JOINTS, SLICKENSIDES. DIP 50° to 72°.			

--- SHEETISHAN --- DR-17
SITE AND PROJECT HOLE NO.

DEPTH OF HOLE ROCK DRILLED		OF OB % REC		DIAMETER OF HOLE	
AZIMUTH HOLE		MADE		DATE STARTED	
INITIALS				DATE COMPLETED	
SURFACE EL.		HOLE NO.		CONTRACT NO.	
ELEVATION	GRA PNC LBS	DESCRIPTION OF MATERIALS	REMARKS		
10		SILT, ORGANIC, BLACK.	MATERIAL DETERMINED BY DRIVE & WASH SAMPLES IN OVERBINDER.		
20		SILT, DARK GRAY			
30		SANDY GRAVEL, COBBLES COMMON.	100% W.B., BROWN.		
34.3		TOP OF ROCK	START NO CORING.		
34.3		QUARTZ DIORITE - HARD, MEDIUM TO COARSE GRAINED, WHITE & BLACK. OPEN BUT STAINED JOINT.	100% W.B., GRAY BROWN W.B.		
34.3		DIORITE - HARD, MEDIUM GRAINED, BLACK & WHITE.			

--- SHEETISHAN --- DR-18
SITE AND PROJECT HOLE NO.

DEPTH OF HOLE ROCK DRILLED		OF OB % REC		DIAMETER OF HOLE	
AZIMUTH HOLE		MADE		DATE STARTED	
INITIALS				DATE COMPLETED	
SURFACE EL.		HOLE NO.		CONTRACT NO.	
ELEVATION	GRA PNC LBS	DESCRIPTION OF MATERIALS	REMARKS		
10		TOP SOIL - ORGANIC, BROWN	MATERIAL DETERMINED BY DRIVE & WASH SAMPLES IN OVERBINDER.		
20		GRAVELLY SAND - BEST BROWN			
21.8		SILTY SAND, GRAY GREEN			
21.8		GRAVELLY SAND			
21.8		QUARTZ DIORITE - HARD, MEDIUM TO COARSE GRAINED, WHITE & BLACK, MASSIVE, FOLIATION INDISTINCT.	100% DRILL WATER RETURN, GRAY		

--- SHEETISHAN --- DR-19
SITE AND PROJECT HOLE NO.

DEPTH OF HOLE ROCK DRILLED		OF OB % REC		DIAMETER OF HOLE	
AZIMUTH HOLE		MADE		DATE STARTED	
INITIALS				DATE COMPLETED	
SURFACE EL.		HOLE NO.		CONTRACT NO.	
ELEVATION	GRA PNC LBS	DESCRIPTION OF MATERIALS	REMARKS		
10		FOREST LITTER, MOSS LEAVES, ETC.	MATERIAL DETERMINED BY DRIVE AND WASH SAMPLES.		
20		SAND, GRAVEL, COBBLES AND Boulders			
30					
36			NO SAMPLES SAVED		

--- SHEETISHAN (POWERHOUSE) --- DR-42
SITE AND PROJECT HOLE NO.

DEPTH OF HOLE ROCK DRILLED		OF OB % REC		DIAMETER OF HOLE	
AZIMUTH HOLE		MADE		DATE STARTED	
INITIALS				DATE COMPLETED	
SURFACE EL.		HOLE NO.		CONTRACT NO.	
ELEVATION	GRA PNC LBS	DESCRIPTION OF MATERIALS	REMARKS		
10		PEAT, BROWN AND BLACK	MATERIAL DETERMINED BY DRIVE AND WASH SAMPLES		
20		SAND, GRAVEL, COBBLES AND Boulders			
30					
37			NO SAMPLES SAVED		

--- SHEETISHAN (POWERHOUSE) --- DR-43
SITE AND PROJECT HOLE NO.

DEPTH OF HOLE 11.5		DIAMETER OF HOLE 11.5	
ROCK DRILLED 11.5		DATE STARTED 10/10/65	
AZIMUTH HOLE 11.5		DATE COMPLETED 10/10/65	
INITIALS 11.5		CONTRACT NO. 11.5	
SURFACE EL. 11.5 HOLE NO. 11.5			
DEPTH	GRAVIMETER LOG	DESCRIPTION OF MATERIALS	REMARKS
11.5		SILE, ORGANIC, GRAY BLACK GRAVELLY SAND, SAND IS FINE TO COARSE.	WITNESS DETERMINED IF NEEDED & WASH SAMPLES IN OVER- BURN
11.5		SANDY GRAVEL, COBBLES COMMON, OCCASIONAL BOULDER.	
11.5		TOP OF ROCK	
11.5		QUARTZ DIORITE - HARD, MEDIUM GRAINED, WHITE & BLACK, LIGHT STAINING.	100% DRILL WATER RETURN - GRAY

SHETTISHAM
SITE AND PROJECT

DR-17
HOLE NO.

DEPTH OF HOLE 11.5		DIAMETER OF HOLE 11.5	
ROCK DRILLED 11.5		DATE STARTED 10/10/65	
AZIMUTH HOLE 11.5		DATE COMPLETED 10/10/65	
INITIALS 11.5		CONTRACT NO. 11.5	
SURFACE EL. 11.5 HOLE NO. 11.5			
DEPTH	GRAVIMETER LOG	DESCRIPTION OF MATERIALS	REMARKS
11.5		SILTY SAND, SOME GRAVEL, MEDIUM.	TOP OF ROCK.
11.5		DIORITE - MED. GRAINED BLACK, WHITE QZ BANDS COMMON.	
11.5		QUARTZ DIORITE - HARD, MEDIUM TO COARSE GRAINED, WHITE & BLACK JOINT, SLICKS	100% DRILL WATER RETURN - GRAY

SHETTISHAM
SITE AND PROJECT

DR-17
HOLE NO.

DEPTH OF HOLE 11.5		DIAMETER OF HOLE 11.5	
ROCK DRILLED 11.5		DATE STARTED 10/10/65	
AZIMUTH HOLE 11.5		DATE COMPLETED 10/10/65	
INITIALS 11.5		CONTRACT NO. 11.5	
SURFACE EL. 11.5 HOLE NO. 11.5			
DEPTH	GRAVIMETER LOG	DESCRIPTION OF MATERIALS	REMARKS
11.5		FOREST LITTER, MOSS, LEAVES, ETC.	NO SAMPLES SAVED
11.5		SAND, GRAVEL, COBBLES AND BOULDERS	
11.5		TOP OF ROCK	
11.5		QUARTZ DIORITE - HARD, MEDIUM TO COARSE GRAINED, WHITE AND BLACK, LIGHTLY WEATHERED, MASSIVE	START OF RX CORE 100% DRILL WATER RETURN
11.5			1 BOX OF CORE

NOTE:
SEE LOG RECORD NUMBER 1 FOR NOTES AND LEGEND

U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
SHETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT	
LOG RECORDS NO. 11	
DESIGNED By: A. T. T. T. T.	APPROVED By: J. J. J. J.
DRAWN By: J. J. J. J.	SCALE DATE: 10/10/65
CHECKED By: J. J. J. J.	FILE NUMBER
PREPARED By: J. J. J. J.	SHEET OF

DEPTH OF HOLE				OF OB		DIAMETER OF HOLE	
ROCK DRILLED				% REC		DATE STARTED	
AZIMUTH HOLE				% REC		DATE COMPLETED	
INITIALS				DATE		CONTRACT NO.	
SURFACE EL.				HOLE NO.		DATE	
ELEVATION				GRA		CORE	
LOG				DESCRIPTION OF MATERIALS		REMARKS	
11				FOREST LITTER, LEAVES, WOOD, PLANT		NO SAMPLES SAVED	
21				SAND, GRAVEL, COBBLES AND BOULDERS			
31				TOP OF ROCK			
41				QUARTZ DIORITE - HARD, MEDIUM GRAINED, JOINTED TO MASSIVE, LIGHTLY WEATHERED FOR 2-4 FEET, BLACK AND WHITE		1 BOX OF CORE	

SNETTISHAM
SITE AND PROJECT

HOLE NO.

DEPTH OF HOLE				OF OB		DIAMETER OF HOLE	
ROCK DRILLED				% REC		DATE STARTED	
AZIMUTH HOLE				% REC		DATE COMPLETED	
INITIALS				DATE		CONTRACT NO.	
SURFACE EL.				HOLE NO.		DATE	
ELEVATION				GRA		CORE	
LOG				DESCRIPTION OF MATERIALS		REMARKS	
21				FOREST LITTER, LEAVES, WOOD, PLANT ETC.		MATERIALS DETERMINED BY WASH SAMPLES AND SIFTINGS	
31				SAND, GRAVEL, COBBLES AND BOULDERS			
41				TOP OF ROCK		START RE CORE	
51				QUARTZ DIORITE - MODERATELY HARD TO HARD, FINE TO MEDIUM GRAINED, JOINTED TO MASSIVE, BLACK & WHITE		60 TO 80% WATER RETURN	
61						1 BOX OF CORE	

SNETTISHAM
SITE AND PROJECT

HOLE NO.

DEPTH OF HOLE				OF OB		DIAMETER OF HOLE	
ROCK DRILLED				% REC		DATE STARTED	
AZIMUTH HOLE				% REC		DATE COMPLETED	
INITIALS				DATE		CONTRACT NO.	
SURFACE EL.				HOLE NO.		DATE	
ELEVATION				GRA		CORE	
LOG				DESCRIPTION OF MATERIALS		REMARKS	
13				FOREST LITTER, LEAVES, WOOD, ETC.		MATERIALS DETERMINED BY WASH SAMPLES AND SIFTINGS	
23				SAND, GRAVEL, COBBLES AND BOULDERS			
33						NO SAMPLES SAVED	

SNETTISHAM
SITE AND PROJECT

HOLE NO.

DEPTH OF HOLE				OF OB		DIAMETER OF HOLE	
ROCK DRILLED				% REC		DATE STARTED	
AZIMUTH HOLE				% REC		DATE COMPLETED	
INITIALS				DATE		CONTRACT NO.	
SURFACE EL.				HOLE NO.		DATE	
ELEVATION				GRA		CORE	
LOG				DESCRIPTION OF MATERIALS		REMARKS	
10				DIORITE - MOD. HARD, MEDIUM GRAINED, GRAY-BLACK		100% DRILL WATER RETURN, GREEN GRAY	
20				QUARTZ DIORITE - HARD, MEDIUM GRAINED, WHITE AND BLACK, MASSIVE		K 40	
30				LOST DRILL WATER		NO CASING USED	
40						K 23	
50				JOINTS 18" - 12" SLIGHTLY WEATHERED		K 19	
60				JOINTS, 18" - 18" WEATHERED		WATER LOSS	
70				ZONE OF JOINTING 11" - 82" WEATHERED & RUST STAINED		K 0.5	
80				MOD WEATHERED, RUST MOD SOFT		K 0	
90						K 0	
100				BOTTOM OF HOLE		K 0	

SNETTISHAM
SITE AND PROJECT

HOLE NO.

DEPTH OF HOLE				OF OB		DIAMETER OF HOLE	
ROCK DRILLED				% REC		DATE STARTED	
AZIMUTH HOLE				% REC		DATE COMPLETED	
INITIALS				DATE		CONTRACT NO.	
SURFACE EL.				HOLE NO.		DATE	
ELEVATION				GRA		CORE	
LOG				DESCRIPTION OF MATERIALS		REMARKS	
10				DIORITE - MOD. HARD, MEDIUM GRAINED, GRAY-BLACK		100% W.R. (DRILL WATER RETURN) GRAY	
20				QUARTZ DIORITE - HARD MEDIUM GRAINED, WHITE AND BLACK		LOST DRILL WATER EL. 815	
30				JOINTS 52 & 58" WEATHERED AND STAINED		K 50	
40				OPEN FISSURE OR CLAY FILLED JOINT		K 12	
50				ZONE OF MOD WEATHERING, RUST STAINING ALONG HIGH ANGLE JOINTS		0.5" VERY FAST PENETRATION	
60				JOINTS 18 - 66" LIGHT WEATHERING		K 29	
70				CLOSE JOINTS 11" - 37", LT WEA		K 0.5	

SNETTISHAM
SITE AND PROJECT

HOLE NO.

DEPTH OF HOLE				OF OB		DIAMETER OF HOLE	
ROCK DRILLED				% REC		DATE STARTED	
AZIMUTH HOLE				% REC		DATE COMPLETED	
INITIALS				DATE		CONTRACT NO.	
SURFACE EL.				HOLE NO.		DATE	
ELEVATION				GRA		CORE	
LOG				DESCRIPTION OF MATERIALS		REMARKS	
10				QUARTZ DIORITE - HARD, MEDIUM GRAINED, WHITE AND BLACK, OCCASIONAL SEAMS OF MOD HARD GRAY BLACK DIORITE		100% DRILL WATER RETURN, WHITE-GRAY	
20				OPEN FRACTURES		LOST DRILL WATER AT EL. 814.7	
30				JOINT, RUST STAINED		K 454	
40				MOD WEA		K 474	
50				JOINT RUST STAINED		K 0	
60						K 0.7	
70				OPEN FRACTURES, LIGHT WEATHERED		K 22	
80				JOINTS, OPEN, LIGHTLY WEATHERED 12 - 20"		K 18	
90				MOD WEA, OPEN		K 0.0	
100				JOINT, WEA, RUST STAINED		K 0.2	
110				JOINT OPEN, FILLED 5" WITH RESIDUAL WOODEN - DIORITE		K 0.13	
120				ZONE OF JOINTS, WEA, RUST STAINED, DIPS 1" - 20"		K 0.1	
130				JOINTS WITH 5" - 3/4", RUST STAINED DIORITE SEAM, MOD SOFT BOTTOM		K 0.1	

SNETTISHAM
SITE AND PROJECT

HOLE NO.

DEPTH OF HOLE		DIAMETER OF HOLE	
IN	FEET	IN	FEET
1.0	0.3	1.0	0.3
DATE STARTED 10-15-57		DATE COMPLETED 10-15-57	
HOLE NO. 103-153		CONTRACT NO. 32-123	
SURFACE EL. 815.3			
HOLE NO. 103-153			
DESCRIPTION OF MATERIALS			
WATER - LONG LAKE			
SILT, SAND, GRAVEL, Boulders, TREES, BRANCHES AND OTHER DEBRIS			

SNETTISHAM LONG LAKE 2-2-1
SITE AND PROJECT

HOLE NO.

DEPTH OF HOLE		DIAMETER OF HOLE	
IN	FEET	IN	FEET
1.0	0.3	1.0	0.3
DATE STARTED 10-15-57		DATE COMPLETED 10-15-57	
HOLE NO. 103-153		CONTRACT NO. 32-123	
SURFACE EL. 815.3			
HOLE NO. 103-153			
DESCRIPTION OF MATERIALS			
SILT, SAND, GRAVEL, Boulders, BRANCHES AND OTHER DEBRIS			
TOP OF ROCK			
QUARTZ DIORITE - HARD, FINE TO COARSE GRAINED, MASSIVE			
BOTTOM OF HOLE			

SNETTISHAM LONG LAKE 2-2-1
SITE AND PROJECT

HOLE NO.

DEPTH OF HOLE		DIAMETER OF HOLE	
IN	FEET	IN	FEET
1.0	0.3	1.0	0.3
DATE STARTED 10-15-57		DATE COMPLETED 10-15-57	
HOLE NO. 103-153		CONTRACT NO. 32-123	
SURFACE EL. 815.3			
HOLE NO. 103-153			
DESCRIPTION OF MATERIALS			
WATER			
BOULDERS, CORBELS AND GRAVELS WITH INTERSTITIAL SILT AND SAND			
TOP OF ROCK			
QUARTZ DIORITE - HARD, MEDIUM TO COARSE GRAINED, WHITE AND BLACK, IRREGULAR QUARTZ PEGGON AND DIORITE ZONES			
JOINT, LT. MUST STAINED			
JOINTING, MD. WEA. MUST STAINED			
FRESH, MASSIVE			
LEGEND WATER RETURN FROM LOGS EL. 751.4			
TOP DRILL WATER RETURN, WHITE-CRAY			

SNETTISHAM LONG LAKE 2-2-1
SITE AND PROJECT

HOLE NO.

NOTE:
SEE LOG RECORD NUMBER 1 FOR NOTES AND LEGEND

U. S. ARMY ENGINEER DISTRICT, ALASKA	
CORPS OF ENGINEERS	
ANCHORAGE, ALASKA	
DESIGNED:	SNETTISHAM PROJECT, ALASKA
DRAWN:	FIRST STAGE DEVELOPMENT
CHECKED:	
PREPARED:	
LOG RECORDS NO. 12	
SUPERVISOR:	APPROVED:
DATE: 10-15-57	DATE: 10-15-57
SCALE:	FILE NUMBER:
SHEET:	OF:

DEPTH OF HOLE		DIAMETER OF HOLE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
ROCK DRILLED		% REC.		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
AZIMUTH HOLE		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
INITIALS		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
SURFACE EL. 820.1		HOLE NO. 10-31		N 101.414		E 95.220			
ELEVATION		CORE		DESCRIPTION OF MATERIALS		REMARKS			
720.4		100%		QUARTZ DIORITE - HARD, MEDIUM GRAINED, WHITE AND BLACK		100% WATER RETURN			
721.0		100%		JOINTS SLIGHTLY WEATHERED		K 10			
721.5		100%		NOT TO 1" HORIZ. CLAY SPAN, HARD WEATHERED, THIS FILM OF SILT		K 11			
722.0		100%		JOINTS, SLIGHTLY WEATHERED		K 12			
722.5		100%		JOINTS, SLIGHTLY WEATHERED		K 13			
723.0		100%		JOINTS, SLIGHTLY WEATHERED		K 14			
723.5		100%		JOINTS, SLIGHTLY WEATHERED		K 15			
724.0		100%		JOINTS, SLIGHTLY WEATHERED		K 16			
724.5		100%		JOINTS, SLIGHTLY WEATHERED		K 17			
725.0		100%		JOINTS, SLIGHTLY WEATHERED		K 18			
725.5		100%		JOINTS, SLIGHTLY WEATHERED		K 19			
726.0		100%		JOINTS, SLIGHTLY WEATHERED		K 20			

SUTTERMAN

HOLE NO.

DEPTH OF HOLE		DIAMETER OF HOLE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
ROCK DRILLED		% REC.		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
AZIMUTH HOLE		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
INITIALS		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
SURFACE EL. 820.1		HOLE NO. 10-32		N 101.414		E 95.220			
ELEVATION		CORE		DESCRIPTION OF MATERIALS		REMARKS			
720.4		100%		QUARTZ DIORITE - HARD, MEDIUM GRAINED, WHITE AND BLACK		100% WATER RETURN			
721.0		100%		JOINT (OPEN) 15"		K 10			
721.5		100%		CLOSELY JOINTED ZONE 15" - 30"		K 11			
722.0		100%		JOINT (STAINED) 23"		K 12			
722.5		100%		4 BOXES OF CORE		K 13			
723.0		100%				K 14			
723.5		100%				K 15			
724.0		100%				K 16			
724.5		100%				K 17			
725.0		100%				K 18			
725.5		100%				K 19			
726.0		100%				K 20			

SUTTERMAN

HOLE NO.

DEPTH OF HOLE		DIAMETER OF HOLE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
ROCK DRILLED		% REC.		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
AZIMUTH HOLE		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
INITIALS		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
SURFACE EL. 813.3		HOLE NO. 10-33		N 101.331		E 95.161			
ELEVATION		CORE		DESCRIPTION OF MATERIALS		REMARKS			
720.4		100%		WATER - LONG LANE		K 10			
721.0		100%		BOTTOM OF LANE		K 11			
721.5		100%		SILT, SAND, GRAVEL, COARSE AND Boulders		K 12			
722.0		100%		TOP OF ROCK		K 13			
722.5		100%		QUARTZ DIORITE - HARD TO MEDIUM GRAINED, IMMEDIATELY WEATHERED, CLOSELY JOINTED, WHITE AND BLACK		K 14			
723.0		100%		QUARTZ DIORITE - HARD, MEDIUM TO COARSE GRAINED, MEDIUM, WHITE AND BLACK		K 15			
723.5		100%		3 BOXES OF CORE		K 16			
724.0		100%				K 17			
724.5		100%				K 18			
725.0		100%				K 19			
725.5		100%				K 20			

SUTTERMAN

HOLE NO.

DEPTH OF HOLE		DIAMETER OF HOLE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
ROCK DRILLED		% REC.		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
AZIMUTH HOLE		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
INITIALS		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
SURFACE EL. 813.4		HOLE NO. 10-34		N 101.414		E 95.220			
ELEVATION		CORE		DESCRIPTION OF MATERIALS		REMARKS			
720.4		100%		WATER, LONG LANE		K 10			
721.0		100%				K 11			
721.5		100%				K 12			
722.0		100%				K 13			
722.5		100%				K 14			
723.0		100%				K 15			
723.5		100%				K 16			
724.0		100%				K 17			
724.5		100%				K 18			
725.0		100%				K 19			
725.5		100%				K 20			

SUTTERMAN

HOLE NO.

DEPTH OF HOLE		DIAMETER OF HOLE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
ROCK DRILLED		% REC.		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
AZIMUTH HOLE		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
INITIALS		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
SURFACE EL. 813.0		HOLE NO. 10-35		N 101.331		E 95.161			
ELEVATION		CORE		DESCRIPTION OF MATERIALS		REMARKS			
720.4		100%		QUARTZ DIORITE - HARD, MEDIUM GRAINED, WHITE AND BLACK, 17' HORIZ. CLAY SPAN, HARD WEATHERED, THIS FILM OF SILT		K 10			
721.0		100%		WEA, BROWN OXIDIZED, HARD SOFT		K 11			
721.5		100%		HARD WEA, RUST STAINED, CORE BADLY BROKEN		K 12			
722.0		100%		NOTE: TOTAL DEVIATION SURVEY INDICATED 2" - 4" DEVIATION FROM VERTICAL AT 131.4		K 13			
722.5		100%				K 14			
723.0		100%				K 15			
723.5		100%				K 16			
724.0		100%				K 17			
724.5		100%				K 18			
725.0		100%				K 19			
725.5		100%				K 20			

SUTTERMAN

HOLE NO.

DEPTH OF HOLE		DIAMETER OF HOLE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
ROCK DRILLED		% REC.		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
AZIMUTH HOLE		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
INITIALS		MADE		DATE STARTED		DATE COMPLETED		CONTRACT NO.	
SURFACE EL. 803.4		HOLE NO. 10-36		N 101.331		E 95.161			
ELEVATION		CORE		DESCRIPTION OF MATERIALS		REMARKS			
720.4		100%		ORGANIC SANDY SILT, BROWN		K 10			
721.0		100%		QUARTZ DIORITE - HARD, MEDIUM GRAINED, WHITE & BLACK, WITH LOCAL VARIATIONS TO DIORITE, DENSE, DARK GRAY TO BLACK		K 11			
721.5		100%		OPEN JOINT; WATER LOSS		K 12			
722.0		100%		LIGHTLY WEATHERED JOINTS		K 13			
722.5		100%		LIGHTLY WEATHERED JOINTS		K 14			
723.0		100%		LIGHTLY WEATHERED JOINTS		K 15			
723.5		100%		LIGHTLY WEATHERED JOINTS		K 16			
724.0		100%		LIGHTLY WEATHERED JOINTS		K 17			
724.5		100%		LIGHTLY WEATHERED JOINTS		K 18			
725.0		100%		LIGHTLY WEATHERED JOINTS		K 19			
725.5		100%		LIGHTLY WEATHERED JOINTS		K 20			

SUTTERMAN

HOLE NO.

DEPTH OF HOLE		OF OB		DIAMETER OF HOLE	
ROCK DRILLED		% REC		DATE STARTED	
AZIMUTH HOLE		MADE		DATE COMPLETED	
INITIALS		CONTRACT NO.		CONTRACT NO.	
SURFACE EL. 847.7 HOLE NO. 08-16 E 92,127					
ELEVATION	GRA	CORE	DESCRIPTION OF MATERIALS	REMARKS	
FEET	LOG	%			
700			ORGANIC SANDY SILT		K 40
705			QUARTZ DIORITE - HARD, MEDIUM GRAINED, WHITE & BLACK, SLIGHTLY WEATHERED, BEST STAINED JOINTS GREEN		K 40
710			JOINT WITH 1/16" SOFT, BEST MANN FILLING	LOST WATER AT	K 10.9
715			JOINT, OPEN LT WEA		K 7.7
720					K 0
725					K 20
730					K 0.8
735			JOINT ZONE, LT WEA		K 0
740			JOINT, 1" SILT & SAND SIZE PARTICLES OF BUTTER ROCK		K 0
745					K 0
750					K 0

SNETTISHAM
SITE AND PROJECT

HOLE NO.

DEPTH OF HOLE		OF OB		DIAMETER OF HOLE	
ROCK DRILLED		% REC		DATE STARTED	
AZIMUTH HOLE		MADE		DATE COMPLETED	
INITIALS		CONTRACT NO.		CONTRACT NO.	
SURFACE EL. 847.7 HOLE NO. 08-16 E 92,127					
ELEVATION	GRA	CORE	DESCRIPTION OF MATERIALS	REMARKS	
FEET	LOG	%			
700					K 10
705					K 0
710					K 0
715					K 0
720					K 0
725			JOINT, CHLORITE COATED		K 0
730					K 0
735			JOINT, CHLORITE FILM COATING		K 0
740			CLOSE JOINTING, LT TO MOD WEA, ALONG JOINTS, WITH HENKALOID FILLING. ANGLES FROM 2" TO 8"		K 0.15
745					K 0
750					K 0
755					K 0
760					K 0
765					K 0
770					K 0
775					K 0
780					K 0
785					K 0
790					K 0
795					K 0
800					K 0
805					K 0
810					K 0
815					K 0
820					K 0
825					K 0
830					K 0
835					K 0
840					K 0
845					K 0
850					K 0
855					K 0
860					K 0
865					K 0
870					K 0
875					K 0
880					K 0
885					K 0
890					K 0
895					K 0
900					K 0
905					K 0
910					K 0
915					K 0
920					K 0
925					K 0
930					K 0
935					K 0
940					K 0
945					K 0
950					K 0
955					K 0
960					K 0
965					K 0
970					K 0
975					K 0
980					K 0
985					K 0
990					K 0
995					K 0
1000					K 0

SNETTISHAM
SITE AND PROJECT

HOLE NO.

DEPTH OF HOLE		OF OB		DIAMETER OF HOLE	
ROCK DRILLED		% REC		DATE STARTED	
AZIMUTH HOLE		MADE		DATE COMPLETED	
INITIALS		CONTRACT NO.		CONTRACT NO.	
SURFACE EL. 847.7 HOLE NO. 08-16 E 92,127					
ELEVATION	GRA	CORE	DESCRIPTION OF MATERIALS	REMARKS	
FEET	LOG	%			
700			QUARTZ DIORITE AS ABOVE		K 0.11
705			DEEPLY WEATHERED JOINT		K 0.19
710					K 0
715			Moderately weathered joint		K 0.08
720					K 0
725			Moderately weathered joint		K 0
730					K 0
735			Moderately weathered joint		K 0
740					K 0
745			Lightly weathered		K 0
750					K 0
755					K 0
760					K 0
765					K 0
770					K 0
775					K 0
780					K 0
785					K 0
790					K 0
795					K 0
800					K 0
805					K 0
810					K 0
815					K 0
820					K 0
825					K 0
830					K 0
835					K 0
840					K 0
845					K 0
850					K 0
855					K 0
860					K 0
865					K 0
870					K 0
875					K 0
880					K 0
885					K 0
890					K 0
895					K 0
900					K 0
905					K 0
910					K 0
915					K 0
920					K 0
925					K 0
930					K 0
935					K 0
940					K 0
945					K 0
950					K 0
955					K 0
960					K 0
965					K 0
970					K 0
975					K 0
980					K 0
985					K 0
990					K 0
995					K 0
1000					K 0

SNETTISHAM
SITE AND PROJECT

HOLE NO.

NOTE:
SEE LOG RECORD NUMBER 1 FOR NOTES AND LOGGING

U. S. ARMY ENGINEER DISTRICT, ALASKA	
CORPS OF ENGINEERS	
ANCHORAGE, ALASKA	
SNETTISHAM PROJECT, ALASKA	
FIRST STAGE DEVELOPMENT	
LOG RECORDS NO. 13	
APPROVED	DATE 25 OCT 60
SCALE	FILE NUMBER
DATE	OF

CORPS OF ENGINEERS

DEPTH OF HOLE 50.1		OF OB 5.1		DIAMETER OF HOLE 11	
ROCK DRILLED 57.5		% REC 5.1		DATE STARTED 1-27-57	
AZIMUTH HOLE 345		MADE 57		DATE COMPLETED 2-14-57	
INITIALS JET		CONTRACT NO 103,712		E 92,284	
SURFACE EL 844.4 HOLE NO 59-59					
ELEVATION	GRA	CORE	DESCRIPTION OF MATERIALS	REMARKS	
FEET	LOG	%			
10			ORGANIC SILT, DARK BROWN		
15			QUARTZ DIORITE - MD, MED TO COARSE GRAINED, WHITE & BLACK, LT WEA, WITH ZONES OF MD WEA, MUST STAINED CLOSE JOINTING	E 8.7	
20				E 7.7	
25				E 7.2	
30			LOST DRILL WATER AT ELEV 818"		
35			CONDENSED THIS ZONE DUE TO VERY HEAVY CAVE IN LATER DAYS	E 9.1	
40				E 9.0	
45				E 8.2	
50			JOINT WITH 1" MD SOFT, BLACK NICA SEAM	E 8.3	
55				E 7.1	
60			OPEN JOINT, 0.3", DIP 6" - 28"	E 6.6	
65				E 6.2	
70				E 5.0	
75			MD TO MASSIVE JOINTING	E 3.07	
80				E 3.05	
85					
90					
95					
100					
			4 BOXES OF CORE		

SMITHSONIAN SITE AND PROJECT HOLE NO.

DEPTH OF HOLE 123.2		OF OB 5.3		DIAMETER OF HOLE 11	
ROCK DRILLED 26.0		% REC 5.3		DATE STARTED 1-27-57	
AZIMUTH HOLE 345		MADE 57		DATE COMPLETED 2-14-57	
INITIALS JET		CONTRACT NO 103,717		E 92,322	
SURFACE EL 813.9 HOLE NO 59-60					
ELEVATION	GRA	CORE	DESCRIPTION OF MATERIALS	REMARKS	
FEET	LOG	%			
10			WATER, LONG LANE		
15					
20					
25					
30					
35					
40					
45					
50					
55					
60					
65					
70					
75					
80					
85					
90					
95					
100					
			TOP OF ROCK		
			QUARTZ DIORITE - MD, MED TO CRS GR, WHITE AND BLACK, MASSIVE JOINTING		

SMITHSONIAN SITE AND PROJECT HOLE NO.

DEPTH OF HOLE 111.1		OF OB 5.3		DIAMETER OF HOLE 11	
ROCK DRILLED 26.0		% REC 5.3		DATE STARTED 1-27-57	
AZIMUTH HOLE 345		MADE 57		DATE COMPLETED 2-14-57	
INITIALS JET		CONTRACT NO 103,717		E 92,322	
SURFACE EL 813.9 HOLE NO 59-60					
ELEVATION	GRA	CORE	DESCRIPTION OF MATERIALS	REMARKS	
FEET	LOG	%			
10			QUARTZ DIORITE, AS ABOVE		
15					
20					
25					
30					
35					
40					
45					
50					
55					
60					
65					
70					
75					
80					
85					
90					
95					
100					
			2 BOXES OF CORE		

SMITHSONIAN SITE AND PROJECT HOLE NO.

DEPTH OF HOLE 50.0		OF OB 5.0		DIAMETER OF HOLE 11	
ROCK DRILLED 57.5		% REC 5.0		DATE STARTED 1-27-57	
AZIMUTH HOLE 345		MADE 57		DATE COMPLETED 2-14-57	
INITIALS JET		CONTRACT NO 103,407		E 92,442	
SURFACE EL 812 HOLE NO 59-63					
ELEVATION	GRA	CORE	DESCRIPTION OF MATERIALS	REMARKS	
FEET	LOG	%			
10			QUARTZ DIORITE - HARD, MEDIUM TO COARSE GRAINED, WHITE & BLACK, SLIGHT TO MD WEA, MUST STAINING, OCCASIONAL CHLORITE FILM ON JOINTS, CLOSE TO MASSIVE JOINTING.	E 38	
15				E 32	
20				E 39	
25				E 9.1	
30					
35					
40					
45					
50					
55					
60					
65					
70					
75					
80					
85					
90					
95					
100					
			5 CORE BOXES		

SMITHSONIAN SITE AND PROJECT HOLE NO.

DEPTH OF HOLE 123.2		OF OB 5.3		DIAMETER OF HOLE 11	
ROCK DRILLED 22.8		% REC 100		DATE STARTED 1-27-57	
AZIMUTH HOLE 345		MADE 57		DATE COMPLETED 2-14-57	
INITIALS JET		CONTRACT NO 103,397		E 92,445	
SURFACE EL 822.2 HOLE NO 59-64					
ELEVATION	GRA	CORE	DESCRIPTION OF MATERIALS	REMARKS	
FEET	LOG	%			
10			SILTY SANDY GRAVEL, COBBLES COMMON OCCASIONAL BOULDER UP TO 2.0"		
15					
20					
25					
30					
35					
40					
45					
50					
55					
60					
65					
70					
75					
80					
85					
90					
95					
100					
			TOP OF ROCK		
			100% DRILL WATER RETURN		
			DIORITE - MD HARD, MEDIUM TO COARSE GRAINED, BLACK & WHITE, MASSIVE, FRESH QUARTZ SEAMS COMMON	E 1.0	
			QUARTZ DIORITE - HARD		
			2 CORE BOXES OF CORE		

SMITHSONIAN SITE AND PROJECT HOLE NO.

DEPTH OF HOLE 111.1		OF OB 5.3		DIAMETER OF HOLE 11	
ROCK DRILLED 22.8		% REC 26.4		DATE STARTED 1-27-57	
AZIMUTH HOLE 345		MADE 57		DATE COMPLETED 2-14-57	
INITIALS JET		CONTRACT NO 92,344		E 92,621	
SURFACE EL 1.4 HOLE NO 59-65					
ELEVATION	GRA	CORE	DESCRIPTION OF MATERIALS	REMARKS	
FEET	LOG	%			
10			SILT, ORGANIC, GRAY BLACK, SANDY GRAVEL, OCCASIONAL COBBLES AND BOULDERS		
15					
20					
25					
30					
35					
40					
45					
50					
55					
60					
65					
70					
75					
80					
85					
90					
95					
100					
			TOP OF ROCK		
			QUARTZ DIORITE - HARD, MED GRAINED, WHITE AND BLACK, MASSIVE, LT WEA, MUST STAINING, OCCASIONAL CHLORITE FILM COVERED JOINTS		
			2 CORE BOXES OF CORE		

SMITHSONIAN SITE AND PROJECT HOLE NO.

DEPTH OF HOLE, U.S.	OF G.B. & C.	DIAMETER OF HOLE, IN.	
ROCK DRILLED, J.A.	% REC -	DATE STARTED	
AZIMUTH HOLE	MADE	DATE COMPLETED	
INITIALS	VEE	CONTRACT NO.	
SURFACE EL. 867	HOLE NO. TH-61	N 103-132 E 32-283	
ELEVATION FEET	GRA. PHIC LOG	DESCRIPTION OF MATERIALS	REMARKS
925		QUARTZ DIORITE - HARD, MED GRAINED, WHITE & BLACK, MASSIVE, LT VEA, JOINTING	K 12
920		JOINTING, PARALLEL, WEATHERED MUST STAINING	K 100
915			K 65
910			K 65
905		OPEN FISSURE "2" ZONE OF CLOSE JOINTING, LT TO MOD VEA, RUST STAINED	VERY FAST PENETRATION
900			K 24
895		JOINT, MOD VEA, RANDOM, 1/8" MOD GRAINED, GRAY BROWN CRUS	K 26
890		CLOSE JOINTING, LIGHT WEATHERING RUST STAINED C ⁺ - 75° DIP	K 33
885		BASALT DIKE, MILD, DENSE, VERY FINE GRAINED, BLACK, MOD VEA, RUST STAINED.	K 1
880		ZONES OF MOD VEA, RUST STAINING & CHEMICAL ALTERATION DUE TO CON- TACT METAMORPHISM BY BASALT DIKE	
875		BASALT DIKE, HARD, DENSE, APHA- NITIC, BLACK TO GREENISH BLACK - WITH ZONE OF ALTERATION ON TOP & BOTTOM	K 29
870			
867		NOTES: SURFACE ELEV APPROXIMATE	1 BOXES OF CORE

DEPTH OF HOLE 35		OF OB 2.2		DIAMETER OF HOLE 3.5	
ROCK DRILL 1248		% REC 28.8		DATE STARTED 3-10-50	
AZIMUTH HOLE		MADE VERICAL		DATE COMPLETED 3-10-50	
INITIALS				CONTRACT NO.	
SURFACE EL. Approx 558		HOLE NO 62		F 707.77	
				N 95.238	
ELEVATIONS	DEPTH	GRA PHIC LOG	DESCRIPTION OF MATERIALS	REMARKS	
558	0		ORGANIC SILT, DARK BROWN		
	10		QUARTZ DIOBRITE, HARD, MEDIUM GRAIN	K 0.0	
	20		SD, BLACK & WHITE. LOCALLY VARIES TO DIOBRITE, DARK GRAY TO BLACK		
	30		LIGHTLY WEATHERED JOINT		
84.3	40		LIGHTLY WEATHERED OPEN JOINTS	K 42	
	50		LIGHTLY WEATHERED JOINTS	K 19	
835	60		MODERATE WEATHERED JOINTS		
832	70		LIGHTLY WEATHERED JOINTS	K 4.4	
	80		LIGHTLY WEATHERED JOINTS	K 35	
	90		LIGHTLY WEATHERED JOINTS	K 0.1	
787	100		LIGHTLY WEATHERED JOINTS	K 0.0	
	110		MODERATELY WEATHERED JOINTS	K 0.0	
	120		LIGHTLY WEATHERED JOINTS	K 0.55	
778	130		80.0' - 87.0' SOME OF HIGH-ANGLE WEATHERED JOINTS	K 0.34	
752	140				

NOTE: SEE LOG RECORD NUMBER 1 FOR NOTES AND LEGEND

Signature	Date	Signature	Date
<p align="center">U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA</p>			
SUBJECT: D. J. L. HANDEL GRAVES W. C. HANDEL CRICKEN Wm. S. Treadwell HANDEL (none)		<p align="center">SNETTIMHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT</p> <p align="center">LOG RECORD NO. 14</p>	
SUPERVISOR <i>W. D. Dixon</i> SUBVISED BY <i>W. D. Dixon</i> CHECKED BY RECORDED BY <i>Herman Gerd</i> (none)		APPROVED <i>Wm. S. Treadwell</i> SPECIAL AGENT IN CHARGE SCALE DATE 2-10-61 FILE NUMBER SHEET OF	

CORPS OF ENGINEERS

DEPTH OF HOLE 21.1		OF B 33		DIAMETER OF HOLE 11	
ROCK DRILLED 123		% REC 100		DATE STARTED 17 DEC 70	
AZIMUTH HOLE 000		HARD 000		DATE COMPLETED 01 JAN 71	
INITIALS		CONTRACT NO.			
SURFACE EL. 1.9		HOLE NO. 21-1		N 34.555 E 31.818	
ELEVATION FEET	CORRECTION LOG	CORRECTION LOG	DESCRIPTION OF MATERIALS		REMARKS
- 7.1	10.0	10.0	SILT, GRAY BLAC, ORGANIC		TOP OF ROCK
			SANDY GRAVEL, WITH CORNERS, BOLD- END AND FINE GRAINED SAND LENSES		
- 20.0	20.0	20.0	QUARTZ DIORITE - HARD, MED GRAINED WHITE & BLACK, CLOSE JOINTING, SLIGHT TO MODERATE WEA, WITH MUST STAINING & OCCASIONAL CONTINGS OF CHLORITE ON JOINTS.		1001 DRILL WATER RETURN
					1 BOX OF CORE

SHETTISHAW
SITE AND PROJECT

21-1
HOLE NO.

NOTE:
SEE LOG RECORD NUMBER 1 FOR NOTES AND LEGEND

DESIGNED <i>Chas. M. Hall</i>		U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT LOG RECORD NO. 15	
DRAWN <i>Chas. M. Hall</i>			
CHECKED <i>Chas. M. Hall</i>			
BY <i>Chas. M. Hall</i>			
SUPERVISOR <i>W. D. Dixon</i>		APPROVED <i>Chas. M. Hall</i>	
SUBMITTER <i>Chas. M. Hall</i>		SCALE <i>1" = 100'</i>	
RECOMMENDED <i>Chas. M. Hall</i>		DATE <i>25 Oct 61</i>	
DATE <i>25 Oct 61</i>		FILE NUMBER	

PART 8

LOG RECORDS

AUGER HOLES

[illegible][illegible][illegible]

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA EXPLOSION LOG		PROJECT Shettisham (EXPLOSION OCCURRED AT: 10:00) IN 94,340 100 YDS. AND NEARBY E 74,610 100 YDS. OF DISTANCE		SHEET # 1	
FIELD NO. 5 DATE OF EXPLOSION: 11 AUG 57 TIME OF DAY: 11:00 AM TYPE OF WEAPON: 5" HAND AUGER TOTAL NO. OF SAMPLES: 11		NAME OF BULLY: Bellow & Benfield TYPE OF WEAPON: HAND AUGER TYPE OF EXPLOSION: HAND AUGER TYPE OF SAMPLES: 11		NAME OF BULLY: Bellow & Benfield TYPE OF WEAPON: HAND AUGER TYPE OF EXPLOSION: HAND AUGER TYPE OF SAMPLES: 11	
1. NO. OF HOLE: 11 NO. OF SAMPLES: 11		2. NO. OF HOLE: 11 NO. OF SAMPLES: 11		3. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
4. NO. OF HOLE: 11 NO. OF SAMPLES: 11		5. NO. OF HOLE: 11 NO. OF SAMPLES: 11		6. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
7. NO. OF HOLE: 11 NO. OF SAMPLES: 11		8. NO. OF HOLE: 11 NO. OF SAMPLES: 11		9. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
10. NO. OF HOLE: 11 NO. OF SAMPLES: 11		11. NO. OF HOLE: 11 NO. OF SAMPLES: 11		12. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
13. NO. OF HOLE: 11 NO. OF SAMPLES: 11		14. NO. OF HOLE: 11 NO. OF SAMPLES: 11		15. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
16. NO. OF HOLE: 11 NO. OF SAMPLES: 11		17. NO. OF HOLE: 11 NO. OF SAMPLES: 11		18. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
19. NO. OF HOLE: 11 NO. OF SAMPLES: 11		20. NO. OF HOLE: 11 NO. OF SAMPLES: 11		21. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
22. NO. OF HOLE: 11 NO. OF SAMPLES: 11		23. NO. OF HOLE: 11 NO. OF SAMPLES: 11		24. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
25. NO. OF HOLE: 11 NO. OF SAMPLES: 11		26. NO. OF HOLE: 11 NO. OF SAMPLES: 11		27. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
28. NO. OF HOLE: 11 NO. OF SAMPLES: 11		29. NO. OF HOLE: 11 NO. OF SAMPLES: 11		30. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
31. NO. OF HOLE: 11 NO. OF SAMPLES: 11		32. NO. OF HOLE: 11 NO. OF SAMPLES: 11		33. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
34. NO. OF HOLE: 11 NO. OF SAMPLES: 11		35. NO. OF HOLE: 11 NO. OF SAMPLES: 11		36. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
37. NO. OF HOLE: 11 NO. OF SAMPLES: 11		38. NO. OF HOLE: 11 NO. OF SAMPLES: 11		39. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
40. NO. OF HOLE: 11 NO. OF SAMPLES: 11		41. NO. OF HOLE: 11 NO. OF SAMPLES: 11		42. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
43. NO. OF HOLE: 11 NO. OF SAMPLES: 11		44. NO. OF HOLE: 11 NO. OF SAMPLES: 11		45. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
46. NO. OF HOLE: 11 NO. OF SAMPLES: 11		47. NO. OF HOLE: 11 NO. OF SAMPLES: 11		48. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
49. NO. OF HOLE: 11 NO. OF SAMPLES: 11		50. NO. OF HOLE: 11 NO. OF SAMPLES: 11		51. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
52. NO. OF HOLE: 11 NO. OF SAMPLES: 11		53. NO. OF HOLE: 11 NO. OF SAMPLES: 11		54. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
55. NO. OF HOLE: 11 NO. OF SAMPLES: 11		56. NO. OF HOLE: 11 NO. OF SAMPLES: 11		57. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
58. NO. OF HOLE: 11 NO. OF SAMPLES: 11		59. NO. OF HOLE: 11 NO. OF SAMPLES: 11		60. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
61. NO. OF HOLE: 11 NO. OF SAMPLES: 11		62. NO. OF HOLE: 11 NO. OF SAMPLES: 11		63. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
64. NO. OF HOLE: 11 NO. OF SAMPLES: 11		65. NO. OF HOLE: 11 NO. OF SAMPLES: 11		66. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
67. NO. OF HOLE: 11 NO. OF SAMPLES: 11		68. NO. OF HOLE: 11 NO. OF SAMPLES: 11		69. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
70. NO. OF HOLE: 11 NO. OF SAMPLES: 11		71. NO. OF HOLE: 11 NO. OF SAMPLES: 11		72. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
73. NO. OF HOLE: 11 NO. OF SAMPLES: 11		74. NO. OF HOLE: 11 NO. OF SAMPLES: 11		75. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
76. NO. OF HOLE: 11 NO. OF SAMPLES: 11		77. NO. OF HOLE: 11 NO. OF SAMPLES: 11		78. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
79. NO. OF HOLE: 11 NO. OF SAMPLES: 11		80. NO. OF HOLE: 11 NO. OF SAMPLES: 11		81. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
82. NO. OF HOLE: 11 NO. OF SAMPLES: 11		83. NO. OF HOLE: 11 NO. OF SAMPLES: 11		84. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
85. NO. OF HOLE: 11 NO. OF SAMPLES: 11		86. NO. OF HOLE: 11 NO. OF SAMPLES: 11		87. NO. OF HOLE: 11 NO. OF SAMPLES: 11	
88. NO. OF HOLE: 11 NO. OF SAMPLES: 11		89. NO. OF HOLE: 11 NO. OF SAMPLES: 11		90. NO. OF HOLE: 11 NO. OF SAMPLES: 11	

[illegible]

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Snettisham
LOCATION (Latitude & Longitude): N 71, 32, 00 E 152, 12, 00
DATE: 10/1/57
NAME OF ENGINEER: [blank]

EXPLORATION LOG
NO. 1
TYPE OF HOLE: [blank]
NAME OF HOLE: [blank]
DATE OF HOLE: [blank]
TIME OF HOLE: [blank]
TIME OF DAY: [blank]
TIME OF SAMPLES: [blank]
TIME OF ANALYSIS: [blank]
TIME OF REPORT: [blank]

CLASSIFICATION: [blank]
FORMATION DESCRIPTION & REMARKS: [blank]

NOTES: 1. Location approximate, no survey control
2. Hole stopped because of broken auger
3. Water level varies with tide

PROJECT: Snettisham
PERMANENT HOLE NO.: AN 3

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Snettisham
LOCATION (Latitude & Longitude): N 71, 32, 00 E 152, 12, 00
DATE: 10/1/57
NAME OF ENGINEER: [blank]

EXPLORATION LOG
NO. 2
TYPE OF HOLE: [blank]
NAME OF HOLE: [blank]
DATE OF HOLE: [blank]
TIME OF HOLE: [blank]
TIME OF DAY: [blank]
TIME OF SAMPLES: [blank]
TIME OF ANALYSIS: [blank]
TIME OF REPORT: [blank]

CLASSIFICATION: [blank]
FORMATION DESCRIPTION & REMARKS: [blank]

NOTES: 1. Location approximate, no survey control
2. Hole stopped because of broken auger
3. Water level varies with tide

PROJECT: Snettisham
PERMANENT HOLE NO.: AN 4

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Snettisham
LOCATION (Latitude & Longitude): N 71, 32, 00 E 152, 12, 00
DATE: 10/1/57
NAME OF ENGINEER: [blank]

EXPLORATION LOG
NO. 3
TYPE OF HOLE: [blank]
NAME OF HOLE: [blank]
DATE OF HOLE: [blank]
TIME OF HOLE: [blank]
TIME OF DAY: [blank]
TIME OF SAMPLES: [blank]
TIME OF ANALYSIS: [blank]
TIME OF REPORT: [blank]

CLASSIFICATION: [blank]
FORMATION DESCRIPTION & REMARKS: [blank]

NOTES: 1. Location approximate, no survey control
2. Hole stopped because of broken auger
3. Water level varies with tide

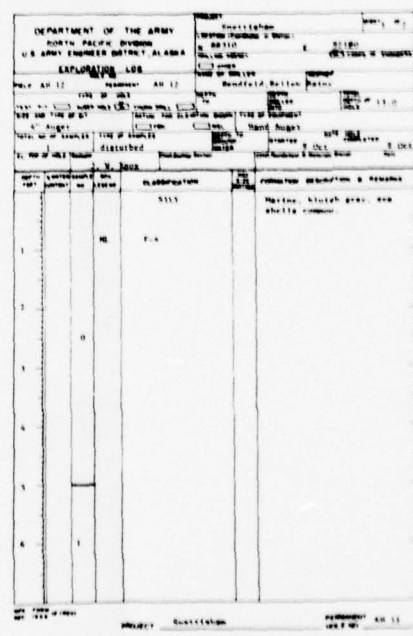
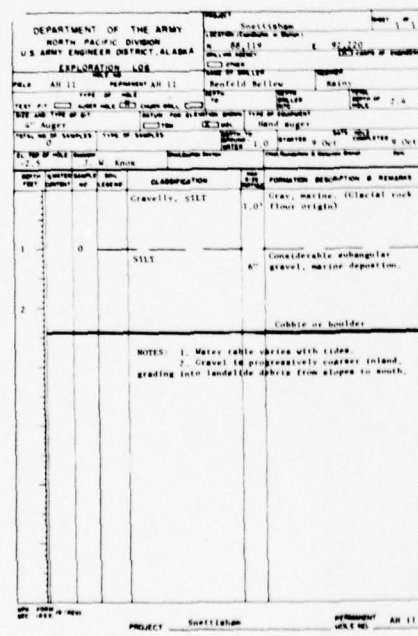
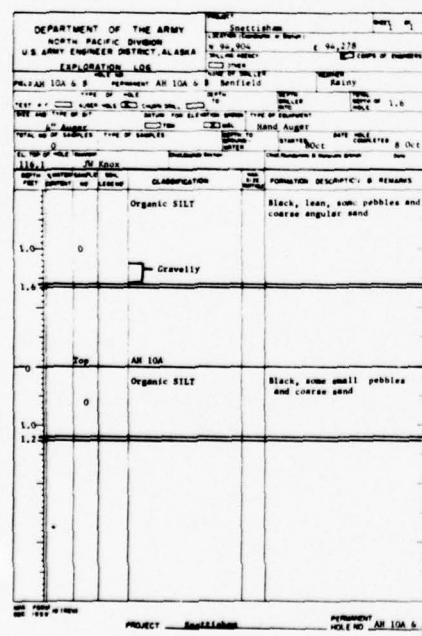
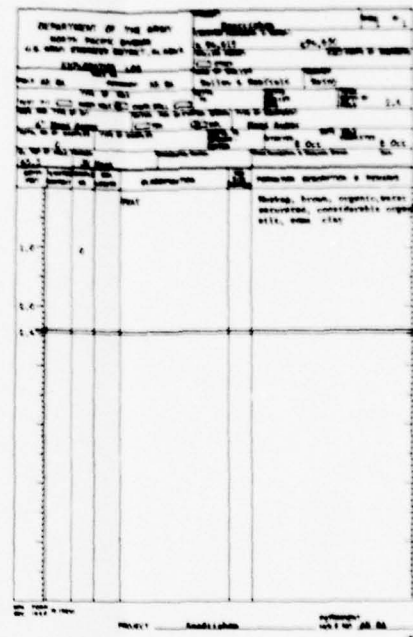
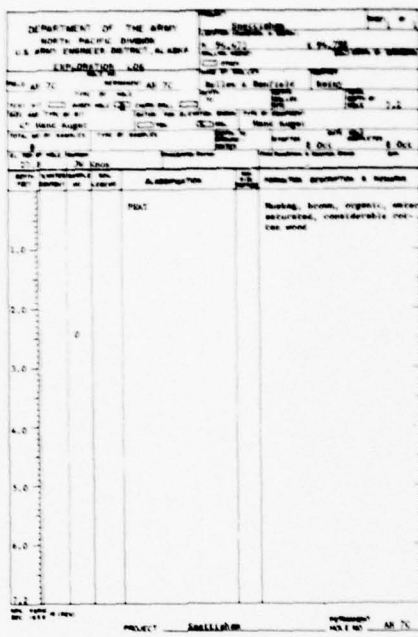
PROJECT: Snettisham
PERMANENT HOLE NO.: AN 7A

U. S. ARMY ENGINEER DISTRICT, ALASKA
CORPS OF ENGINEERS
ANCHORAGE, ALASKA

SNETTISHAM PROJECT, ALASKA
FIRST STAGE DEVELOPMENT
AUGER HOLE LOG
LOG RECORD NO. 1

DESIGNED: [blank]
DRAWN: [blank]
CHECKED: [blank]
PREPARED: [blank]
DATE: [blank]

SUPERVISOR: [blank]
APPROVED: [blank]
SCALE: [blank]
FILE NUMBER: [blank]
SHEET: [blank] OF [blank]



DEPARTMENT OF THE ARMY			
NORTH PACIFIC DIVISION			
ENGINEER DISTRICT, ALASKA			
EXPANSION LOG			
PROJECT: Snettisham			
SHEET: 1			
DATE: 10/10/45			
LOCATION: Snettisham			
ELEVATION: 100			
DISTANCE: 100			
TIME OF DAY: 10:00			
TYPE OF SOIL: 1			
DISTURBED: 1			
REMARKS: 1			
CLASSIFICATION: 1			
FORMATION DESCRIPTION & REMARKS: 1			
0	PEAT		Mudgy, brown, organic, saturated, considerable organic silt, some clay
1	SP Gravelly SAND		Fine subrounded pea gravel and coarse sand, some silt fines

DEPARTMENT OF THE ARMY			
NORTH PACIFIC DIVISION			
ENGINEER DISTRICT, ALASKA			
EXPANSION LOG			
PROJECT: Snettisham			
SHEET: 2			
DATE: 10/10/45			
LOCATION: Snettisham			
ELEVATION: 100			
DISTANCE: 100			
TIME OF DAY: 10:00			
TYPE OF SOIL: 1			
DISTURBED: 1			
REMARKS: 1			
CLASSIFICATION: 1			
FORMATION DESCRIPTION & REMARKS: 1			
1.0	Organic SILT		Gradually sandier with depth.
1.5	Silty SAND		Some clay fines.

DEPARTMENT OF THE ARMY			
NORTH PACIFIC DIVISION			
ENGINEER DISTRICT, ALASKA			
EXPANSION LOG			
PROJECT: Snettisham			
SHEET: 3			
DATE: 10/10/45			
LOCATION: Snettisham			
ELEVATION: 100			
DISTANCE: 100			
TIME OF DAY: 10:00			
TYPE OF SOIL: 1			
DISTURBED: 1			
REMARKS: 1			
CLASSIFICATION: 1			
FORMATION DESCRIPTION & REMARKS: 1			
	SILT		Heavy, squeezing ground, silt flowing into hole.

U. S. ARMY ENGINEER DISTRICT, ALASKA			
CORPS OF ENGINEERS			
ANCHORAGE, ALASKA			
SNETTISHAM PROJECT, ALASKA			
FIRST STAGE DEVELOPMENT			
AUGER HOLE LOG			
LOG RECORD NO. 2			
DESIGNED: Wm. T. T. T.	APPROVED: Chas. J. Farley		
DRAWN: Wm. T. T. T.	SCALE: 1" = 10'		
CHECKED: J. H. H.	DATE: 10/10/45		
RECORDED: J. H. H.	FILE NUMBER: 100		

[illegible][illegible]

DEPARTMENT OF THE ARMY
SOUTH
U.S. ARMY ENGINEERING CENTER
KEMPER

FILE# AM 15

TEST # 44

DATE AND TIME OF TEST
4" August

TYPE OF SOIL

K. TOP OF SHAFT

DEPTH (feet)

WATER (feet)

TEMPERATURE (°C)

8

9

10

11

12

13

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[illegible]

DEPARTMENT OF THE ARMY				PROPERTY		Serial #	
NORTH PACIFIC DIVISION				UNCLASSIFIED		DATE 9/1/85	
U.S. ARMY ENGINEER DISTRICT, ALASKA				DOLLAR VALUE		CLASS OF MATERIAL	
EXPLANATION: SEE				TYPE OF DAMAGE		REMARKS	
MILAN 17		TYPE OF MILE		MILAN 17		MILAN 17	
TEST MILE		MILE NO.		MILE NO.		MILE NO.	
DATE AND TIME OF TEST		TESTER		TESTER		TESTER	
01 August		TYPE OF SAMPLES		TYPE OF SAMPLES		TYPE OF SAMPLES	
TOTAL NO. OF SAMPLES		NO. OF SAMPLES		NO. OF SAMPLES		NO. OF SAMPLES	
10		10		10		10	
TYPE OF MILE		M. Knox		M. Knox		M. Knox	
1		2		3		4	
PROPERTY		DESCRIPTION		PROPERTY		DESCRIPTION	
1.0		SILT		1.0		SILT	
2.0		SILT		2.0		SILT	
3.0		SILT		3.0		SILT	
4.0		SILT		4.0		SILT	
5.0		SILT		5.0		SILT	
6.0		SILT		6.0		SILT	
7.0		SILT		7.0		SILT	
8.0		SILT		8.0		SILT	
9.0		SILT		9.0		SILT	
10.0		SILT		10.0		SILT	
11.0		SILT		11.0		SILT	
12.0		SILT		12.0		SILT	
13.0		SILT		13.0		SILT	
14.0		SILT		14.0		SILT	
15.0		SILT		15.0		SILT	
16.0		SILT		16.0		SILT	
17.0		SILT		17.0		SILT	
18.0		SILT		18.0		SILT	
19.0		SILT		19.0		SILT	
20.0		SILT		20.0		SILT	
21.0		SILT		21.0		SILT	
22.0		SILT		22.0		SILT	
23.0		SILT		23.0		SILT	
24.0		SILT		24.0		SILT	
25.0		SILT		25.0		SILT	
26.0		SILT		26.0		SILT	
27.0		SILT		27.0		SILT	
28.0		SILT		28.0		SILT	
29.0		SILT		29.0		SILT	
30.0		SILT		30.0		SILT	
31.0		SILT		31.0		SILT	
32.0		SILT		32.0		SILT	
33.0		SILT		33.0		SILT	
34.0		SILT		34.0		SILT	
35.0		SILT		35.0		SILT	
36.0		SILT		36.0		SILT	
37.0		SILT		37.0		SILT	
38.0		SILT		38.0		SILT	
39.0		SILT		39.0		SILT	
40.0		SILT		40.0		SILT	
41.0		SILT		41.0		SILT	
42.0		SILT		42.0		SILT	
43.0		SILT		43.0		SILT	
44.0		SILT		44.0		SILT	
45.0		SILT		45.0		SILT	
46.0		SILT		46.0		SILT	
47.0		SILT		47.0		SILT	
48.0		SILT		48.0		SILT	
49.0		SILT		49.0		SILT	
50.0		SILT		50.0		SILT	
51.0		SILT		51.0		SILT	
52.0		SILT		52.0		SILT	
53.0		SILT		53.0		SILT	
54.0		SILT		54.0		SILT	
55.0		SILT		55.0		SILT	
56.0		SILT		56.0		SILT	
57.0		SILT		57.0		SILT	
58.0		SILT		58.0		SILT	
59.0		SILT		59.0		SILT	
60.0		SILT		60.0		SILT	
61.0		SILT		61.0		SILT	
62.0		SILT		62.0			

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA				PROJECT <u>Enfieldishan</u> (OFFICIAL DESIGNATION OF PROJECT) N. 92-390 SHELLS USED: <input type="checkbox"/> none <input checked="" type="checkbox"/> other		INST: # <u>2</u> E 92,740 (C) CODE OF DRAWING	
EXPLANATION—AGE 100% AIR 18 100% AIR 18				NAME OF SHELTER <u>Sanfield & Willow</u>		NUMBER <u>Salicy</u>	
"YES" OR "NO" <input type="checkbox"/> YES <input checked="" type="checkbox"/> NO TYPE AND SIZE OF SHELTER <u>4" AUSTRIAN</u>		TYPE OF SOIL <input type="checkbox"/> SANDY SOIL <input type="checkbox"/> CHALKY SOIL <input type="checkbox"/> GRAVELLY SOIL <input type="checkbox"/> OTHER <u>none</u>		TYPE OF WEATHERING <input type="checkbox"/> NONE <input type="checkbox"/> SLIGHT <input type="checkbox"/> MODERATE <input type="checkbox"/> SEVERE <input type="checkbox"/> EXTREME		TIME OF EXPOSURE <u>13.0</u>	
TIME OF DAY OF SAMPLING <u>11:00</u>		TIME OF SAMPLING <u>11:00</u>		TYPE OF SAMPLING <u>Standard Auger</u>		DATE <u>10/10/50</u>	
NO. OF SHELLS SAMPLED <u>1</u>				DRAINAGE SYSTEM <u>none</u>		DATE OF ANALYSIS <u>9.0</u>	
NAME <u>Enfieldishan</u>				LOCATION (Municipality & Township) <u>Enfieldishan</u>			
DEPTH FEET	LOCATION EASTING	DATE YEAR	CLASSIFICATION	DATE YEAR	REMARKS DESCRIPTION & REMARKS		
1.0			Sandy SILT		Dark blue gray, naturaline deposition, plastic, sensitive		
2.0							
3.0							
4.0							
5.0							
6.0							
7.0							
DATE OF PHOTO <u>10/10/50</u>				PHOTOGRAPHIC HOLES NO. <u>AIR 18</u>			
PROJECT <u>Enfieldishan</u>							

DEPARTMENT NORTH PACIFIC
U.S. ARMY ENGINEER
EXPLOSION

FIELD AM 18

TEST #1 ☐ FUEL #1

DATE AND TIME OF TEST

4" AMBT

TOTAL NO OF SAMPLES

1. NO OF WALKS

DEPTH IN
INCHES

1.0

2.0

3.0

4.0

5.0

6.0

7.0

8.0

9.0

10.0

11.0

12.0

13.0

NO. OF TESTS IN 1960

DEPARTMENT OF THE ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Snettisham		SHEET: 2 of 2	
EXPLOSION LOG		EXPLOSION LOG		EXPLOSION LOG	
PERMANENT AIR 15		PERMANENT AIR 15		PERMANENT AIR 15	
CLASSIFICATION		CLASSIFICATION		CLASSIFICATION	
FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS	
SILT		SILT		SILT	
Plasticity of flowage of clay and squeezing in of hole increase with depth.		Plasticity of flowage of clay and squeezing in of hole increase with depth.		Plasticity of flowage of clay and squeezing in of hole increase with depth.	
NOTE: 1. Classified in field, no laboratory test at this time.		NOTE: 1. Classified in field, no laboratory test at this time.		NOTE: 1. Classified in field, no laboratory test at this time.	
2. Under water at high tide.		2. Under water at high tide.		2. Under water at high tide.	
PROJECT: Snettisham		PROJECT: Snettisham		PROJECT: Snettisham	
PERMANENT AIR 15		PERMANENT AIR 15		PERMANENT AIR 15	

DEPARTMENT OF THE ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Snettisham		SHEET: 1 of 2	
EXPLOSION LOG		EXPLOSION LOG		EXPLOSION LOG	
PERMANENT AIR 16		PERMANENT AIR 16		PERMANENT AIR 16	
CLASSIFICATION		CLASSIFICATION		CLASSIFICATION	
FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS	
SILT		SILT		SILT	
Dark blue gray, oolitic deposition, probable glacial flour in origin.		Dark blue gray, oolitic deposition, probable glacial flour in origin.		Dark blue gray, oolitic deposition, probable glacial flour in origin.	
SILT		SILT		SILT	
Dark blue to greenish gray, marine deposition.		Dark blue to greenish gray, marine deposition.		Dark blue to greenish gray, marine deposition.	
PROJECT: Snettisham		PROJECT: Snettisham		PROJECT: Snettisham	
PERMANENT AIR 16		PERMANENT AIR 16		PERMANENT AIR 16	

DEPARTMENT OF THE ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Snettisham		SHEET: 2 of 2	
EXPLOSION LOG		EXPLOSION LOG		EXPLOSION LOG	
PERMANENT AIR 15		PERMANENT AIR 15		PERMANENT AIR 15	
CLASSIFICATION		CLASSIFICATION		CLASSIFICATION	
FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS	
Sandy SILT		Sandy SILT		Sandy SILT	
PROJECT: Snettisham		PROJECT: Snettisham		PROJECT: Snettisham	
PERMANENT AIR 15		PERMANENT AIR 15		PERMANENT AIR 15	

U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: <i>W. B. Tamm</i>	
DRAWN: <i>W. B. Tamm</i>	
CHECKED: <i>D. J. Lema</i>	
PREPARED: <i>W. B. Tamm</i>	
SUPERVISED: <i>W. B. Tamm</i>	
APPROVED: <i>W. B. Tamm</i>	
SCALE: <i>1/4" = 1'</i>	
DATE: <i>20 Oct 64</i>	
FILE NUMBER: <i>100-10-10-10</i>	
SHEET: <i>2</i> OF <i>2</i>	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA EXPLORATION LOG				PROJECT <u>Sonlightash</u> (REPORT NUMBER OF PROJECT) N 95,348 DIST. NO. <u>1001</u> <input type="checkbox"/> UNDER NAME OF LOCALITY _____		SHEET # <u>1</u> OF <u>2</u> E 94,265 (EASTING OF LOCATION)	
FIELD NO. <u>AN 27</u>		DATE <u>July 27</u>		LOCATION <u>Island & River</u>		RAINY <u>YES</u>	
TYPE OF HOLE _____		MATERIAL <u>GRAVEL</u>		DEPTH OF HOLE <u>2.0</u>		TYPE OF SOIL <u>GRAVEL</u>	
TEST #1 <u>1</u> AUGER HOLE <u>1</u> CHAIN HOLE _____		SECTION <u>1</u>		TYPE OF EQUIPMENT <u>Hand Auger</u>		NO. OF SAMPLES <u>1</u>	
DATE AND TIME OF DAY _____		TIME OF DAY _____		TIME OF DAY _____		TIME OF DAY _____	
6" AUGER _____		12" AUGER _____		18" AUGER _____		24" AUGER _____	
TOTAL NO. OF SAMPLES _____		TYPE OF SAMPLES _____		NO. OF SAMPLES _____		NO. OF SAMPLES _____	
EL. TOP OF HOLE <u>100</u>		IN FEET _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____	
100.0		IN FEET _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____	
100.0		IN FEET _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____	
100.0		IN FEET _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____	
100.0		IN FEET _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____	
100.0		IN FEET _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____		TOTAL DISTANCE TO BOTTOM OF HOLE _____	
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DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION ENGINEER DISTRICT ALASKA		PROJECT Shettisham SHEET NO. 1	
EXPLOSION LOG		EXPLOSION LOG	
TIME OF HOLE 10:00 AM		TIME OF HOLE 10:00 AM	
CLASSIFICATION SILT		CLASSIFICATION SILT	
FORMATION DESCRIPTION & REMARKS Organic, gray, brown to black mudstone origin.		FORMATION DESCRIPTION & REMARKS Organic, gray, brown to black mudstone origin.	
Silty SAND		Silty SAND	
Some pebbles, angular		Some pebbles, angular	
PROJECT Shettisham		PROJECT Shettisham	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION ENGINEER DISTRICT ALASKA		PROJECT Shettisham SHEET NO. 1	
EXPLOSION LOG		EXPLOSION LOG	
TIME OF HOLE 10:00 AM		TIME OF HOLE 10:00 AM	
CLASSIFICATION SILT		CLASSIFICATION SILT	
FORMATION DESCRIPTION & REMARKS Blue gray, organic near sur- face, estuarine origin.		FORMATION DESCRIPTION & REMARKS Blue gray, organic near sur- face, estuarine origin.	
GRAVEL		GRAVEL	
Coarse cobbles & boulders, interstitial silt.		Coarse cobbles & boulders, interstitial silt.	
Stopped by 6" Cobble		Stopped by 6" Cobble	
PROJECT Shettisham		PROJECT Shettisham	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION ENGINEER DISTRICT ALASKA		PROJECT Shettisham SHEET NO. 1	
EXPLOSION LOG		EXPLOSION LOG	
TIME OF HOLE 10:00 AM		TIME OF HOLE 10:00 AM	
CLASSIFICATION SILT		CLASSIFICATION SILT	
FORMATION DESCRIPTION & REMARKS Dark gray, brown organic		FORMATION DESCRIPTION & REMARKS Dark gray, brown organic	
SANDY GRAVEL		SANDY GRAVEL	
Red brown, rust stained		Red brown, rust stained	
Tightly packed cobbles.		Tightly packed cobbles.	
PROJECT Shettisham		PROJECT Shettisham	

U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE ALASKA	
DESIGNED BY: [Signature]	APPROVED BY: [Signature]
DRAWN BY: [Signature]	SCALE DATE: 2008-03
CHECKED BY: [Signature]	FILE NUMBER
PREPARED BY: [Signature]	SHEET OF
SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT AUGER HOLE LOG LOG RECORD NO. 4	
DESIGNED BY: [Signature]	
DRAWN BY: [Signature]	
CHECKED BY: [Signature]	
PREPARED BY: [Signature]	
APPROVED BY: [Signature]	
SCALE DATE: 2008-03	
FILE NUMBER	
SHEET OF	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Shelikhan SHEET: #2	
EXPLOSION LOG		EXPLOSION LOG	
HOLE NO. 19 PERMANENT HOLE NO. 19		HOLE NO. 19 PERMANENT HOLE NO. 19	
TEST PIT: 19		TEST PIT: 19	
TEST AND TIME OF BT: 19		TEST AND TIME OF BT: 19	
TOTAL NO. OF SAMPLES: 19		TOTAL NO. OF SAMPLES: 19	
CLASSIFICATION: Sandy SILT		CLASSIFICATION: Sandy SILT	
FORMATION DESCRIPTION & REMARKS: Dark blue gray		FORMATION DESCRIPTION & REMARKS: Dark blue gray	
PROJECT: Shelikhan		PROJECT: Shelikhan	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Shelikhan SHEET: #2	
EXPLOSION LOG		EXPLOSION LOG	
HOLE NO. 19 PERMANENT HOLE NO. 19		HOLE NO. 19 PERMANENT HOLE NO. 19	
TEST PIT: 19		TEST PIT: 19	
TEST AND TIME OF BT: 19		TEST AND TIME OF BT: 19	
TOTAL NO. OF SAMPLES: 19		TOTAL NO. OF SAMPLES: 19	
CLASSIFICATION: Sandy SILT		CLASSIFICATION: Sandy SILT	
FORMATION DESCRIPTION & REMARKS: Increasingly plastic and fluid with depth		FORMATION DESCRIPTION & REMARKS: Increasingly plastic and fluid with depth	
PROJECT: Shelikhan		PROJECT: Shelikhan	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Shelikhan SHEET: #2	
EXPLOSION LOG		EXPLOSION LOG	
HOLE NO. 20 PERMANENT HOLE NO. 20		HOLE NO. 20 PERMANENT HOLE NO. 20	
TEST PIT: 20		TEST PIT: 20	
TEST AND TIME OF BT: 20		TEST AND TIME OF BT: 20	
TOTAL NO. OF SAMPLES: 20		TOTAL NO. OF SAMPLES: 20	
CLASSIFICATION: SILT		CLASSIFICATION: SILT	
FORMATION DESCRIPTION & REMARKS: Dark blue gray, upper 1.0' some organic matter, returning deposition.		FORMATION DESCRIPTION & REMARKS: Dark blue gray, upper 1.0' some organic matter, returning deposition.	
PROJECT: Shelikhan		PROJECT: Shelikhan	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Shelikhan SHEET: #1	
EXPLOSION LOG		EXPLOSION LOG	
HOLE NO. 22 PERMANENT HOLE NO. 22		HOLE NO. 22 PERMANENT HOLE NO. 22	
TEST PIT: 22		TEST PIT: 22	
TEST AND TIME OF BT: 22		TEST AND TIME OF BT: 22	
TOTAL NO. OF SAMPLES: 22		TOTAL NO. OF SAMPLES: 22	
CLASSIFICATION: Sandy SILT		CLASSIFICATION: Sandy SILT	
FORMATION DESCRIPTION & REMARKS: Dark blue gray, estuarine origin, probable chiefly glacial rock flour, plastic		FORMATION DESCRIPTION & REMARKS: Dark blue gray, estuarine origin, probable chiefly glacial rock flour, plastic	
PROJECT: Shelikhan		PROJECT: Shelikhan	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Shelikhan SHEET: #1	
EXPLOSION LOG		EXPLOSION LOG	
HOLE NO. 23 PERMANENT HOLE NO. 23		HOLE NO. 23 PERMANENT HOLE NO. 23	
TEST PIT: 23		TEST PIT: 23	
TEST AND TIME OF BT: 23		TEST AND TIME OF BT: 23	
TOTAL NO. OF SAMPLES: 23		TOTAL NO. OF SAMPLES: 23	
CLASSIFICATION: SILT		CLASSIFICATION: SILT	
FORMATION DESCRIPTION & REMARKS: Organic, muck-like origin, black to brown		FORMATION DESCRIPTION & REMARKS: Organic, muck-like origin, black to brown	
PROJECT: Shelikhan		PROJECT: Shelikhan	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: Shelikhan SHEET: #1	
EXPLOSION LOG		EXPLOSION LOG	
HOLE NO. 24 PERMANENT HOLE NO. 24		HOLE NO. 24 PERMANENT HOLE NO. 24	
TEST PIT: 24		TEST PIT: 24	
TEST AND TIME OF BT: 24		TEST AND TIME OF BT: 24	
TOTAL NO. OF SAMPLES: 24		TOTAL NO. OF SAMPLES: 24	
CLASSIFICATION: Sandy SILT		CLASSIFICATION: Sandy SILT	
FORMATION DESCRIPTION & REMARKS: Organic, brown		FORMATION DESCRIPTION & REMARKS: Organic, brown	
PROJECT: Shelikhan		PROJECT: Shelikhan	

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Snettisham SHEET: 1 of 1

LOCATION (Grid Zone & Name): N 93 100 E 30 000
DATE OF SURVEY: 10 OCT 1964
TIME OF SURVEY: 10 OCT 1964
NAME OF HOLE: 1
TYPE OF HOLE: ☒ AUGER HOLE ☐ OTHER
PERMANENT: ☒ AN 20
REMARKS: FURLEY & GUSHULKA RAINY
DEPTH OF HOLE: 13.0
TYPE OF SOIL: SILT
SATUR FOR ELEVATION: NONE
TYPE OF EQUIPMENT: HAND AUGER
DATE OF SAMPLE: 10 OCT 1964
TIME OF SAMPLE: 10 OCT 1964
NAME OF SAMPLE: 1
TYPE OF SAMPLE: DISPERSED
DATE OF ANALYSIS: 10 OCT 1964
NAME OF ANALYST: J. W. Kline
TYPE OF ANALYSIS: Standard Methods
FORM NO. 1 (Rev. 1-64)

DEPTH (Feet)	CLASSIFICATION	FORM NO. 1 (Rev. 1-64)	DESCRIPTION & REMARKS
0.0	SILT		AS ABOVE
1.0			
2.0			
3.0			
4.0			
5.0			
6.0			
7.0			
8.0			
9.0			
10.0			
11.0			
12.0			
13.0			

NOTE: 1. Location Approximate, no survey control.
2. Classified in field, no laboratory tests at this time.

PROJECT: Snettisham PERMANENT: AN 20
HOLE NO. 1

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Snettisham SHEET: 1 of 1

LOCATION (Grid Zone & Name): N 93 100 E 30 000
DATE OF SURVEY: 10 OCT 1964
TIME OF SURVEY: 10 OCT 1964
NAME OF HOLE: 1
TYPE OF HOLE: ☒ AUGER HOLE ☐ OTHER
PERMANENT: ☒ AN 21
REMARKS: FURLEY & GUSHULKA RAINY
DEPTH OF HOLE: 6.0
TYPE OF SOIL: SILT
SATUR FOR ELEVATION: NONE
TYPE OF EQUIPMENT: HAND AUGER
DATE OF SAMPLE: 10 OCT 1964
TIME OF SAMPLE: 10 OCT 1964
NAME OF SAMPLE: 1
TYPE OF SAMPLE: DISPERSED
DATE OF ANALYSIS: 10 OCT 1964
NAME OF ANALYST: J. W. Kline
TYPE OF ANALYSIS: Standard Methods
FORM NO. 1 (Rev. 1-64)

DEPTH (Feet)	CLASSIFICATION	FORM NO. 1 (Rev. 1-64)	DESCRIPTION & REMARKS
0.0	SILT		Dark blue gray, plastic, estuarine origin
1.0			
2.0			
3.0			
4.0			
5.0			
6.0			

NOTE: 1. Location Approximate, no survey control
2. Classified in field, no laboratory tests at this time.

PROJECT: Snettisham PERMANENT: AN 21
HOLE NO. 1

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Snettisham SHEET: 1 of 1

LOCATION (Grid Zone & Name): N 93 100 E 30 000
DATE OF SURVEY: 10 OCT 1964
TIME OF SURVEY: 10 OCT 1964
NAME OF HOLE: 1
TYPE OF HOLE: ☒ AUGER HOLE ☐ OTHER
PERMANENT: ☒ AN 25
REMARKS: BLACK & KILGORE RAINY
DEPTH OF HOLE: 1.5
TYPE OF SOIL: Organic SILT
SATUR FOR ELEVATION: NONE
TYPE OF EQUIPMENT: HAND AUGER
DATE OF SAMPLE: 10 OCT 1964
TIME OF SAMPLE: 10 OCT 1964
NAME OF SAMPLE: 1
TYPE OF SAMPLE: DISPERSED
DATE OF ANALYSIS: 10 OCT 1964
NAME OF ANALYST: J. W. Kline
TYPE OF ANALYSIS: Standard Methods
FORM NO. 1 (Rev. 1-64)

DEPTH (Feet)	CLASSIFICATION	FORM NO. 1 (Rev. 1-64)	DESCRIPTION & REMARKS
0.0	Organic SILT		Black and brown peaty, marine origin
1.0			
2.0			
3.0			
4.0			
5.0			
6.0			
7.0			
8.0			
9.0			
10.0			
11.0			
12.0			
13.0			
14.0			
15.0			

NOTE: 1. Water hole 25 ft South of this hole 2.8' deep, contains 4-5" of coarse white gravelly sand, underlying solid material.
Series of similar holes occur across terrace No. 2 near base of terrace No. 3. Holes are up to 4.5' deep.

PROJECT: Snettisham PERMANENT: AN 25
HOLE NO. 1

U.S. ARMY ENGINEER DISTRICT, ALASKA
CORPS OF ENGINEERS
ANCHORAGE, ALASKA

DESIGNED: W. B. TAYLOR
DRAWING: W. B. TAYLOR
CHECKED: D. L. KILGORE
PREPARED: D. L. KILGORE

SNETTISHAM PROJECT, ALASKA
FIRST STAGE DEVELOPMENT
AUGER HOLE LOG
LOG RECORD NO. 5

APPROVED: W. B. Taylor
SCALE: DATE: 20 OCT 1964
FILE NUMBER: 20 OCT 1964
SHEET: 1 OF 1

DEPARTMENT OF THE ARMY		PROJECT <u>Snellgleham</u>		MOBILE UNIT	
NORTH PACIFIC DIVISION		1. Station (Location or Name)		E. MOBILE UNIT	
U.S. ARMY ENGINEER DISTRICT ALABAMA		a. <input type="checkbox"/> MILE AND		SECTION OF DRAWING	
EXPLOSION LOG		b. <input type="checkbox"/> QUART			
DATE <u>10-11-50</u> AM <u>10</u> HOURS <u>10</u> IN		NAME OF MILLER		NAME OF	
TYPE OF HOLE		Cannon-Killer		Railway	
1. <input type="checkbox"/> 2. <input type="checkbox"/> 3. <input type="checkbox"/> 4. <input type="checkbox"/> 5. <input type="checkbox"/> 6. <input type="checkbox"/> 7. <input type="checkbox"/> 8. <input type="checkbox"/> 9. <input type="checkbox"/> 10. <input type="checkbox"/> 11. <input type="checkbox"/> 12. <input type="checkbox"/> 13. <input type="checkbox"/> 14. <input type="checkbox"/> 15. <input type="checkbox"/> 16. <input type="checkbox"/> 17. <input type="checkbox"/> 18. <input type="checkbox"/> 19. <input type="checkbox"/> 20. <input type="checkbox"/> 21. <input type="checkbox"/> 22. <input type="checkbox"/> 23. <input type="checkbox"/> 24. <input type="checkbox"/> 25. <input type="checkbox"/> 26. <input type="checkbox"/> 27. <input type="checkbox"/> 28. <input type="checkbox"/> 29. <input type="checkbox"/> 30. <input type="checkbox"/> 31. <input type="checkbox"/> 32. <input type="checkbox"/> 33. <input type="checkbox"/> 34. <input type="checkbox"/> 35. <input 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DEPARTMENT OF THE ARMY		PROJECT		SNETTISHAM		SHEET # 1	
PACIFIC DIVISION		NORTH PACIFIC DIVISION		ENGINEER DISTRICT ALASKA		ENGINEER DISTRICT ALASKA	
LOCATION LOG		EXPLOSION LOG		EXPLOSION LOG		EXPLOSION LOG	
PERMANENT AD-18		PERMANENT AD-18		PERMANENT AD-18		PERMANENT AD-18	
DATE OF HOLE		DATE OF HOLE		DATE OF HOLE		DATE OF HOLE	
TIME OF HOLE		TIME OF HOLE		TIME OF HOLE		TIME OF HOLE	
NAME OF HOLE		NAME OF HOLE		NAME OF HOLE		NAME OF HOLE	
CLASSIFICATION		CLASSIFICATION		CLASSIFICATION		CLASSIFICATION	
FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS	
1. ML Sandy SILT F-4		1. ML Sandy SILT F-4		1. ML Sandy SILT F-4		1. ML Sandy SILT F-4	
Silty, fine sand, light gray (dry), brown-gray (wet)		Silty, fine sand, light gray (dry), brown-gray (wet)		Silty, fine sand, light gray (dry), brown-gray (wet)		Silty, fine sand, light gray (dry), brown-gray (wet)	
Hole dug below water table for entire depth, troubled by running sands.		Hole dug below water table for entire depth, troubled by running sands.		Hole dug below water table for entire depth, troubled by running sands.		Hole dug below water table for entire depth, troubled by running sands.	
SP- Silty SAND F-2		SP- Silty SAND F-2		SP- Silty SAND F-2		SP- Silty SAND F-2	
NOTE: Location not surveyed.		NOTE: Location not surveyed.		NOTE: Location not surveyed.		NOTE: Location not surveyed.	

DEPARTMENT OF THE ARMY		PROJECT		SNETTISHAM		SHEET # 1	
PACIFIC DIVISION		NORTH PACIFIC DIVISION		ENGINEER DISTRICT ALASKA		ENGINEER DISTRICT ALASKA	
LOCATION LOG		EXPLOSION LOG		EXPLOSION LOG		EXPLOSION LOG	
PERMANENT AD-18		PERMANENT AD-18		PERMANENT AD-18		PERMANENT AD-18	
DATE OF HOLE		DATE OF HOLE		DATE OF HOLE		DATE OF HOLE	
TIME OF HOLE		TIME OF HOLE		TIME OF HOLE		TIME OF HOLE	
NAME OF HOLE		NAME OF HOLE		NAME OF HOLE		NAME OF HOLE	
CLASSIFICATION		CLASSIFICATION		CLASSIFICATION		CLASSIFICATION	
FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS	
1. ML Sandy SILT		1. ML Sandy SILT		1. ML Sandy SILT		1. ML Sandy SILT	
Silty fine sand, light gray (dry) to dark brown-gray (wet), small amount of organic material.		Silty fine sand, light gray (dry) to dark brown-gray (wet), small amount of organic material.		Silty fine sand, light gray (dry) to dark brown-gray (wet), small amount of organic material.		Silty fine sand, light gray (dry) to dark brown-gray (wet), small amount of organic material.	
Hole dug below high-tide and below water table, difficult digging due to running sands.		Hole dug below high-tide and below water table, difficult digging due to running sands.		Hole dug below high-tide and below water table, difficult digging due to running sands.		Hole dug below high-tide and below water table, difficult digging due to running sands.	
2. SM Silty SAND F-2		2. SM Silty SAND F-2		2. SM Silty SAND F-2		2. SM Silty SAND F-2	
NOTE: Location not surveyed.		NOTE: Location not surveyed.		NOTE: Location not surveyed.		NOTE: Location not surveyed.	

DEPARTMENT OF THE ARMY		PROJECT		SNETTISHAM		SHEET # 1	
PACIFIC DIVISION		NORTH PACIFIC DIVISION		ENGINEER DISTRICT ALASKA		ENGINEER DISTRICT ALASKA	
LOCATION LOG		EXPLOSION LOG		EXPLOSION LOG		EXPLOSION LOG	
PERMANENT AD-4		PERMANENT AD-4		PERMANENT AD-4		PERMANENT AD-4	
DATE OF HOLE		DATE OF HOLE		DATE OF HOLE		DATE OF HOLE	
TIME OF HOLE		TIME OF HOLE		TIME OF HOLE		TIME OF HOLE	
NAME OF HOLE		NAME OF HOLE		NAME OF HOLE		NAME OF HOLE	
CLASSIFICATION		CLASSIFICATION		CLASSIFICATION		CLASSIFICATION	
FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS		FORMATION DESCRIPTION & REMARKS	
1. ML SILT F-4		1. ML SILT F-4		1. ML SILT F-4		1. ML SILT F-4	
Light gray sandy silt, many grass-roots.		Light gray sandy silt, many grass-roots.		Light gray sandy silt, many grass-roots.		Light gray sandy silt, many grass-roots.	
2. ML Sandy SILT F-4		2. ML Sandy SILT F-4		2. ML Sandy SILT F-4		2. ML Sandy SILT F-4	
Light gray sandy silt, with very few grass-roots.		Light gray sandy silt, with very few grass-roots.		Light gray sandy silt, with very few grass-roots.		Light gray sandy silt, with very few grass-roots.	
3. ML SILT F-4		3. ML SILT F-4		3. ML SILT F-4		3. ML SILT F-4	
Sandy silt as above.		Sandy silt as above.		Sandy silt as above.		Sandy silt as above.	
Small depth. Hole stopped by running sand.		Small depth. Hole stopped by running sand.		Small depth. Hole stopped by running sand.		Small depth. Hole stopped by running sand.	
Hole was full of water while being drilled.		Hole was full of water while being drilled.		Hole was full of water while being drilled.		Hole was full of water while being drilled.	
NOTE: Location not surveyed.		NOTE: Location not surveyed.		NOTE: Location not surveyed.		NOTE: Location not surveyed.	

U. S. ARMY ENGINEER DISTRICT, ALASKA		CORPS OF ENGINEERS		ANCHORAGE, ALASKA	
DESIGNED		SNETTISHAM PROJECT, ALASKA		FIRST STAGE DEVELOPMENT	
DRAWN		AUGER HOLE LOG		LOG RECORDS NO. 6	
CHECKED					
PREPARED					
SUPERVISOR		APPROVED		DATE 20 Oct 60	
SUBMITTER		SCALE		FILE NUMBER	
RECORDED		SHEET		OF	

[illegible][illegible][illegible]

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT ALASKA EXPLOSION LOG				PROJECT Glacier Creek Gas SWETTIGAM (Location of Explosion) H. 93,177 (Alt. in Feet) (Elevation) DATE OF TEST WEATHER (Type of Weather) (Direction of Wind) (Force of Wind) (Temperature) (Humidity) (Barometer) (Rainfall) (Snowfall) (Ice) (Fog) (Mist) (Clouds) (Moon) (Stars) (Comets) (Aurora) (Other)			
NAME (Full Name) (Last Name) (First Name) (Middle Name) (Suffix) (Rank) (Grade) (Branch) (Assignment) (Station) (Post) (Address) (City) (State) (Zip) (Country)		NAME (Full Name) (Last Name) (First Name) (Middle Name) (Suffix) (Rank) (Grade) (Branch) (Assignment) (Station) (Post) (Address) (City) (State) (Zip) (Country)		NAME (Full Name) (Last Name) (First Name) (Middle Name) (Suffix) (Rank) (Grade) (Branch) (Assignment) (Station) (Post) (Address) (City) (State) (Zip) (Country)		NAME (Full Name) (Last Name) (First Name) (Middle Name) (Suffix) (Rank) (Grade) (Branch) (Assignment) (Station) (Post) (Address) (City) (State) (Zip) (Country)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
DATE (Month) (Day) (Year)		DATE (Month) (Day) (Year)		DATE (Month) (Day) (Year)		DATE (Month) (Day) (Year)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
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TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
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TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)		TIME OF DAY (Hour) (Minute) (Second)	
TIME OF DAY 							

[illegible][illegible]

DEPART 1000
U.S. ARMY
FIELD AIR-5
TEST RUN
6.7 AUGUST
TIME IN OF 20
C. 1000
DEPTH FEET
WIND DIRECTION
WIND SPEED

2
4
6

WIND SPEED 10 KNOTS
WIND DIRECTION 100°

[illegible]

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION ARMY ENGINEER DISTRICT, ALASKA		PROJECT: SHNETTISHAM REPORT: 91,220 DATE: 10/1/55	
EXPLORATION LOG			
NAME OF HOLE: AD-47		NAME OF HOLE: AD-47	
TYPE OF HOLE: 1" Auger		TYPE OF HOLE: 1" Auger	
DATE OF HOLE: 10/1/55		DATE OF HOLE: 10/1/55	
TIME OF HOLE: 10:00 AM		TIME OF HOLE: 10:00 AM	
LOCATION: 100' S of AD-47		LOCATION: 100' S of AD-47	
TYPE OF SAMPLES: 1" Auger		TYPE OF SAMPLES: 1" Auger	
DISCUSSION: 1" Auger		DISCUSSION: 1" Auger	
FINDINGS: 1" Auger		FINDINGS: 1" Auger	
REMARKS: 1" Auger		REMARKS: 1" Auger	
PROJECT: SHNETTISHAM		PROJECT: SHNETTISHAM	
HOLE NO: AD-47		HOLE NO: AD-47	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: SHNETTISHAM REPORT: 91,220 DATE: 10/1/55	
EXPLORATION LOG			
NAME OF HOLE: AD-48		NAME OF HOLE: AD-48	
TYPE OF HOLE: 1" Auger		TYPE OF HOLE: 1" Auger	
DATE OF HOLE: 10/1/55		DATE OF HOLE: 10/1/55	
TIME OF HOLE: 10:00 AM		TIME OF HOLE: 10:00 AM	
LOCATION: 100' S of AD-47		LOCATION: 100' S of AD-47	
TYPE OF SAMPLES: 1" Auger		TYPE OF SAMPLES: 1" Auger	
DISCUSSION: 1" Auger		DISCUSSION: 1" Auger	
FINDINGS: 1" Auger		FINDINGS: 1" Auger	
REMARKS: 1" Auger		REMARKS: 1" Auger	
PROJECT: SHNETTISHAM		PROJECT: SHNETTISHAM	
HOLE NO: AD-48		HOLE NO: AD-48	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT: SHNETTISHAM REPORT: 91,220 DATE: 10/1/55	
EXPLORATION LOG			
NAME OF HOLE: AD-52		NAME OF HOLE: AD-52	
TYPE OF HOLE: 1" Auger		TYPE OF HOLE: 1" Auger	
DATE OF HOLE: 10/1/55		DATE OF HOLE: 10/1/55	
TIME OF HOLE: 10:00 AM		TIME OF HOLE: 10:00 AM	
LOCATION: 100' S of AD-47		LOCATION: 100' S of AD-47	
TYPE OF SAMPLES: 1" Auger		TYPE OF SAMPLES: 1" Auger	
DISCUSSION: 1" Auger		DISCUSSION: 1" Auger	
FINDINGS: 1" Auger		FINDINGS: 1" Auger	
REMARKS: 1" Auger		REMARKS: 1" Auger	
PROJECT: SHNETTISHAM		PROJECT: SHNETTISHAM	
HOLE NO: AD-52		HOLE NO: AD-52	

U.S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: Mr. T. J. T. T.	SHNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT AUGER HOLE LOG LOG RECORDS NO. 7
DRAWN: Mr. T. J. T. T.	
CHECKED: D. H. H.	
PREPARED: D. H. H.	
SUPERVISOR: Mr. T. J. T. T.	APPROVED: Mr. T. J. T. T.
SCALE: 1" = 10'	DATE: 10/1/55
FILE NUMBER: 91,220	
SHEET: 1	OF: 1

PROJECT SKETTISHAM PERMIT NO. 411 58

PROJECT SHETTISHAM PERMANENT HOLE NO AH-59

PROJECT SNETTISHAM PERMANENT FILE NO. AR-63

U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA			
DESIGNED: <i>Wm B Tenthoff</i>		SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT AUGER HOLE LOG LOG RECORDS NO. 8	
DRAWN: <i>Wm B Tenthoff</i>			
CHECKED: <i>D. Hesse</i>			
PREPARED: 			
USED:			
SUPERVISED: <i>MacDion</i>		APPROVED: <i>Wm J Farley</i>	
UNIT: <i>1st COLONY</i>		CONTRACT, OR SURVEY DISTRICT:	
SUBMITTAL: <i>1st Lt B. Jones</i>		SCALE:	
DATE: <i>11-1-66</i>		DATE: <i>20 Oct 65</i>	
RECOMMENDED: <i>James Grogg</i>		FILE NUMBER:	
USED:		SHEET OF:	

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT SNETTISHAN		SHEET 1 OF 1	
EXPLORATION LOG		LOCATION (Continued in Remarks)		E 92,730	
PERM. HOLE NO. 1		PERMANENT HOLE NO. 1		DATE OF LOG 17 JUL 65	
1	Sandy Silt	Non plastic, fine, gray with organic matter.			
2	Sandy Silt	Fine, gray.			
3	Sandy Silt				
4	Sandy Silt				
5	Sandy Silt				
6	Sandy Silt				
8.0 feet - Bottom of Hole					
NOTE: 1. Location 10' NE of "P" line sta. 84+00. 2. Classified in field, no lab tests to date.					

PROJECT SNETTISHAN PERMANENT HOLE NO. AH-64

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT SNETTISHAN		SHEET 1 OF 1	
EXPLORATION LOG		LOCATION (Continued in Remarks)		E 92,730	
PERM. HOLE NO. 2		PERMANENT HOLE NO. 2		DATE OF LOG 17 JUL 65	
1	Sandy Silt	Non plastic, gray, fine with organic matter.			
2	Sandy Silt	Gray, fine.			
3	Sandy Silt				
4	Sandy Silt				
5	Sandy Silt				
6	Sandy Silt				
7	Sandy Silt				
8	Sandy Silt				
9	Sandy Silt				
10	Sandy Silt				
8.9 feet - Bottom of Hole					
NOTE: 1. Location 10' NE of "P" line sta. 84+00. 2. Classified in field, no lab tests to date.					

PROJECT SNETTISHAN PERMANENT HOLE NO. AH-65

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT SNETTISHAN		SHEET 1 OF 1	
EXPLORATION LOG		LOCATION (Continued in Remarks)		E 92,730	
PERM. HOLE NO. 3		PERMANENT HOLE NO. 3		DATE OF LOG 17 JUL 65	
1	Sandy Silt	Non plastic, gray, fine with organic matter.			
2	Sandy Silt	Gray, fine.			
3	Sandy Silt	Gray, fine.			
4	Sandy Silt	Gray, fine.			
5	Sandy Silt	Gray, fine.			
6	Sandy Silt				
7	Sandy Silt				
8	Sandy Silt				
9	Sandy Silt				
10	Sandy Silt				
9.1 feet - Bottom of Hole					
NOTE: 1. Location 10' NE of "P" line sta. 84+00. 2. Classified in field, no lab tests to date.					

PROJECT SNETTISHAN PERMANENT HOLE NO. AH-66

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT SNETTISHAN		SHEET 1 OF 1	
EXPLORATION LOG		LOCATION (Continued in Remarks)		E 92,730	
PERM. HOLE NO. 4		PERMANENT HOLE NO. 4		DATE OF LOG 17 JUL 65	
1	Sandy Silt	Non plastic, gray, fine, with organic matter.			
2	Sandy Silt				
3	Sandy Silt				
4	Sandy Silt				
5	Sandy Silt				
6	Sandy Silt				
7	Sandy Silt				
8	Sandy Silt				
9	Sandy Silt				
10	Sandy Silt				
8.8 feet - Bottom of Hole					
NOTE: 1. Location 10' NE of "P" line sta. 84+00. 2. Auger stopped against rock or large boulder. 3. Classified in field, no lab tests to date.					

PROJECT SNETTISHAN PERMANENT HOLE NO. AH-69

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT SNETTISHAN		SHEET 1 OF 1	
EXPLORATION LOG		LOCATION (Continued in Remarks)		E 92,730	
PERM. HOLE NO. 5		PERMANENT HOLE NO. 5		DATE OF LOG 17 JUL 65	
1	Sandy Silt	Non plastic, fine, gray with organic matter.			
2	Sandy Silt				
3	Sandy Silt				
4	Sandy Silt				
5	Sandy Silt				
6	Sandy Silt				
7	Sandy Silt				
8	Sandy Silt				
9	Sandy Silt				
10	Sandy Silt				
9.5 feet - Bottom of Hole					
NOTE: 1. Location 10' NE of "P" line sta. 84+00. 2. Classified in field, no lab tests to date.					

PROJECT SNETTISHAN PERMANENT HOLE NO. AH-70

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT SNETTISHAN		SHEET 1 OF 1	
EXPLORATION LOG		LOCATION (Continued in Remarks)		E 92,730	
PERM. HOLE NO. 6		PERMANENT HOLE NO. 6		DATE OF LOG 17 JUL 65	
1	Sandy Silt	Slightly plastic, gray, fine with some organic material and organic silt.			
2	Sandy Silt				
3	Sandy Silt				
4	Sandy Silt				
5	Sandy Silt				
6	Sandy Silt				
7	Sandy Silt				
8	Sandy Silt				
9	Sandy Silt				
10	Sandy Silt				
7.4 feet - Bottom of Hole					
NOTE: 1. Location 100' SW of AH-70. 2. Stop due to coarse material or gravel. 3. Classified in field, no lab tests to date.					

PROJECT SNETTISHAN PERMANENT HOLE NO. AH-71

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA				ENGINEER DISTRICT, ALASKA		PROJECT : 01	
EXPANSION LOG				DATE: 9/2/54		NAME OF ENGINEER	
HOLE NO. 1		REMARKS		HOLE NO. 2		REMARKS	
TYPE OF SOIL		TYPE OF SOIL		TYPE OF SOIL		TYPE OF SOIL	
TEST NO. 1		TEST NO. 2		TEST NO. 3		TEST NO. 4	
TEST NO. 5		TEST NO. 6		TEST NO. 7		TEST NO. 8	
TEST NO. 9		TEST NO. 10		TEST NO. 11		TEST NO. 12	
TEST NO. 13		TEST NO. 14		TEST NO. 15		TEST NO. 16	
TEST NO. 17		TEST NO. 18		TEST NO. 19		TEST NO. 20	
TEST NO. 21		TEST NO. 22		TEST NO. 23		TEST NO. 24	
TEST NO. 25		TEST NO. 26		TEST NO. 27		TEST NO. 28	
TEST NO. 29		TEST NO. 30		TEST NO. 31		TEST NO. 32	
TEST NO. 33		TEST NO. 34		TEST NO. 35		TEST NO. 36	
TEST NO. 37		TEST NO. 38		TEST NO. 39		TEST NO. 40	
TEST NO. 41		TEST NO. 42		TEST NO. 43		TEST NO. 44	
TEST NO. 45		TEST NO. 46		TEST NO. 47		TEST NO. 48	
TEST NO. 49		TEST NO. 50		TEST NO. 51		TEST NO. 52	
TEST NO. 53		TEST NO. 54		TEST NO. 55		TEST NO. 56	
TEST NO. 57		TEST NO. 58		TEST NO. 59		TEST NO. 60	
TEST NO. 61		TEST NO. 62		TEST NO. 63		TEST NO. 64	
TEST NO. 65		TEST NO. 66		TEST NO. 67		TEST NO. 68	
TEST NO. 69		TEST NO. 70		TEST NO. 71		TEST NO. 72	
TEST NO. 73		TEST NO. 74		TEST NO. 75		TEST NO. 76	
TEST NO. 77		TEST NO. 78		TEST NO. 79		TEST NO. 80	
TEST NO. 81		TEST NO. 82		TEST NO. 83		TEST NO. 84	
TEST NO. 85		TEST NO. 86		TEST NO. 87		TEST NO. 88	
TEST NO. 89		TEST NO. 90		TEST NO. 91		TEST NO. 92	
TEST NO. 93		TEST NO. 94		TEST NO. 95		TEST NO. 96	
TEST NO. 97		TEST NO. 98		TEST NO. 99		TEST NO. 100	
TEST NO. 101		TEST NO. 102		TEST NO. 103		TEST NO. 104	
TEST NO. 105		TEST NO. 106		TEST NO. 107		TEST NO. 108	
TEST NO. 109		TEST NO. 110		TEST NO. 111		TEST NO. 112	
TEST NO. 113		TEST NO. 114		TEST NO. 115		TEST NO. 116	
TEST NO. 117		TEST NO. 118		TEST NO. 119		TEST NO. 120	
TEST NO. 121		TEST NO. 122		TEST NO. 123		TEST NO. 124	
TEST NO. 125		TEST NO. 126		TEST NO. 127		TEST NO. 128	
TEST NO. 129		TEST NO. 130		TEST NO. 131		TEST NO. 132	
TEST NO. 133		TEST NO. 134		TEST NO. 135		TEST NO. 136	
TEST NO. 137		TEST NO. 138		TEST NO. 139		TEST NO. 140	
TEST NO. 141		TEST NO. 142		TEST NO. 143		TEST NO. 144	
TEST NO. 145		TEST NO. 146		TEST NO. 147		TEST NO. 148	
TEST NO. 149		TEST NO. 150		TEST NO. 151		TEST NO. 152	
TEST NO. 153		TEST NO. 154		TEST NO. 155		TEST NO. 156	
TEST NO. 157		TEST NO. 158		TEST NO. 159		TEST NO. 160	
TEST NO. 161		TEST NO. 162		TEST NO. 163		TEST NO. 164	
TEST NO. 165		TEST NO. 166		TEST NO. 167		TEST NO. 168	
TEST NO. 169		TEST NO. 170		TEST NO. 171		TEST NO. 172	
TEST NO. 173		TEST NO. 174		TEST NO. 175		TEST NO. 176	
TEST NO. 177		TEST NO. 178		TEST NO. 179		TEST NO. 180	
TEST NO. 181		TEST NO. 182		TEST NO. 183		TEST NO. 184	
TEST NO. 185		TEST NO. 186		TEST NO. 187		TEST NO. 188	
TEST NO. 189		TEST NO. 190		TEST NO. 191		TEST NO. 192	
TEST NO. 193		TEST NO. 194		TEST NO. 195		TEST NO. 196	
TEST NO. 197		TEST NO. 198		TEST NO. 199		TEST NO. 200	
TEST NO. 201		TEST NO. 202		TEST NO. 203		TEST NO. 204	
TEST NO. 205		TEST NO. 206		TEST NO. 207		TEST NO. 208	
TEST NO. 209		TEST NO. 210		TEST NO. 211		TEST NO. 212	
TEST NO. 213		TEST NO. 214		TEST NO. 215		TEST NO. 216	
TEST NO. 217		TEST NO. 218		TEST NO. 219		TEST NO. 220	
TEST NO. 221		TEST NO. 222					

[illegible]

CORPS OF ENGINEERS

[illegible]

[illegible]

PART 9

LOG RECORDS - WASH PROBES

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Description
1. AS 113
PROJECT NO. 113
NAME OF SITE, PL.

EXPLORATION LOG

DATE: 10-1-60
TIME: 10:00
BY: J. H. H. (J. H. H.)
PAGE: 10.0

PROBING TYPE OF HOLE
1. AS 113
2. AS 113
3. AS 113
4. AS 113
5. AS 113
6. AS 113
7. AS 113
8. AS 113
9. AS 113
10. AS 113
11. AS 113
12. AS 113
13. AS 113
14. AS 113
15. AS 113
16. AS 113
17. AS 113
18. AS 113
19. AS 113
20. AS 113
21. AS 113
22. AS 113
23. AS 113
24. AS 113
25. AS 113
26. AS 113
27. AS 113
28. AS 113
29. AS 113
30. AS 113
31. AS 113
32. AS 113
33. AS 113
34. AS 113
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37. AS 113
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39. AS 113
40. AS 113
41. AS 113
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43. AS 113
44. AS 113
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65. AS 113
66. AS 113
67. AS 113
68. AS 113
69. AS 113
70. AS 113
71. AS 113
72. AS 113
73. AS 113
74. AS 113
75. AS 113
76. AS 113
77. AS 113
78. AS 113
79. AS 113
80. AS 113
81. AS 113
82. AS 113
83. AS 113
84. AS 113
85. AS 113
86. AS 113
87. AS 113
88. AS 113
89. AS 113
90. AS 113
91. AS 113
92. AS 113
93. AS 113
94. AS 113
95. AS 113
96. AS 113
97. AS 113
98. AS 113
99. AS 113
100. AS 113

CLASSIFICATION

FORMATION DESCRIPTION & REMARKS

NOT TO SCALE

DATE: 10-1-60
TIME: 10:00
BY: J. H. H. (J. H. H.)
PAGE: 10.0

[illegible][illegible]

DEPARTMENT OF THE ARMY		PROJECT		SHEET 2 OF 2	
NORTH PACIFIC DIVISION		LOCATION (Name of Project)		DATE	
U.S. ARMY ENGINEER DISTRICT ALASKA		PROJECT NO.		DATE OF COMPLETION	
EXPLORATION LOG		NAME OF SITE		NUMBER	
HOLE NO. 2		PERMANENT HOLE NO. 2		DATE	
PROBE TO BOTTOM OF HOLE		DATE		TIME OF DAY	
USE AND TYPE OF BIT		TYPE OF EQUIPMENT		TYPE OF SOIL	
NAME OF SAMPLE		DATE		TIME OF DAY	
NO. OF HOLE		DATE		TIME OF DAY	
CLASSIFICATION		DESCRIPTION		REMARKS	
15.0		15.0		15.0	
16.0		16.0		16.0	
17.0		17.0		17.0	
18.0		18.0		18.0	
19.0		19.0		19.0	
20.0		20.0		20.0	
21.0		21.0		21.0	
22.0		22.0		22.0	
23.0		23.0		23.0	
24.0		24.0		24.0	
25.0		25.0		25.0	
26.0		26.0		26.0	
27.0		27.0		27.0	
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90.0		90.0		90.0	
91.0		91.0		91.0	
92.0		92.0		92.0	
93.0		93.0		93.0	
94.0		94.0		94.0	
95.0		95.0		95.0	
96.0		96.0		96.0	
97.0		97.0		97.0	
98.0		98.0		98.0	
99.0		99.0		99.0	
100.0		100.0		100.0	

NOTES: 1. Probing made with 30" drill stem, with approximately 3/8" (inside) diameter.
2. Water jet produced by attaching, bean piston high pressure, low volume type water pump.
3. Hole dropped on own weight and water jet.
No drop without water jet.

PROJECT: Snettisham PERMANENT HOLE NO. 2

U.S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA	
DESIGNED: W. S. TERTWIG	SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT PROBE HOLE LOG LOG RECORD NO. 1
DRAWN: W. S. TERTWIG	
CHECKED: D. L. HARRIS	
REPAIRED:	
SUPERVISOR: W. S. TERTWIG	APPROVED: D. L. HARRIS
SUBMITTED: W. S. TERTWIG	SCALE: DATE: 10/10/64
RECOMMENDED: W. S. TERTWIG	FILE NUMBER:
CHIEF ENGINEER DIVISION:	SHEET OF

PART 10

LOG RECORDS - VANE SHEAR

CORPS OF ENGINEERS

DEPARTMENT OF THE ARMY NORTH PACIFIC DIVISION U.S. ARMY ENGINEER DISTRICT, ALASKA		PROJECT		SECTION		DATE	
EXPLORATION USE		PROJECT		SECTION		DATE	
1	Yane shear (field)	100	100	100	100	100	100
2	2400 100/ft ²	100	100	100	100	100	100
3	2300 100/ft ²	100	100	100	100	100	100
4	1550 100/ft ²	100	100	100	100	100	100
5	1300 100/ft ²	100	100	100	100	100	100

[illegible][illegible]

DEPARTMENT OF THE ARMY					
U.S. ARMY FORM NO. 7-60					
PLATE VS-2					
DATE AND TIME OF ISSUE					
TOTAL NO. OF SHEETS					
CL. TOP OF SCALE					
DEPTH FATH					
SOUNDING					
ECHO					
REMARKS					
15					
16					
17					
18					
19					
20					
21					
22					
23					
APP. NAME					
NO.					
UNIT					

[illegible][illegible][illegible]

DEPARTMENT OF THE ARMY
U.S. ARMY ENGINEERING CENTER
Ft. Belvoir, Montana 59717-3990
FACILITY: 35-4
TEST AREA: ☐
SIZE AND TYPE OF: ☐
TOTAL NO. OF SITES: ☐
E. NO. OF WAYS: ☐
DEPT. & AGENCY: ☐
EST. COMPANY: ☐
1
2
3
4
5
6
7
8
9
10
11
12
13
SHEET 12000 OF 12000

DEPARTMENT OF THE ARMY 87th AERIAL DIVISION 87 ENGINEER DISTRICT, ALASKA		PROJECT SECTION TITLE NAME NO. OF SHEETS DATE OF SHEET		Station Location State Date Name		SHEET # 2	
EXPLORATION LOG							
REMARKS TYPE OF SOIL NUMBER OF SOIL DATE OF SOIL							
SAMPLES TYPE OF SAMPLE LOCATION DATE OF SAMPLE NAME OF SAMPLE							
TESTS TYPE OF TEST DATE OF TEST NAME OF TEST							
CLASSIFICATION TYPE OF CLASSIFICATION DATE OF CLASSIFICATION NAME OF CLASSIFICATION							
INFORMATION DESCRIPTION & REMARKS TYPE OF INFORMATION DATE OF INFORMATION NAME OF INFORMATION							
END OF LOGGING							
NAME DATE							
PROJECT							
REMARKS							

DESIGNED: R. GRIFFITH		DRAWN: R. GRIFFITH		CHECKED: D. Hansen		PREPARED:	
DATE: 10/1/54		BY: [Signature]		BY: [Signature]		BY: [Signature]	
<p align="center">U. S. ARMY ENGINEER DISTRICT, ALASKA CORPS OF ENGINEERS ANCHORAGE, ALASKA</p>							
<p align="center">SNETTISHAM PROJECT, ALASKA FIRST STAGE DEVELOPMENT VANE SHEAR</p>				<p align="center">LOG RECORDS NO. 1</p>			
TITLE SUPERVISED BY: [Signature] DRAWN BY: [Signature]		APPROVED: [Signature] COLONEL, 62 INFANTRY BRIGADE		SCALE:		DATE: 10/1/54	
SUBMITTED BY: [Signature] RECOMMENDATIONS & MATERIALS BRANCH: [Signature]		SECTION:		FILE NUMBER:		SHEET OF:	

CORPS OF ENGINEERS

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Shelikof Sound
EXPLORATION LOG
STAR CORE
GRIFFLIN

TEST VS-4
TYPE OF HOLE: CROWN HOLE
DATE AND TIME OF BT: 20 Sep 55
TOTAL NO. OF SAMPLES: 1
TYPE OF SAMPLES: (field)
EL. TOP OF HOLE: 20.0
DATE: 20 Sep 55

FORMATION DESCRIPTION & REMARKS
Soft gray black organic silt with some silt and wood
Pushed 27 Spoon to a depth of 20 ft by hand

PROJECT: _____ HOLE NO: _____

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Shelikof Sound
EXPLORATION LOG
STAR CORE
GRIFFLIN

TEST VS-5
TYPE OF HOLE: CROWN HOLE
DATE AND TIME OF BT: 20 Sep 55
TOTAL NO. OF SAMPLES: 1
TYPE OF SAMPLES: (field)
EL. TOP OF HOLE: 33.5
DATE: 20 Sep 55

FORMATION DESCRIPTION & REMARKS
Yale silt 259 lb/ft²
170 lb/ft²
364 lb/ft²
259 lb/ft²
170 lb/ft²
1430 lb/ft²
1550 lb/ft²
1430 lb/ft²
1550 lb/ft²

PROJECT: _____ HOLE NO: _____

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Shelikof Sound
EXPLORATION LOG
STAR CORE
GRIFFLIN

TEST VS-5
TYPE OF HOLE: CROWN HOLE
DATE AND TIME OF BT: 20 Sep 55
TOTAL NO. OF SAMPLES: 1
TYPE OF SAMPLES: (field)
EL. TOP OF HOLE: 33.5
DATE: 20 Sep 55

FORMATION DESCRIPTION & REMARKS
Medium fine sand, some silt and some small layers of organic silt
Bottom of RT casing 1170 lb/ft²
Artesian pressure present, soil washed up into casing after making valve about least

PROJECT: _____ HOLE NO: _____

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Shelikof Sound
EXPLORATION LOG
STAR CORE
GRIFFLIN

TEST VS-6
TYPE OF HOLE: CROWN HOLE
DATE AND TIME OF BT: 20 Sep 55
TOTAL NO. OF SAMPLES: 1
TYPE OF SAMPLES: (field)
EL. TOP OF HOLE: 43.5
DATE: 20 Sep 55

FORMATION DESCRIPTION & REMARKS
Blows per foot on RT casing using 75 lb hammer, 15.3 in. drop
Very fine sand and silt, some small layers of organic gravel
Medium fine sand, some gravel and silt
Artesian pressure present
Bottom of RT casing

PROJECT: _____ HOLE NO: _____

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Shelikof Sound
EXPLORATION LOG
STAR CORE
GRIFFLIN

TEST VS-6
TYPE OF HOLE: CROWN HOLE
DATE AND TIME OF BT: 20 Sep 55
TOTAL NO. OF SAMPLES: 1
TYPE OF SAMPLES: (field)
EL. TOP OF HOLE: 43.5
DATE: 20 Sep 55

FORMATION DESCRIPTION & REMARKS
Medium fine sand, some gravel & silt
Fine sand and silt, some layers of organic silt

PROJECT: _____ HOLE NO: _____

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT, ALASKA

PROJECT: Shelikof Sound
EXPLORATION LOG
STAR CORE
GRIFFLIN

TEST VS-6
TYPE OF HOLE: CROWN HOLE
DATE AND TIME OF BT: 20 Sep 55
TOTAL NO. OF SAMPLES: 1
TYPE OF SAMPLES: (field)
EL. TOP OF HOLE: 43.5
DATE: 20 Sep 55

FORMATION DESCRIPTION & REMARKS
Fine sand and silt, some layers of organic silt

PROJECT: _____ HOLE NO: _____

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Griffith SHEET: 2 OF 5

LOCATION: Star Cove DISTRICT OF ENGINEERS: Alaska

EXPLORATION LOG

DATE: 27 Aug 55 TIME OF HOLE: 14:25:00 DEPTH OF HOLE: 67.0

TYPE OF HOLE: VS-7 PERMANENT VS-7

TEST: 1 ☐ HOLE HOLE ☐ CROWN HOLE ☐ 14 25:00 25

SIZE AND TYPE OF BIT: 14 25:00 25

TOTAL NO. OF SAMPLES: 1 TYPE OF SAMPLES: Griffith

DATE OF HOLE: 27 Aug 55 TIME OF HOLE: 14:25:00 DEPTH OF HOLE: 67.0

CLASSIFICATION: Griffith

FORMATION DESCRIPTION & REMARKS: Pushed by hand

DEPTH: 15 16 17 18 19 20 21 22 23 24 25 26 27

CLASSIFICATION: 324 20/ft 259 20/ft

FORMATION DESCRIPTION & REMARKS: Organic silt and silt with large layers of fine sand

PROJECT: Griffith PERMANENT HOLE NO: 27 Aug 55

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Griffith SHEET: 3 OF 5

LOCATION: Star Cove DISTRICT OF ENGINEERS: Alaska

EXPLORATION LOG

DATE: 27 Aug 55 TIME OF HOLE: 14:25:00 DEPTH OF HOLE: 67.0

TYPE OF HOLE: VS-7 PERMANENT VS-7

TEST: 1 ☐ HOLE HOLE ☐ CROWN HOLE ☐ 14 25:00 25

SIZE AND TYPE OF BIT: 14 25:00 25

TOTAL NO. OF SAMPLES: 1 TYPE OF SAMPLES: Griffith

DATE OF HOLE: 27 Aug 55 TIME OF HOLE: 14:25:00 DEPTH OF HOLE: 67.0

CLASSIFICATION: Griffith

FORMATION DESCRIPTION & REMARKS: Organic silt and silt with large layers of fine sand

DEPTH: 29 30 31 32 33 34 35 36 37 38 39 40 41

CLASSIFICATION: 324 20/ft 259 20/ft

FORMATION DESCRIPTION & REMARKS: Organic silt and silt with large layers of fine sand

PROJECT: Griffith PERMANENT HOLE NO: 27 Aug 55

DEPARTMENT OF THE ARMY
NORTH PACIFIC DIVISION
U.S. ARMY ENGINEER DISTRICT ALASKA

PROJECT: Griffith SHEET: 4 OF 5

LOCATION: Star Cove DISTRICT OF ENGINEERS: Alaska

EXPLORATION LOG

DATE: 27 Aug 55 TIME OF HOLE: 14:25:00 DEPTH OF HOLE: 67.0

TYPE OF HOLE: VS-7 PERMANENT VS-7

TEST: 1 ☐ HOLE HOLE ☐ CROWN HOLE ☐ 14 25:00 25

SIZE AND TYPE OF BIT: 14 25:00 25

TOTAL NO. OF SAMPLES: 1 TYPE OF SAMPLES: Griffith

DATE OF HOLE: 27 Aug 55 TIME OF HOLE: 14:25:00 DEPTH OF HOLE: 67.0

CLASSIFICATION: Griffith

FORMATION DESCRIPTION & REMARKS: Organic silt and silt with large layers of fine sand

DEPTH: 43 44 45 46 47 48 49 50 51 52 53 54 55

CLASSIFICATION: 324 20/ft 259 20/ft

FORMATION DESCRIPTION & REMARKS: Organic silt and silt with large layers of fine sand

PROJECT: Griffith PERMANENT HOLE NO: 27 Aug 55

[illegible][illegible]

PART II

DIVING RECONNAISSANCE IN LONG LAKE

DIVING RECONNAISSANCE IN LONG LAKE, ALASKA

Report to
The District Engineer
Department of the Army
U. S. Army Engineer District, Alaska
Corps of Engineers
Anchorage, Alaska

Submitted in partial fulfillment of the requirements
as stated in contract DA-95-507-CIVENG-66-11 (NEG) by

Erk Reimnitz

September 12th, 1965

DIVING RECONNAISSANCE IN LONG LAKE, ALASKA

by

Erk Reimnitz and Neil F. Marshall

In the last week of August, 1965, an underwater geologic reconnaissance near the outlet of Long Lake, Alaska, was undertaken. SCUBA, underwater television, photography, and other methods, were employed by two diving geologists for the purpose of extending geological studies of the area to the lake bottom, in order to aid in the evaluation of its outlet as a potential dam site for a hydro-electric plant.

Regional setting.

The dominant rock type found in the Long Lake area is quartz diorite, associated with some schist and gneiss, all of which are highly resistant to erosion.

Long Lake follows the regional geologic trends, paralleling many of the islands, channels, and inlets of Southeastern Alaska. Its present topography is largely due to erosion by glaciers which covered most of the area during the last major glaciation (Wisconsin). Thus the shape of the basin, with the steep flanks, the basement high at the outlet, and its hanging relationship to the major drainage system, is characteristic for regions affected by alpine glaciation. However, the possibility that the natural dam across the outlet represents a landslide deposit needed further investigation.

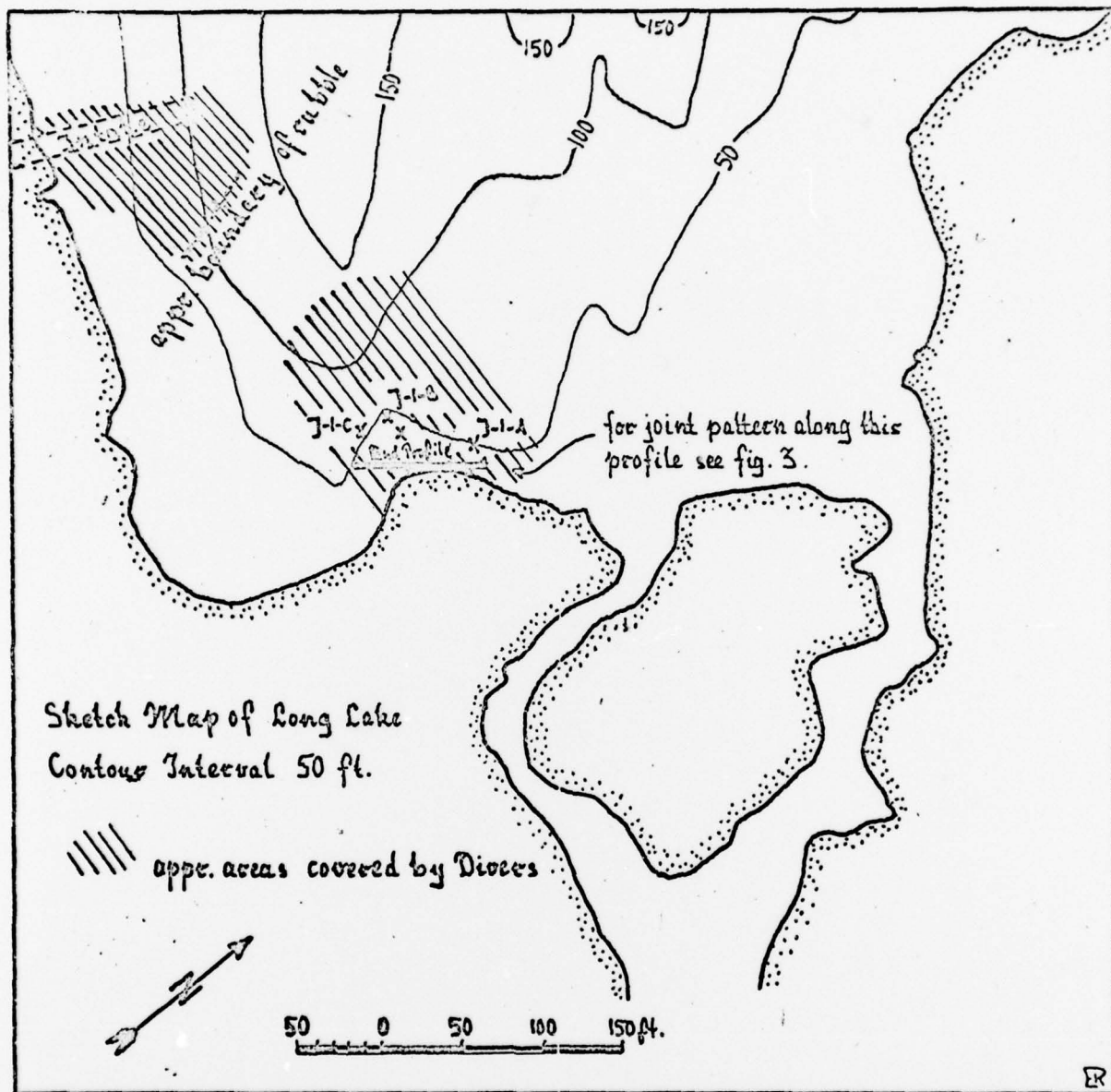
It is clear that in this area, both on the mountain flanks and on the lake bottom (see bathymetry), the topography has hardly been modified by deposition, suggesting that, after the maximum extent of the ice, its retreat was fairly rapid. Sheer mountain sides bearing glacial striae visible from distant vantage points, and the lack of soil and vegetation over large areas, suggest that deglaciation in this valley has taken place later than in the region as a whole.

Results of the present investigation.

Diving reconnaissance was concentrated in two areas: that of the proposed intake location, right abutment, and that of the steep slope of the natural dam, south of the southern branch of the outlet (see map).

Diving at the proposed intake location showed that the smooth, sheer rock face, found on the mountain side above the present lake level, continues to slope at an angle of about 45° to a depth of at least 130 ft., and the depth contours on the survey map suggest that this face continues to the bottom at a depth of about 200 ft. Several high angle joints, extensions of those on the mountain side above, were encountered below the water. Aside from two small pockets, which had collected some debris (Fig. 1), it was found difficult to even get a hand hold, and consequently any coarse material eroded from the mountain side, or supplied by ice rafting, by-passes this area and is deposited at the very lake bottom.

A paper-thin layer of brownish mud covers the slope, so that on touching it, a visible mark is left on the rock. The



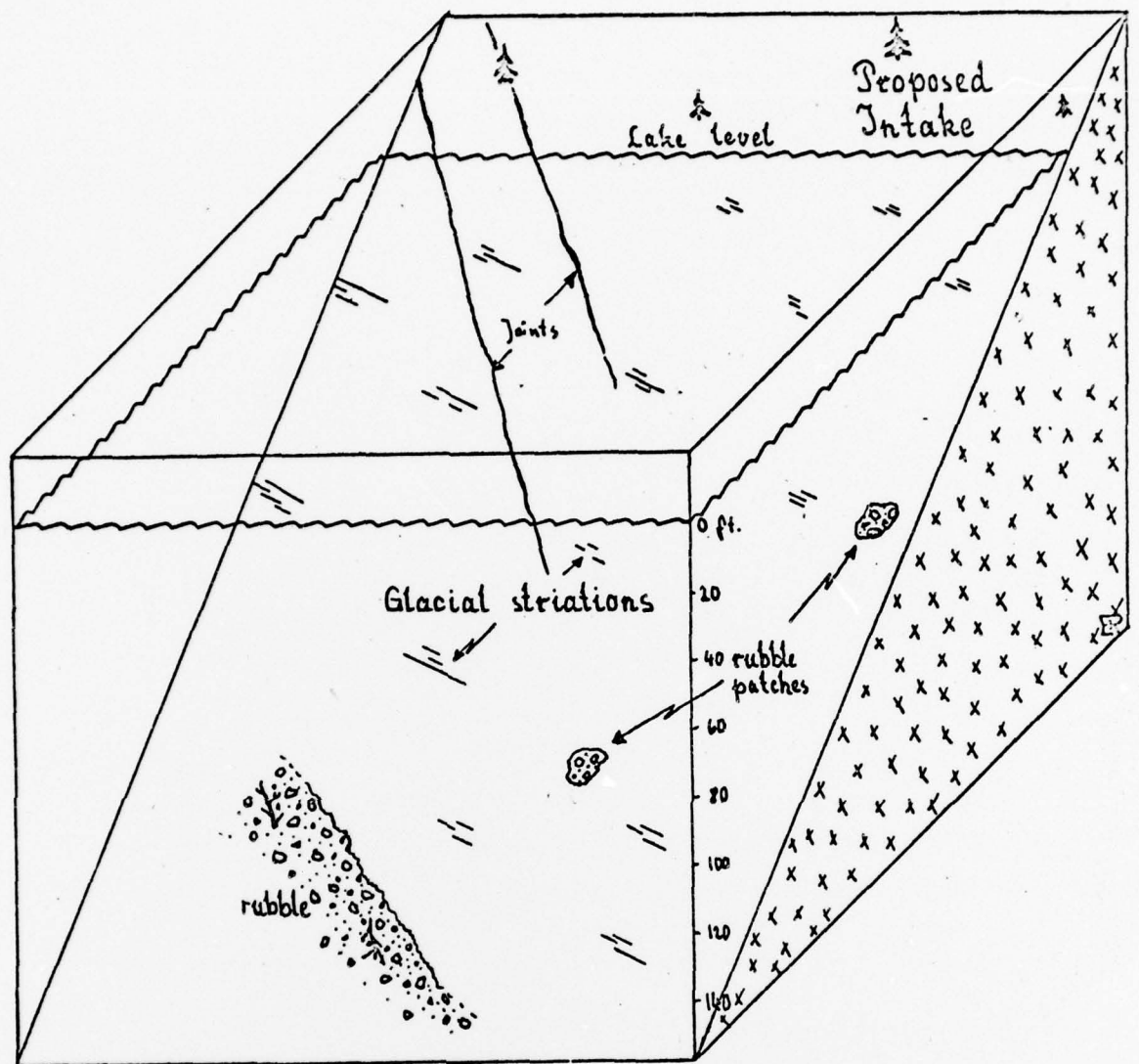


Fig. 1.- Bottom conditions at proposed intake location, right abutment.

absence of such natural marks suggests that very little material is presently traversing the steep slope.

In some areas dark colored streaks were found to extend for several inches downslope from small surface irregularities. These appear to be discolorations of the rock surface.

Glacial striae, similar to the ones seen on the mountain side above in this area, were also observed below water, and were found to slope upward toward the lake's outlet at an angle of approximately 20 to 25 degrees.

The upstream boundary of the rubble found in the southern corner of the lake, was located at a depth of 125 feet and fixed by release of a buoy. (Note: the preliminary position plotted by surveyors on the contour map shows a depth of about 85 feet, and thus should be checked.) This boundary extends upward at an angle of about 45° to the contour lines, to a depth of at least 75 feet (see Fig. 1). The thickness of this rubble, which consists largely of angular blocks of gravel and boulder size, with little mud and a few branches, was no more than a foot near the boundary, and its surface had approximately the same slope as that of the rock face. Here too, the absence of tracks caused by sliding debris on the smooth rock surface above indicates that present rates of supply are low.

In the investigation of the upstream face of the natural dam across the outlet, several interesting observations were made:

- a) Portions of this cliff are overhanging.
- b) Glacial striations and broad gouges trend vertically

across the steep cliff.

c) Low angle joints, similar to those noted on the downstream side, are found.

d) The extremely soft sediment fill at the foot of the cliff, resting at the angle of repose, is littered with sticks, branches, and large trees.

These observations will be treated in some detail.

The upstream face of the natural dam across the lake's outlet, well rounded in the upper reaches by ice action, becomes steep to vertical at a depth of about 15 feet, and in some places is overhanging below about 30 feet (see Fig. 2). Its surface in the undercut portions contrasts very much from that of the upper rounded portions by the hackly, plucked, irregular relief. Individual faces represent smooth fracture planes, but protrusions and corners are sharp. No measurements were made on the amount of undercutting, but it is estimated at 6 to 8 feet.

Glacial striations can be observed on the face of the natural dam to a depth of 50 feet. The deeper occurrences were noted near the outlet, where the cliff is not overhanging. Striations consist both of well defined grooves and ridges and of broad, elongated depressions. They either traverse the face vertically, or point toward the lowest section of the dam near the outlet.

Based on the above observations the following conclusions seem justified: The natural dam across the outlet of Long Lake is not the result of landslide action, nor of tectonism. Its location could be controlled by one or more small fracture zones

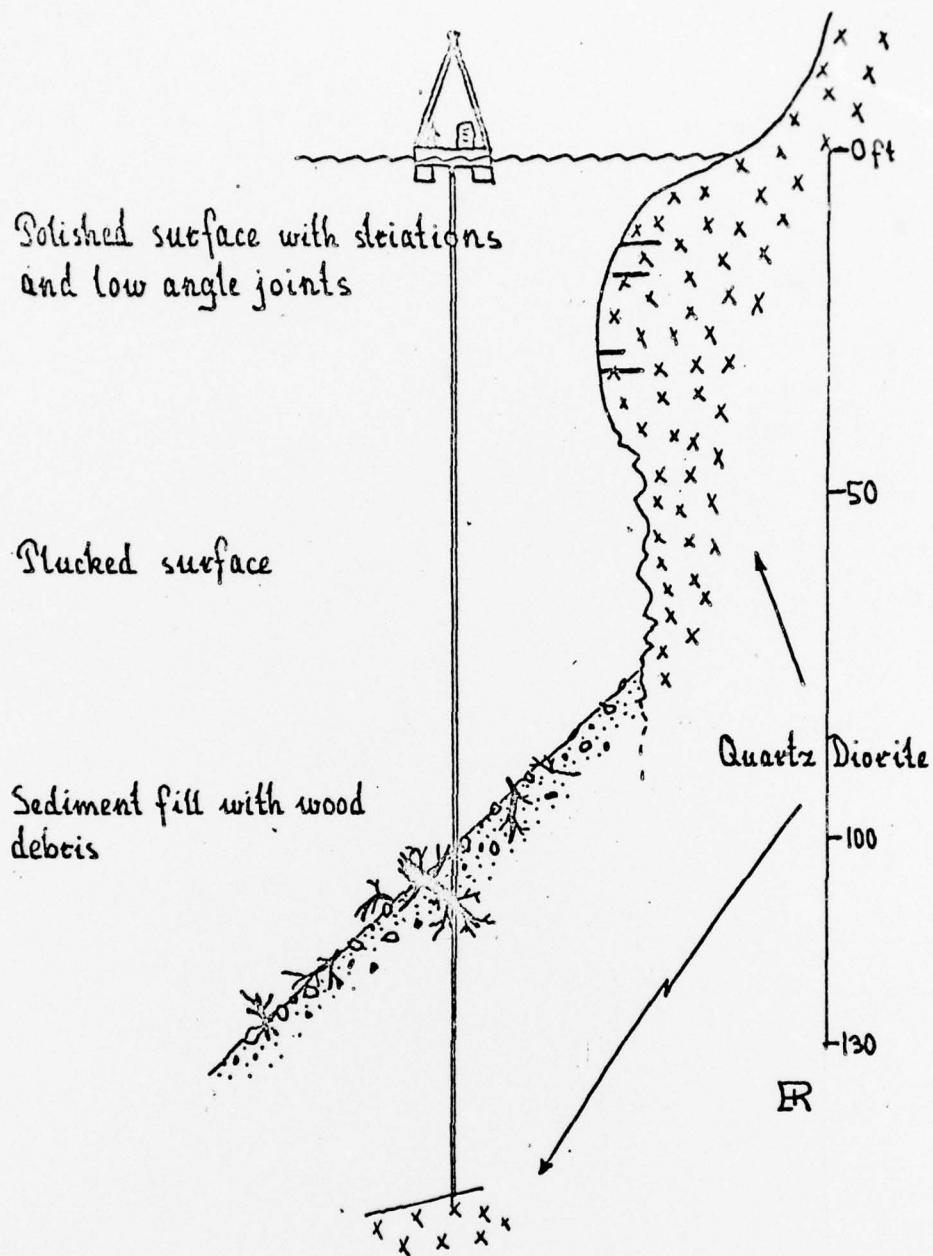


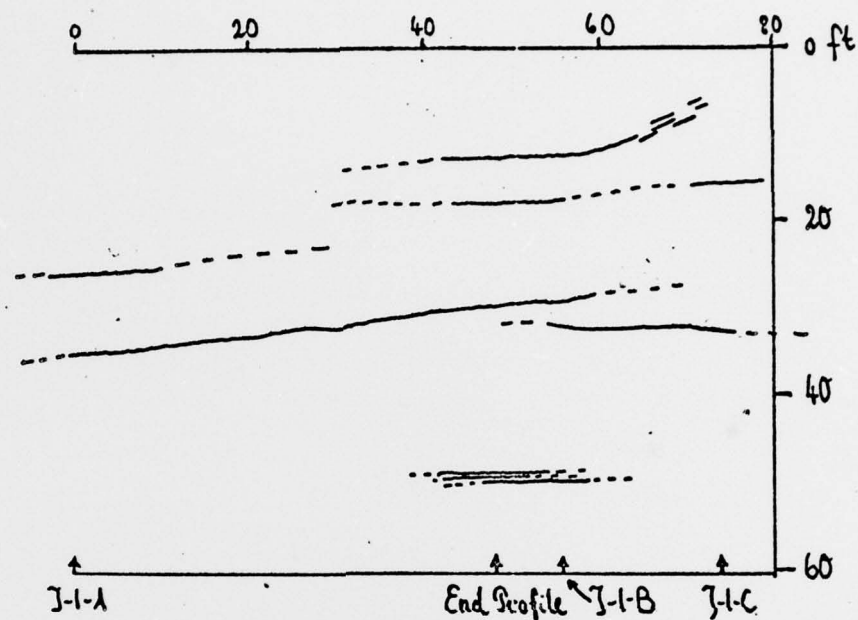
Fig. 2.- Generalized crosssection of upstream face of
natural dam across Long Lake (~ sta. J-1-B).

trending across the lake, one of which coincided with the upstream face. During the maximum extent of the ice the dam as a resistant rock unit withstood its pressure and abrasive forces deflecting the moving ice mass upward by about 200 feet. This is further substantiated by the previously mentioned upward trending glacial striae, at the proposed intake location, right abutment.

The low angle joints in the upper portions of the dam are thought to be due to pressure release associated with the retreat and unloading of the ice (personal communications with Norman Dixon). They are discontinuous and undulating, where exposed.

Observations and measurements made on the joints below lake level show that they are quite similar to those on the downstream face (Fig. 3). The lowest one, consisting of several individual fractures, was found at a depth of about 50 feet. The joints in some places are tight, in others open by about 6-10 inches due to erosion, but always closed at a depth of about one foot from the face of the cliff. One joint was found to pinch out at a place where another one started at a slightly lower level. Another one still ended in a set of very small, sub-parallel fractures.

The pattern of the major low angle joints, as found on the upstream face of the dam, projected onto a vertical plane parallel to the dam, is indicated in Fig. 3. The joints were not traced for their full length, partly due to the poor visibility encountered in the lake. In the lower portions of the natural dam no apparent trends were noted to the jointing.



No overhang
 Indications of ice-
 movement at 50 ft depth.
 (vertical striations)

overhanging cliff
 from ~27 ft downward

Fig. 3.- Diagram of joints surveyed on face of dam
 near outlet as projected onto a flat plane (see map).
 Dashed where inferred.

Below a depth of about 75 feet the face of the rock wall is covered by a wedge of sediment, which, in a drill hole about 20 feet from the cliff is found to be 50 feet thick. Angular boulders and slabs, up to 2 feet in diameter, were encountered on the surface of this wedge, besides material of gravel, sand, silt, and clay size. Because of the high content of silt and clay, however, a field term for this material might be soft, gravelly mud with some boulders. In fact, the high silt and clay content makes this material so soft that a diver can easily penetrate it up to the shoulder with his arm. A sample was collected and given to the chief geologist of the project, but, as some of the finer fractions were washed out during the operation and later ascent, it may not be representative. The presence of numerous twigs, branches, and even trees, in this sediment is striking. One complete tree with a diameter of about one foot at the butt, was found at a depth of 105 feet. Of the area traversed, about one fifth was covered by wood debris. Two wood samples were collected and given to the chief geologist for possible C^{14} analysis. Such age determinations would throw light on the rate of accumulation for wood debris in this area. We believe that this type of material would pose no problem in the operation of the hydro-electric plant.

The mechanism of accumulation for the sediments at the foot of the cliff is not too well understood. Since the dam itself, except for its undercut portions, could not have contributed significant amounts, the source must be looked for elsewhere.

Indicates need for trash same direction

The smooth rock surfaces at the intake location, right abutment, extending to the lake bottom, as well as the relief of the bottom in this area, attest to the slow rates of deposition in the lake as such. It seems conceivable, however, that floating wood, ice, and possibly avalanche snow with incorporated sediment, is gathered by wind against the outlet, and that therefore rates of deposition here are higher than elsewhere in the lake. To establish this point, wind patterns should be defined as well as observations made during breakup of the ice.

Another, perhaps more likely explanation for the greater bulk of the fill, has it consisting of glacial drift, deposited against the dam during a time when the retreating icefront stagnated for some time, or perhaps even readvanced slightly. Since the lake was filled with water since the ice cleared its lower portions, the undercut relationship of the natural dam is difficult to explain by any mechanism except ice plucking.

Miscellaneous observations.

A number of miscellaneous observations made during the diving may be worthwhile mentioning.

The water temperature in the lake was about 38° F. During the diving operation, visibility near the outlet was extremely poor. Daylight did not penetrate deeper than 25 to 30 feet. A large amount of fine suspended matter was noted in the water and possibly was due to the drilling operation in the lake. Near the intake location at a depth of about 70 feet, the water became clearer and visibility with the aid of the TV light was

10-15 feet, but daylight did not penetrate the upper dirty layers of the water.

No currents were noticed on the lake bottom, even near the outlet. The branches and twigs found on the bottom support a ridge of fine sediment, which is easily put into suspension on touch, and then very slowly settles to the bottom.

The only indication of life in the lake were some small cemented sand tubes, up to one inch in length, found at depth hanging along cracks in the rocks.

Part of the diving operation involved the retrieval of a 70 foot long section of drillers pipe, which had been lost near the drilling barge.

Conclusions.

The diving reconnaissance in Long Lake showed that:

- a) The natural dam across the outlet could not represent a slide block since it existed during the last glaciation.
- b) The set of low-angle joints extends to the upstream face of the dam, and appears similar to those studied in other parts.
- c) Rates of deposition in the lake are low.
- d) Wind drift could account for some of the material piled against the dam.

Joint patterns and the sediment-rock wall interface on the upstream face of the dam were surveyed, as well as the boundary of the rubble near the intake location, right abutment.

More material from the rafting

APPENDIX

Five pictures, taken with a 35 mm Calypso camera and flash, are shown. The extremely murky conditions in the lake, well demonstrated by Photo 3, are responsible for the poor quality of the pictures. Here the mercury-vapor lamp of the underwater television camera, illuminating a rod calibrated in feet, barely shows at a distance of four feet.

(Photos not included in DM-7)